# Brine Well Cavity Investigation Program Technical Report

The Detroit River International Crossing Study





February 2008

SUMMARY		S-1
SECTION 1 -	NTRODUCTION	1-1
1.1	Purpose of the Report	
1.2	Geotechnical Analysis Overview	
1.3	U.S. Project Criteria	
1.4	Crossing Descriptions	
	1.4.1 X-10 Crossing Corridor	1-10
	1.4.2 X-11 Crossing Corridor	
SECTION 2 -	REGIONAL GEOLOGIC SUMMARY	2-1
2.1	Overburden	
2.2	Bedrock	
2.3	Groundwater	2-3
	2.3.1 Fresh Groundwater	2-3
	2.3.2 Mineral Groundwater	2-3
	2.3.3 Brackish Groundwater (Brine)	2-4
SECTION 3 -	HISTORICAL INFORMATION	3-1
3.1	Salt Mining	
3.2	Solution Mining Overview	
	3.2.1 X-10 Corridor	
	3.2.2 X-11 Corridor	
SECTION A	THE INVESTIGATION APPROACH	<i>A</i> 1
4.1	Program Concept Development	
4.2	Forward Modeling	
4.2	4.2.1 Forward Modeling of Surface Seismic Techniques	
	4.2.2 Forward Modeling for VSP/RVSP Seismic Techniques	
	4.2.3 Forward Modeling for the Cross-well Seismic Imaging Technique	
	4.2.3.1 Phase I Cross-well Seismic Modeling	
	4.2.3.2 Phase II Cross-well Seismic Modeling	
	4.2.4 Rock Mechanics Forward Modeling	
	4.2.4.1 Phase I Rock Mechanics Modeling	
	4.2.4.2 Phase II Rock Mechanics Modeling	
4.3	Geotechnical Advisory Group	
	4.3.1 Decision Protocol	
4.4	Final Program Details	
		<b>7</b> 1
	FIELD INVESTIGATION	
5.1	Site Conditions	
	5.1.1 X-10 Corridor 5.1.2 X-11 Corridor	
5.2		
5.2	Site Preparation Activities	
	<ul><li>5.2.1 MDEQ Permitting</li><li>5.2.2 Right of Entry</li></ul>	
	5.2.2 Right of Entry 5.2.2.1 Communications Planning	
	5.2.2.1 Communications Planning 5.2.2.2 SCBA Training for City of Detroit	
	5.2.2.2 SCDA framing for City of Deuton	

# **Table of Contents**

# **Table of Contents Continued**

		5.2.2.3	Evacuation Planning	5-4		
		5.2.2.4	Special Insurance Requirements for Right of Entry			
5.3	Additi	onal Proto	col for Investigation Program	5-5		
	5.3.1	Environ	nental Protocol for Drilling	5-5		
		5.3.1.1	Detroit Coke Site (TB-1 through TB-7)	5-5		
		5.3.1.2	Crossing X-11 City of Detroit Parcels (TB-10,12,14,16)	5-5		
		5.3.1.3	Crossing X-11 Private Owners (TB-11, 13, and 15)			
	5.3.2	Noise M	onitoring and Control	5-6		
	5.3.3	Vibratio	n Monitoring and Control	5-6		
	5.3.4		mental Contingency Plan and Procedures			
		5.3.4.1	Training Requirements			
		5.3.4.2	Preparedness and Prevention	5-7		
		5.3.4.3	Contingency Plan	5-7		
<b>SECTION 6</b>	– DRILI	LING PRO	OGRAM	6-1		
6.1	Casing	g Program		6-1		
	6.1.1	Environ	nental Casing	6-1		
	6.1.2	Conduct	or Casing	6-1		
	6.1.3	Surface	Casing	6-2		
	6.1.4	Final Ca	sing	6-2		
	6.1.5	Casing C	Cementing Methods	6-2		
	6.1.6	Blowout	Prevention (BOP) Device	6-2		
6.2	Drillin	ng Fluids	Fluids			
6.3	Rotary	Drilling N	Orilling Methods			
6.4	Coring	g Methods	lethods6-			
6.5	Down	hole Logg	ng Program	6-6		
	6.5.1	Direction	nal/Deviation Surveys	6-6		
	6.5.2	Natural	Gamma Logging	6-6		
	6.5.3	Digital A	Acoustic Logging (Delta-t)	6-6		
	6.5.4	Photoele	ctric (PEF)/Density Logging (Z-Densilog)	6-6		
	6.5.5		erential Borehole Imaging Log			
		(Acousti	c Televiewer – CBIL)	6-7		
6.6	Geoph	ysical Inv	estigation	6-7		
	6.6.1		ell Seismic Imaging Investigation			
	6.6.2	Vertical	Seismic Profiling	6-8		
	6.6.3	Borehole	e Gravity Investigation	6-8		
6.7	Site R	estoration		6-10		
SECTION 7			ANDLING AND TESTING			
7.1		•				
7.2			ge			
	7.2.1		rm			
	7.2.2	Long Te	rm	7-1		
<b>SECTION 8</b>			CONDITIONS AND EVALUATIONS			
8.1						
8.2	•	0 0	erburden			
8.3	Gas C	onditions.	onditions8-2			

# **Table of Contents Continued**

	8.4	Bedroo	ck Condition	ons	8-2
		8.4.1	Direct In	vestigation Results	8-2
			8.4.1.1	Dundee Limestone	8-3
			8.4.1.2	Detroit River Group	8-3
			8.4.1.3	Bois Blanc Formation	8-5
			8.4.1.4	Garden Island Formation	8-5
			8.4.1.5	Bass Islands Group	8-5
			8.4.1.6	Salina Group	8-6
		8.4.2	Cross-we	ell Seismic Imaging Evaluation	
			8.4.2.1	Crossing X-10 Corridor Cross-well Imaging	8-12
			8.4.2.1	Crossing X-11 Corridor Cross-well Imaging	
		8.4.3	Borehole	Gravity Evaluation	8-16
			8.4.3.1	Results of BHGM Modeling	8-16
			8.4.3.2	BHGM Data Analysis	8-17
		8.4.4	Vertical	Seismic Profile Evaluation	8-18
			8.4.4.1	General Method Evaluation	8-18
			8.4.4.2	Imaging the TB-2 Shadow Zone	8-18
			8.4.4.3	Imaging the TB-4 Cross-well Anomaly	
	8.5	Combi	ned Geopl	nysical Investigation	
		(Cross	-well Seisi	nic, BHGM, and VSP)	8-23
		8.5.1		Check on Suspected Solution Features	
		8.5.2		TB-4 and TB-1 to TB-6 Anomaly	
		8.5.3	Ground	Fruthing the Cross-well Results	8-24
			8.5.3.1	Ground Truthing the TB-1 to TB-2 Cross-well Profile	8-24
			8.5.3.2	Synthetic Seismograms	8-25
		8.5.4	Cross-W	ell Seismic Profile Quality Evaluation	8-26
			8.5.4.1	Signal Power	8-26
			8.5.4.2	Noise, Ambient and Electrical	8-26
			8.5.4.3	Signal to Noise Ratio	8-27
			8.5.4.4	Angle of Incidence	8-27
			8.5.4.5	Index of Image Quality	8-27
8.6	Confir	mation o	of Rock M	echanics Model	8-28
		8.6.1	Prelimina	ary Geotechnical Rock Mass Simulations	8-28
			8.6.1.1	Stability of Existing Solution Cavities	8-29
		8.6.2	Prelimina	ary Lithology and Cavity Geometry Failure Model	8-29
			8.6.2.1	Small Isolated Cavities	8-29
			8.6.2.2	Cavity Roof is Stabilized Against Overlying Bed	8-29
			8.6.2.3	Cavity Where Bulking of Rock Arrests Propagation	8-29
			8.6.2.4	Cavities Where Collapse Progresses to Surface	8-29
		8.6.3	Results of	of Rock Mechanics Modeling	8-29
SECT	ION 9 -	CONC	LUSIONS	5	9-1
2201	9.1			e Cavities	
	<i>,</i> ,,,	9.1.1		X-10 Corridor	
		9.1.2		X-11 Corridor	
	9.2			nalies	
			•		
REFEI	RENCES	S			9-4

# **Table of Contents Continued**

Appendix A Appendix B	Detailed Well Schematics Logs of Rotary Boring
Appendix C	Logs of Core Boring
Appendix D	Photographs of Rock Cores
Appendix E	Referenced Figures
Appendix F	Report on Results of Rock Testing
Appendix G	Final Report, Detroit River International Crossing, Cross-well Seismic Imaging, by Roger Turpening and Carol Asiala, Michigan Technological University, Dated January 18, 2008.
Appendix H	Final Report, Detroit River International Crossing, Borehole Gravimeter Surveys, by Jimmy F. Diehl, Michigan Technological University, Dated January 18, 2008.
Appendix I	Stability of Solution Caverns in Salt, Rock Mechanics Investigation for Proposed Detroit River International Crossing, by Edward J. Cording, Dated October 2007
Appendix J	Explanation of Freznel Zone
CD-1	Downhole Wireline Logs Daily Field Reports

#### LIST OF FIGURES

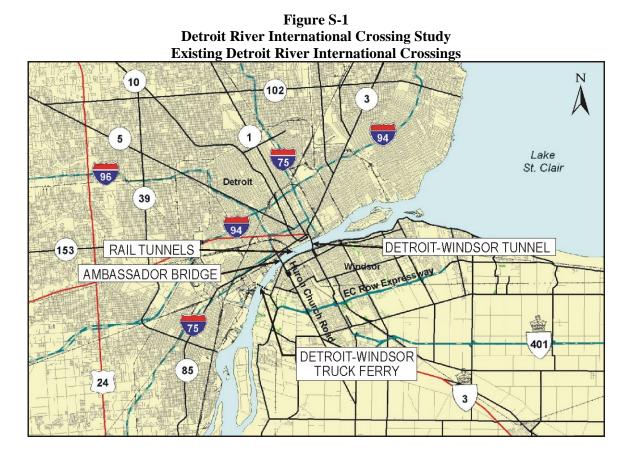
Figure S-1	Existing Detroit River International CrossingsS-1
Figure S-2	U.S. Area of Analysis for Crossing SystemS-2
Figure S-3	Sinkhole in Windsor, CanadaS-3
Figure S-4	Cross-Well Seismic Data Acquisition SchematicS-6
Figure S-5	Generalized Bedrock LithologyS-8
Figure S-6	Apparent Anomalies (Crossing X-10)S-9
Figure 1-1	Detroit River International Crossings1-1
Figure 1-2	Travel Demand vs. Capacity: Combined Detroit River Crossings1-2
Figure 1-3	U.S. Area of Analysis for Crossing System1-3
Figure 1-4	Schematic Representation of X-10 Crossing
	Alternatives #1 through #3, #5, #14 and #161-5
Figure 1-5	Schematic Representation of X-11 Crossing
	Alternatives #7, #9, #111-6
Figure 1-6	Canadian Sinkhole1-7
Figure 1-7	Corridor and Historic Solution Mining Location Plan1-8
Figure 3-1	Solution Salt Mining/Brine Wells
Figure 3-2	Conceptual Representation of Sinkhole Propagation to Surface
Figure 4-1	Test Boring Location Plan – Site X-10
Figure 4-2	Test Boring Location Plan – Site X-11
Figure 4-3	Cross-well Seismic Imaging – X-104-10
Figure 4-4	Cross-well Seismic Imaging – X-114-11
Figure 4-5	Final Coverage Map, X-10 Crossing4-12
Figure 4-6	Final Coverage Map, X-11 Crossing
Figure 8-1	Existing Anomalies
Figure 8-2	Offset, P-wave VSP Shot into Borehole TB-2
Figure 8-3	Logs from the X-10 Corridor
Figure 8-4	Raw Spectrum of Direct, First Arrivals in the VSP
Figure 8-5	Spectrum of Reflections, After Deconvolution, for the TB-2 VSP8-22
Figure 8-6	Offset P-wave VSP Shot into Borehole TB-4

#### LIST OF TABLES

Table 1-1	Crossing Systems Included in DRIC DEIS1	-4
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# SUMMARY

The Detroit River International Crossing (DRIC) Study is a bi-national effort to complete the environmental study processes for the border crossing between Detroit, Michigan and Windsor, Ontario for the United States, Michigan, Canada and Ontario governments. The study will identify solutions that support the region, state, provincial and national economies while addressing civil defense, national defense, and homeland security needs of the busiest trade corridor between the United States and Canada (Figure S-1).



Source: The Corradino Group of Michigan, Inc

The purpose of the DRIC Project for the foreseeable future (at least 30 years) is to:

- Provide safe, efficient, and secure movement of people and goods across the U.S-Canadian border in the Detroit River area to support the economies of Michigan, Ontario, Canada, and the U.S.
- Support the mobility needs of national and civil defense to protect the homeland.

To address future mobility requirements (i.e., at least 30 years) across the U.S-Canadian border, there is a need to:

- Provide new border crossing capacity to meet increased long-term demand;
- Improve system connectivity to enhance the seamless flow of people and goods;
- Improve operations and processing capability;
- Provide reasonable and secure crossing options in the event of incidents, maintenance, congestion, or other disruptions.

The DRIC Study Draft Environmental Impact Statement (DEIS) analyzes issues/impacts on the U.S. side of the crossing system over the Detroit River between Detroit, Michigan, and Windsor, Ontario, Canada. The alternatives are comprised of three components: the crossing, plaza (where tolls are collected), and interchange connecting the plaza to I-75 (Figure S-2).

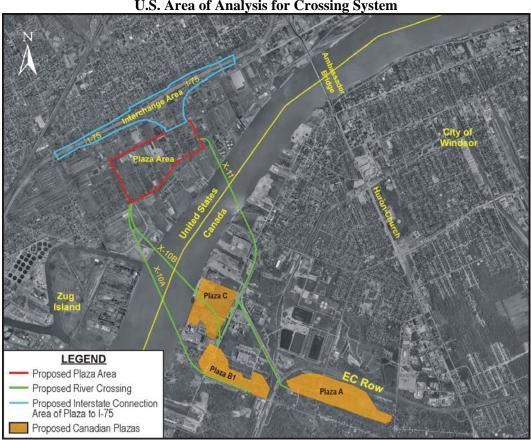


Figure S-2 Detroit River International Crossing Study U.S. Area of Analysis for Crossing System

Source: The Corradino Group of Michigan, Inc.

# **Purpose of the Report**

Salt (halite) has historically been solution mined in the area of the DRIC Practical Alternatives. As part of this solution mining process, fresh water was injected into the ground, natural salt beds were dissolved, and the resulting brine was brought to surface and evaporated to make salt. The solution mining of salt layers ranging from 275 to 490 m (900 to 1,600 feet) below the ground surface was typically conducted in an uncontrolled method before standardized record keeping was common practice. This created underground cavities of unknown location, size, and dimension. A solution mining cavity collapsed to the surface and formed a sinkhole on the Windsor side of the study area in 1954 (Figure S-3). At least two additional sinkholes occurred at Point Hennepin (on Grosse Isle) south of the DRIC crossing site on the U.S. side of the river.

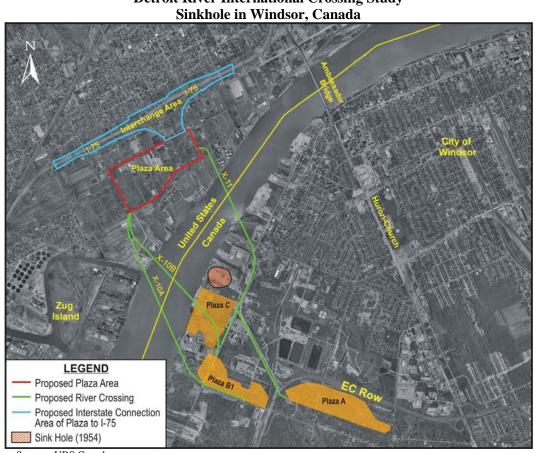


Figure S-3 Detroit River International Crossing Study Sinkhole in Windsor, Canada

Source: URS Canada

Also, settlement of up to several meters were observed near the Wyandotte, MI brinefield.

After consideration of the available data, a Brine Well Cavity Investigation program was developed to delineate the size, locations, and shape of potential brine well cavities in the two proposed crossing corridors on the U.S. side of the crossings. Approval was obtained from Michigan Department of Transportation (MDOT) for a combined geophysical and geotechnical program, which included the drilling of multiple deep-rock borings in combination with Cross-well seismic imaging. "Forward modeling," borehole gravity, vertical seismic profiling (VSP),

and downhole wireline logging were also included in the program. This report presents a summary of the data obtained on the U.S. side during the investigation program.

#### **Geotechnical Analysis Overview**

In early 2006, an intensive literature search of historical brine well operations in the X-10 and X-11 corridors was completed. The results were presented in a report entitled "Preliminary Report on Historical Solution Mining Activities, Crossing X-10 and X-11 Corridors," dated March 24, 2006 and presented under separate cover. The results of this report are summarized herein. A program to then investigate the location, size, and geometry of potential brine well cavities in the proposed crossing location areas was developed by NTH Consultants, Ltd. and the DRIC Team.

As part of the program, a panel of international experts, known as the Geotechnical Advisory Group, was assembled to examine the program's intended plan and conclusions. Members of the Advisory Group, listed alphabetically, are:

Jerry DiMaggio, U.S. Federal Highway Administration Chantale Doucet, Natural Resources Canada Dave Dundas, Ontario Ministry of Transport Dick Endres, Michigan Department of Transportation<sup>1</sup> Peter Gerabek, Public Works and Government Services Canada Khamis Haramy, U.S. Federal Highway Administration Dave Juntunen, Michigan Department of Transportation Tae C. Kim, Ontario Ministry of Transport Stephen McKinnon, Queens University Richard Miller, University of Kansas Pat O'Rourke, Michigan Department of Transportation<sup>1</sup> Leo Rothenburg, University of Waterloo Richard Woods, University of Michigan

The Group met four times in the Detroit-Windsor area and five times by teleconference between June 2006 and February 2008.

# U.S. Project Criteria

Based on criteria established by MDOT in January 2006 and further defined at a June 2006 Geotechnical Advisory Group Meeting, the proposed bridge in Corridors X-10 and X-11 requires: 1) foundations be located outside of the influence of any rock cavities that could have impact on the foundations, including those produced by solution mining activities; and, 2) foundations be built on competent bedrock. The brine well investigation program was developed and implemented to define conditions in the corridors to determine if these criteria could be satisfied.

<sup>&</sup>lt;sup>1</sup> In 2007, Dick Endres of MDOT replaced Pat O'Rourke upon his retirement.

Figure S-4 Detroit River International Crossing Study

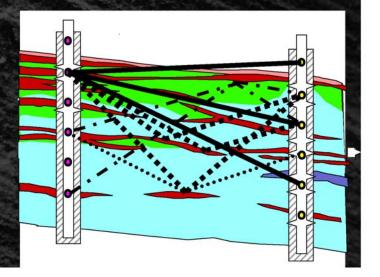
# **Crosswell Data Processing**

Objectives

Image formations from
 400 to 1750 feet in
 depth at high resolution

Approach

Crosswell seismic
 processing using
 combined up and
 downgoing imaging



1

Source: Z-Seis Reservoir Seismic

#### **Final Results**

A total of 12 cross-well seismic imaging profiles were performed for the X-10 corridor and 16 profiles for the X-11 corridor. A general schematic of the cross-well seismic imaging objectives and approach is presented in Figure S-4. The final processed images have vertical resolution of about two to three meters (seven to ten feet) in both corridors, together with horizontal resolution of approximately 6 to 8 m (20 to 25 feet) in the X-10 corridor and 9 to 11 m (30 to 35 feet) in the X-11 corridor.

Analysis of the cross-well images provides evidence that the upper contacts of the F, D, and B-Salt layers are generally unbroken and continuous in both corridors. Some images underwent additional processing and interpretation to provide further clarity and resolution, due to ambient seismic and electrical noise .The general lithology of the bedrock in the subject area, including the relative depths of the various salt layers, is presented for reference in Figure S-5.

The processed cross-well images and other geophysical data show two anomalies of interest, neither of which is of significant concern. At approximately 107 m (350 feet) from the borehole labeled TB-6, at a relative depth of 338 m (1,110 feet), a feature is visible in the data. It is approximately 38 m (125 feet) wide and 6 to 8 m (20 to 25 feet) high (Anomaly A on Figure S-6). The feature is observed in the top of the F-Unit of the Salina Group (F2 bed) and the roof appears to be at the contact with the overlying shaley rock. A "crooked" stem can be viewed directly below the main portion of the feature, ending at the underlying E-Dolomite. The seismic reflectors associated with the underlying D and B-Units are unbroken below this feature, indicating that if the feature is the result of historical solution mining, the mining was not performed in the lower salt units. Most notably, the shale and carbonate stringers in the B-Salt are visible and continuous.

The profile from the boreholes labeled TB-6 to TB-4 was reprocessed using true-amplitude seismic processing techniques to provide some insight into whether the observed feature is brine or rubble-filled. The results illustrate the feature is potentially filled with material ranging from bulked debris fallen from the roof (most likely), possible limited debris flow from above (through the initial well), and the recrystallization of salt.

A second and relatively small anomaly is present in cross-well profiles TB-1 to TB-4 and TB-1 to TB-6. It is observed at the top of the B-Salt/C-Shale contact (Anomaly B on Figure S-6). The anomaly is approximately 37 to 52 m (120 to 170 feet) wide and 6 to 8 m (20 to 25 feet) high. The feature appears to have a tabular (i.e., "hockey-puck") shape. Upon initial review, it appeared the anomaly could have been caused by natural solutioning (and possibly natural recrystallization), artificial solutioning (mining), or was simply a naturally-occurring variation in the B-Salt. Reprocessing the data using true-amplitude techniques indicates this feature is apparently filled with material (not brine).

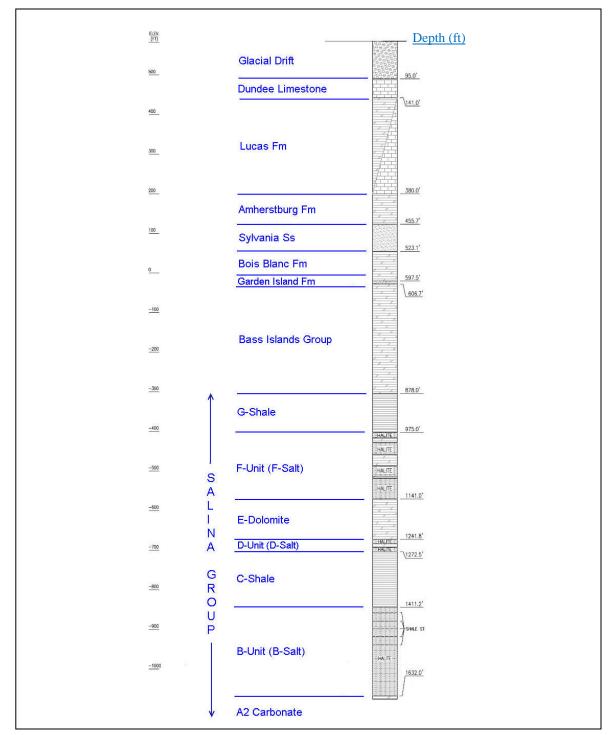
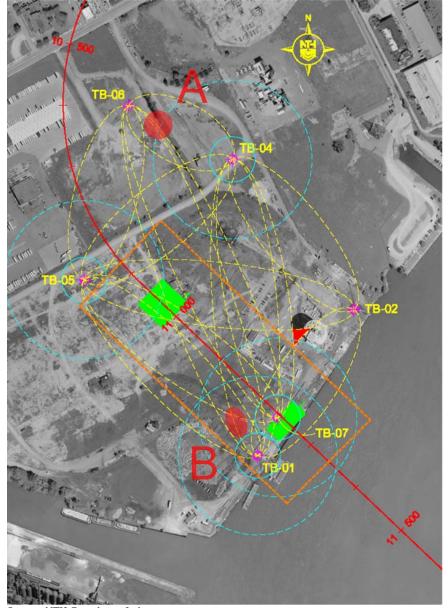


Figure S-5 Detroit River International Crossing Study Generalized Bedrock Lithology

Source: NTH Consultants, Ltd.

#### **Figure S-6 Detroit River International Crossing Study Apparent Anomalies (Crossing X-10)**



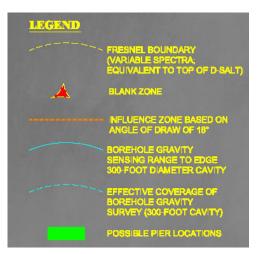
#### Anomaly "A":

- Size: About 20 to 25 feet high, about 125 ft diameter. ٠
- Depth: Centered at about 1100 feet BGS. •
- Shape: Round in Plan, "Morning Glory" in Profile. •
- Probably fully or partially "bulked-up," possible limited • recrystallization.

#### Anomaly "B":

- Size: About 6.1 to 7.6 m (20 to 25 feet) high, about 36.6 to 51.8 m (120 to 170 feet) in diameter.
- Depth: Centered at about 429.8 m (1410 feet) BGS. ٠
- Shape: Elliptical in Plan, Hockey Puck in profile. .
- Most likely a density anomaly within the B-Salt, where • density is lower due to geological deposition (recrystallization, high purity, etc.)

Note: No anomalies greater than those illustrated in this figure were detected in the X-11 Crossing area investigated.



As this anomaly is very close to a borehole, it was also examined using Borehole Gravity techniques (BHGM), which indicate a very small feature or less-dense zone. A more-refined estimate of the B-Salt density has been obtained to re-calculate apparent-density values for the TB-1 BHGM survey. Utilizing this value in the apparent-density calculations, the anomaly coincides with measured noise levels within the borehole, essentially making it a non-detectable feature. This further supports the conclusion that this feature is infilled, or simply a naturally occurring deviation in the top of the B-Salt. Because the roof rock appears to be intact (although arched slightly), it appears that if infilling has occurred into a naturally-occurring solution feature, it is the result of recrystallization or limited silt/mud infilling through joints or fractures. Because such infilling would generally be expected over hundreds of thousands to millions of years, this anomaly is likely naturally occurring.

#### **Rock Mechanics Investigation Results**

A preliminary model of geotechnical rock mass characteristics has been completed to evaluate the potential instability of possible solution cavities of similar shape and size of anomalies discovered during the geophysical investigation program. The evaluation is also based on review of the historical instability of existing solution cavities in the Detroit-Windsor vicinity and on the results of a three-dimensional, distinct-element (3DEC) analysis of suspected or potential solution cavity geometry.

For this analysis, two types of cavity geometries and characteristics were considered. With the first type, in which cavities are completely contained within the respective salt layers, the salt that remains in the roof contains the internal overburden pressure and provides a stabilizing effect. The roof of salt acts as a confinement layer for the saturated brine and provides support for the salt and bedded layers above. In this case, cavities are small, and do not allow for loosening or excessive breaking of the overlying bedded deposits.

The second type of cavity features a roof in bedded deposits above the salt. In this case, the fluid pressure acts on all sides of the roof above the salt layers and does not provide stability. However, the buoyant effect of the brine does reduce the effective weight of the roof and provides a slight measure of stability. In this case, loosening and localized fallout of roof rock occur where rock is thinly bedded, shaley, or where solution cavity spans are significant to allow sag and corresponding tension in the roof rock. Roofs developing in this manner often form arched geometry and continue to propagate upwards until a bed with sufficient thickness and strength is encountered to arrest further movement.

#### Combined Geophysical Investigation and Rock Mechanics Results

Based on the observations made during the deep drilling and subsequent geophysical investigations, potential cavities were modeled with a width of 37 to 52 m (125 to 170 feet) in diameter, a height of 6 m (20 feet) and in a "morning-glory" shaped geometry. Using core data from the TB-7 and TB-11 cores and downhole geophysical data from the other borings, overlying rock bed thickness and strength were input into the model. Based on the evaluations, such geometries are likely to exhibit the characteristics mentioned above for small isolated cavities, and, in the worst case, cavities where roof fallout is stabilized by overlying bedded roof rock.

In both cases, roof spans are considered small, and analysis indicates little to no localized break or fallout of the roof. Cavities such as these would be arrested quickly as thicker bedded formations are encountered. Furthermore, if thicker bedded formations were not encountered, bulking (i.e., filling of the cavity with fallen debris) would occur, which due to the high width-toheight ratios in the localized fallout, would arrest upward movement. The height and width of the isolated cavity is small with respect to the depth of the cavity, so roof collapse would not approach (or even come near) the top of rock and would not cause subsidence or sinkholes at the surface.

The angle of draw in rock is estimated to be 15 degrees or less from the edge of large brine fields to the top of the rock surface where subsidence would occur. The angle of draw is steeper (or even negative) for cavities whose width is small with respect to the depth. The combined geophysical and rock mechanics evaluations indicate that there is no potential for significant settlement and, thus, no angle of draw for a solution cavity of the size and shape of the detected anomalies at the depth of the salt units at the X-10 and X-11 crossing corridors.

#### Conclusions

Based on the data gathered and analyzed to date, there is no evidence of cavities in either X-10 or X-11 corridors larger than Anomalies A and B shown on Figure S-6, nor evidence of potential instability of the rock mass. In fact, the analysis shows that the observed anomalies have probably been filled by one or a combination of several mechanisms. In addition, even for the largest of the anomalies located, and assuming an unfilled cavity, the analysis shows the anomaly is stable and will not progress upward any significant distance.

# 1. INTRODUCTION

The Detroit River International Crossing (DRIC) Study is a bi-national effort to complete the environmental study processes for the border crossing between Detroit, Michigan and Windsor, Ontario for the United States, Michigan, Canada and Ontario governments. The study proposes solutions that support the region, state, provincial and national economies while addressing civil national defense and homeland security needs of the busiest trade corridor between the United States and Canada (Figure 1-1).

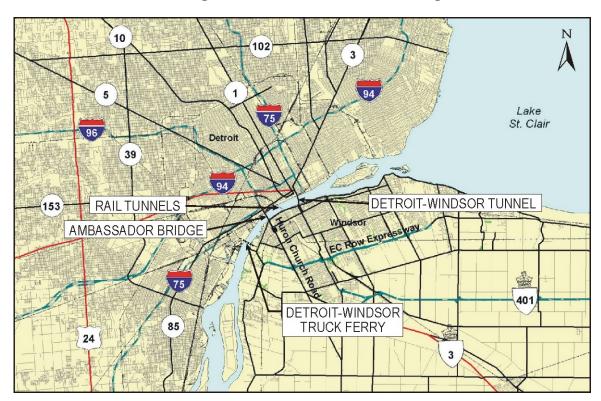


Figure 1-1 Detroit River International Crossing Study Existing Detroit River International Crossings

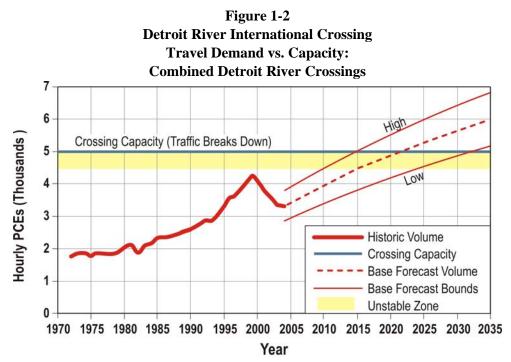
The purpose of the DRIC Project for the foreseeable future (i.e., at least 30 years) is to:

- Provide safe, efficient and secure movement of people and goods across the U.S.-Canadian border in the Detroit River area to support the economies of Michigan, Ontario, Canada and the U.S.
- Support the mobility needs of national and civil defense to protect the homeland.

To address future mobility requirements (i.e., at least 30 years) across the U.S.-Canada border, there is a need to:

- Provide new border crossing <u>capacity</u> to meet increased long-term demand;
- Improve system connectivity to enhance the seamless flow of people and goods;
- Improve operations and processing capability; and
- Provide <u>reasonable and secure crossing options</u> in the event of incidents, maintenance, congestion, or other disruptions.

Over the next 30 years, the Detroit River area cross-border passenger car traffic is forecast to increase by approximately 57 percent, with movement of trucks increasing by 128 percent. Traffic demand could exceed the "breakdown" cross-border roadway capacity as early as 2015 under high growth scenarios. Even under "low" projections of cross-border traffic, the "breakdown" roadway capacity of the existing Detroit River border crossings (bridge and tunnel combined) will be exceeded by 2033 (Figure 1-2). Additionally, the capacity of the connections and plaza operations will be exceeded in advance of capacity constraints of the roadway. Without improvements, this will result in a deterioration of operations, increased congestion and unacceptable delays to the movement of people and goods in this strategic international corridor.



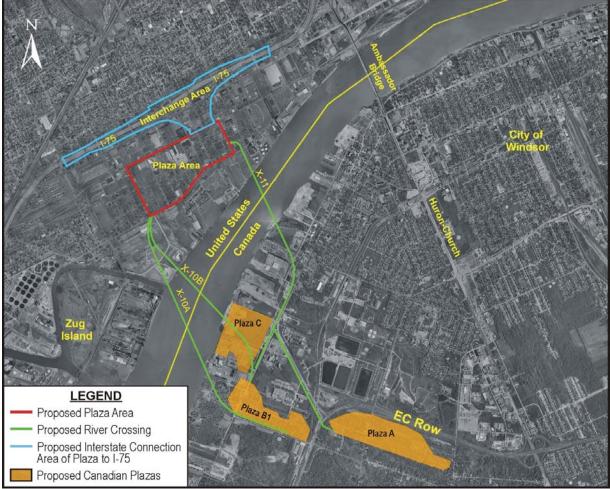
Note: Figure 1-2 is from the DRIC Travel Demand Forecast Working Paper (September 2005), prepared by the IBI Group. The Passenger Car Equivalent factor (PCE) used in that report, and in Figure 1-2, is 3.0 cars per truck. SEMCOG calculates PCEs at a rate of 2.5 cars per truck in its regional roadway system. The DEIS calculates, on the ramps, the interstate system and other roadways, PCEs at 2.5 cars per truck. Source: IBI Group

The forecast of capacity indicates that there will be inadequacies in: 1) the roads leading to the existing bridge and tunnel; 2) the ability to process vehicles through customs and immigration; and 3) the capacities (number of lanes) of the existing Ambassador Bridge and Detroit-Windsor Tunnel themselves. Therefore, even though incremental adjustments can and will be made to the plazas, and even though there is adequate border crossing capacity today (bridge and tunnel combined), the planning, design and construction of any major international crossing takes time.

Therefore, it is prudent to address, at this time, how and when the capacity needs are to be satisfied at the crossing itself, as well as the connecting roads.

The DRIC Draft Environmental Impact Statement (DEIS) analyzes issues/impacts on the U.S. side of the border of the end-to-end crossing system over the Detroit River between Detroit, Michigan, and Windsor, Ontario, Canada. The alternatives are comprised of three components: The crossing, plaza (where tolls are collected and Customs inspections take place), and interchange connecting the plaza to I-75 (Figure 1-3). Nine alternatives exist in the U.S and are listed on Table 1-1 and presented schematically in Figures 1-4 and 1-5.

Figure 1-3 Detroit River International Crossing Study U.S. Area of Analysis for Crossing System



Source: The Corradino Group of Michigan, Inc.

Alternative	Interchange	Plaza	Crossing	Proposed Status
#1	А	P-a	<b>A</b>	Analyzed in DEIS
#2	В	P-a		Analyzed in DEIS
#3	С	P-a	X-10	Analyzed in DEIS
#5	Е	P-a		Analyzed in DEIS
#14	G	P-a		Analyzed in DEIS
#16	Ι	P-a		Analyzed in DEIS
#7	А	P-c	<b>A</b>	Analyzed in DEIS
#9	В	P-c	X-11	Analyzed in DEIS
#11	С	P-c		Analyzed in DEIS

Table 1-1Detroit River International Crossing StudyCrossing Systems Included in DRIC DEIS

Source: The Corradino Group of Michigan, Inc.

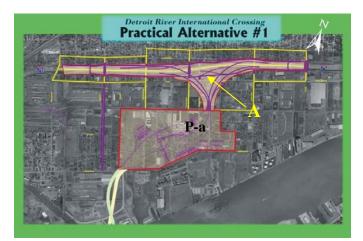
### **1.1 Purpose of the Report**

Salt (halite) has been mined historically in the form of solution mining in the area of the DRIC Practical Alternatives. As part of this solution mining process, fresh water was injected into the ground, natural salt beds were dissolved, and the resulting brine was brought to surface and evaporated to make salt. The solution mining from salt layers ranging from 275 to 490 m (900 to 1,600 feet) below the ground surface was typically conducted in an uncontrolled method before standardized record keeping was common practice. This created underground cavities of unknown location, size, and dimension. A solution mining cavity collapsed in the mid-1950s to the surface and formed a sinkhole on the Windsor side of the study area in 1954 (Figure 1-6). At least two additional sinkholes occurred at Point Hennepin (on Grosse Isle) south of the DRIC crossing site on the U.S. side of the river.

After consideration of the available data, NTH Consultants, Ltd. (NTH), a member of the DRIC consultant team, developed a Brine Well Cavity Investigation program to delineate the size, locations, and shape of potential brine well cavities in the two proposed crossing corridors on the U.S. side of the crossings. NTH proposed a combined geophysical and geotechnical program which included the drilling of multiple deep rock borings, in combination with cross-well seismic imaging. "Forward modeling," borehole gravity, vertical seismic profiling (VSP), and downhole wireline logging were also included in the program. This report presents a summary of the preliminary data obtained and preliminary evaluations preformed during the drilling operations.

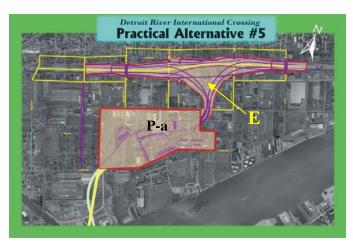
Figure 1-4 Detroit River International Crossing Study Schematic Representation of

X-10 Crossing Alternatives #1 through #3, #5, #14 and #16







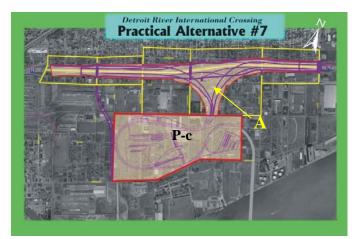


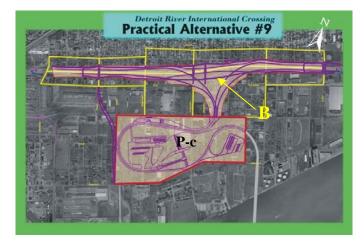


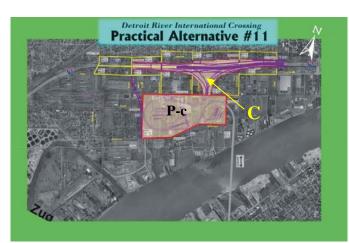


Source: The Corradino Group of Michigan, Inc. and Parsons Transportation Group

Figure 1-5 Detroit River International Crossing Study Schematic Representation of X-11 Crossing Alternatives #7, #9, #11







Source: The Corradino Group of Michigan, Inc. and Parsons Transportation Group

 Legend
 Norme

 Proposed Plaza Area
 Proposed Interstate Connection

 Proposed Interstate Connection
 Area of Plaza to 1-75

 Source: URS Canada
 Source: URS Canada

Figure 1-6 Detroit River International Crossing Study Canadian Sinkhole

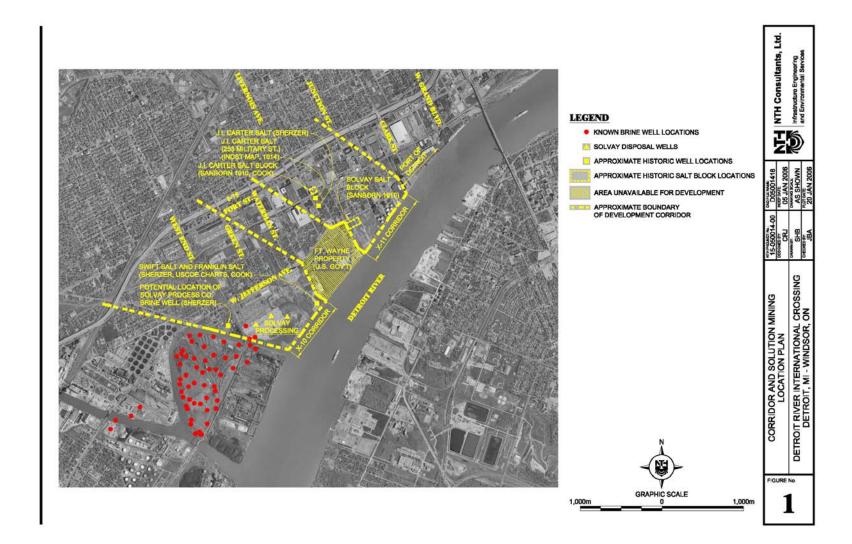
Source. OKS Canada

#### 1.2 Geotechnical Analysis Overview

In early 2006, the DRIC consultant team performed an intensive literature search involving brine well operations in the vicinity of the X-10 and X-11 corridors. The results were presented in a report entitled "Preliminary Report on Historical Solution Mining Activities, Crossing X-10 and X-11 Corridors," dated March 24, 2006 and presented under separate cover. The material derived from this report is summarized herein and a graphic from the report showing historical salt mining activities revealed during the search is presented as Figure 1-7.

After review of the report, the potential for brine well cavities to compromise the stability of the future bridge foundations in the areas under consideration was recognized. This led to preparation and funding in December 2006, of the Brine Well Cavity Investigation program noted above to investigate the locations, sizes, and geometry of potential brine well cavities in the proposed crossing location areas.

Figure 1-7 Detroit River International Crossing Study Corridor and Historic Solution Mining Location Plan



As part of the program development, two reports were prepared by Dr. Edward Cording, entitled "Stability of Caverns in Salt, Detroit Area; A Rock Mechanics Study for Bridge Foundations for Proposed Detroit River International Crossing," dated December 28, 2006; and, "Interim Report: Characteristics of Subsidence and Sinkholes in Salt, Detroit Area – A Study for the Proposed Detroit River International Crossing," dated May 27, 2006. These documents provide a background of the expected subsurface conditions, review of previous sinkhole mechanisms that have occurred over other brine well fields in the Detroit area, and recommendations for the field investigation effort. The contents of those reports are summarized herein.

A panel of international experts known as the Geotechnical Advisory Group was also assembled to review the programs plan and results. Members of the Advisory Group, listed alphabetically, are:

Jerry DiMaggio, U.S. Federal Highway Administration Chantale Doucet, Natural Resources Canada Dave Dundas, Ontario Ministry of Transport Dick Endres, Michigan Department of Transportation<sup>2</sup> Peter Gerabek, Public Works and Government Services Canada Khamis Haramy, U.S. Federal Highway Administration Dave Juntunen, Michigan Department of Transportation Tae C. Kim, Ontario Ministry of Transport Stephen McKinnon, Queens University Richard Miller, University of Kansas Pat O'Rourke, Michigan Department of Transportation<sup>2</sup> Leo Rothenburg, University of Waterloo Richard Woods, University of Michigan

The Group met five times in the Detroit-Windsor area and five times by teleconference between June 2006 and February 2008.

#### 1.3 U.S. Project Criteria

Based on criteria established by MDOT in January 2006 and further defined at the June 2006 Geotechnical Advisory Group Meeting, the proposed bridge in both corridors X-10 and X-11 requires: 1) foundations be located outside of the influence of any rock cavities that could have impact on the foundations, including those produced by solution mining activities; and, 2) foundations be built on competent bedrock. The brine well investigation program was developed and implemented to define conditions in the corridors so it could be determined if these criteria could be satisfied.

#### **1.4** Crossing Descriptions

The two crossings that were examined as part of this study are proposed to be located between the existing Ambassador Bridge and Zug Island in southwest Detroit. These crossings are described as follows:

<sup>&</sup>lt;sup>2</sup> In 2007, Dick Endres replaced Pat O'Rourke upon his retirement.

#### 1.4.1 X-10 Crossing Corridor

This corridor generally consists of the area immediately north of Zug Island to historic Fort Wayne (Figure 1-3). The area is generally flat with a slight drop in elevation at the river, with large vacated areas, parking lots, and paved/unpaved roads. Current land use includes light-to-moderate industrial areas, including a cement terminal, a major trucking terminal, a truck ferry operation, and aggregate storage areas. Residential areas exist north of Jefferson Avenue, but are generally intermingled with light commercial and industrial areas.

Historic land use includes light-to-heavy industrial areas, including a major chemical processing plant and power plant, along with two suspected salt solution mining well operations. Known solution mining wells exist adjacent to the Rouge River, along the west portion of the corridor, as well as possible undocumented solution mining wells adjacent to the current Fort Wayne property. Historic maps indicate the original shoreline of the Detroit River to be set back approximately 5 to 24 m (16 to 80 feet) from its current position, with possible docks and former boat slips prevalent throughout.

#### 1.4.2 X-11 Crossing Corridor

The X-11 corridor generally consists of the area along the banks of the Detroit River immediately east of historic Fort Wayne to just east of the existing Mistersky Power Station (Figure 1-3). The area is generally flat with a slight drop in elevation at the river, with large vacated areas between the river and Jefferson Avenue. Current land use includes light-to-moderate industrial regions, power generation facilities, and a large vacant area adjacent to the river. Residential areas exist north of Jefferson Avenue, but are generally intermingled with light commercial and industrial areas. Historic land use includes light-to-heavy industrial areas, including a major copper and brass fabrication operation, along with two suspected solution well operations.

Historic maps indicate the potential solution mining operations occurred directly to the east of a historic copper and brass fabrication facility and the eastern portion of the corridor, in what are now intermingled residential and commercial areas. Historic maps also indicate the original shoreline of the Detroit River in the X-11 corridor to be set back approximately 3.0 to 15.2 m (10 to 50 feet) from its current position, with possible docks and former boat slips prevalent throughout.

# 2. REGIONAL GEOLOGIC SUMMARY

The generalized subsurface geology for the area is summarized in this section of the report.

### 2.1 Overburden

The bedrock in the project corridors is overlain by soils (glacial drift), which have been deposited either directly by glacial ice (till), glacial meltwater streams (glaciofluvial deposits), or impounded glacial lakes (lacustrine deposits). The upper soil formations along the alignment generally consist of a relatively thick mantle of Wisconsin-aged lacustrine clays (10,000 to 50,000 years ago) that with the exception of the near-surface deposits, are typically very soft to soft in consistency. The lacustrine soils were deposited as sediments from a series of glacial lakes impounded between the ice front and the Inner Defiance Moraine located near the northwest corner of Wayne County. The upper 3 to 6 m (10 to 20 feet) of these deposits where still present have been desiccated during historical low-water periods, resulting in soils of very stiff to hard consistency near the surface. The clay soils frequently contain intermittent sand and gravel layers that were produced from glacial rivers carrying coarser sediments as lake levels fluctuated. Localized alluvial soils are present along existing rivers and streams that drain the inland areas. In some locations, lake shorelines are identified by relatively thick layers of sand and gravel.

The lacustrine deposits are typically underlain by a thin layer of highly over-consolidated glacial till, generally consisting of sand, silt, and gravel within a matrix of clay. This formation is locally termed "hardpan" and usually overlies the bedrock formation. Depending on the amount of clay binder contained in the hardpan, the material may range in nature from cohesive to granular. The hardpan is generally believed to be from the Illinoian Ice Age (200,000 years ago) and can also contain calcium carbonate producing a cemented condition. Given the glacial origins of the hardpan layer, occasional cobbles and large boulders are typically present in this layer. Methane and hydrogen sulfide may also be encountered in this layer.

The total glacial drift thickness along the X-10 and X-11 corridors varies from approximately 27 to 30 m (90 to 100 feet). The surface topography was formed during the Wisconsin stage (youngest) of Pleistocene Series glaciations of the Cenozoic Era, and has been somewhat modified by surface erosion since that time.

# 2.2 Bedrock

The proposed crossing corridor is located at the geologically-termed southeast margin of the Michigan Basin geomorphic province and within the Erie-Huron glacial lowland. The Michigan Basin is termed as such due to the structural basin shape of the bedrock, in which layers of Paleozoic era sedimentary rock that overlay the Precambrian Basement Complex, dip inwards to the center of the Lower Peninsula of Michigan from each direction as a series of bowls. The youngest layers of bedrock are located in the center of the state, with older rock layers progressing outwards to the outer margins.

The Michigan Basin was initially formed during the early Cambrian Period, when the remnants of the mountains formed during the Cambrian-Penokean Orogeny remained in a belt extending from Ontario, Canada, across the central part of the Upper Peninsula to present-day Wisconsin. The erosion of these "northern highlands" began the series of depositions and erosions that constitute the modern basin. The later effects of the Appalachian Orogeny likely caused the structural

deformation and localized downward movement in what had been a relatively stable interior continental region.

As a result, several intracratonic structural basins were formed throughout the central lowland areas of North America creating arches and domes. The Michigan Basin is bounded on the west by the Wisconsin Arch and Wisconsin Dome; on the north and northeast by the Canadian Shield; on the east and southeast by the Algonquin Arch in Ontario and the Findlay Arch in Ohio; and by the Kankakee Arch in northern Indiana and Illinois.

The Michigan Basin has undergone several periods of deposition and erosion during the Paleozoic Era, creating a complex deposition of conformable and unconformable layers of sedimentary rocks. During the early Paleozoic Era, the remnants of the Cambrian highlands began to wash into the lowlands to the north and south.

During the subsequent Ordovician Period, shallow seas covered most of the basin, except to the north where an erosional unconformity or non-deposition is noted. The shallow seas returned during the Middle Ordovician Period to the entire basin and were again fully established. Erosional sediment from the Taconic Mountains to the east during the late Ordovician Period began deposition in the deltaic regions of the basin.

The shallow seas during Ordovician Period continued into Silurian times and were beyond the reach of all but the most fine-grained clastic sediments. Due to the sediment size and transport mechanisms, the rocks deposited during this time are mostly chemical precipitates and evaporates (salts) formed in shallow clear seas. Barrier reefs formed around the margins of the basin during the Silurian when the basin was subjected to massive down-warping as evidenced by the many thousands of meters of deposition in the central portion of the basin. During the close of the Silurian, the shallow seas withdrew from the basin. During this period of erosion, or non-deposition, a gap or unconformity, is evidenced in the geologic record.

Carbonate deposits of limestone and dolomite again occurred during the Middle Epoch of the Devonian Period as the shallow salty seas returned for the remainder of the Devonian Period. Also during the Devonian Period, occasional isolation (regression and transgression of the seas) of the basin led to renewed deposition of evaporate rocks, such as salt, gypsum, and anhydrite.

During the late Devonian/Early Mississippian Periods, the uplift of the Appalachian region (later Acadian Orogeny) to the east supplied clastic sediments which were eroded from the emerging ancestral Appalachian Mountains and deposited in the basin to form shale with some sandstone and siltstone. The deposition of black mud (shale) during this time continues well into the Mississippian Period. Toward the close of the Early Mississippian time, the shallow seas regressed and caused much of the basin to become a deltaic, near shore, and beach deposition zone.

The seas returned again in the Pennsylvanian Period, where the end of the Paleozoic Era is marked by the last deposits of stream-lain sandstones as the basin was uplifted. The geologic record in the basin is then almost completely unrepresented from the end of the Pennsylvanian Period until the last stretch of the Pleistocene (Wisconsin Age), with the exception of the Jurassic "red beds" which have only been discovered in the central portion of the basin.

Based on the position of Detroit, Michigan, along the southeast rim of the Michigan Basin, the Paleozoic rocks that comprise the basin in this area typically dip to the northwest, with each formation being buried by successive younger formations in the direction of the dip. The regional dip is slight, and is estimated at approximately 6 to 10 meters per kilometer (30 to 50 feet per mile).

The topography of the bedrock surface within the area is somewhat variable and characterized by numerous irregular features in the bedrock surface. These features include many syncline and anticlinal structures believed to have developed before the Pleistocene Epoch and subsequently modified by repetitive glacial action. The bedrock features also include the existence of ancient stream valleys and possible healed faults that cut the bedrock surface. Based on historical information, the bedrock features are understood to be fairly broad, and become narrow as they reach the terminus of the Erie/Huron Lowlands.

Due to the movement of the earth's crust, these strata (especially in the Middle Devonian Lucas Formation) are seamed and fissured with vertical and horizontal joints that permit movement of ground water. Where carbon dioxide dissolved within these groundwater-filled cracks, solution cavities typically developed within the limestone and to some degree dolomite. Both the limestone and dolomite formations are known to contain dissolved sulfides, which can produce hydrogen sulfide gas upon exposure to atmospheric conditions. Hydrogen sulfide gas in the Detroit area has a history of causing nuisances and toxic conditions during tunneling operations and deep excavations, causing great bodily harm and even death to construction workers. The natural decay of organic compounds that also existed within the ancient seas became trapped within cavities formed in the limestone and dolomites and is evident today as petroleum, carbon monoxide, and methane. Small amounts of petroleum found within the limestone and dolomite tend to cause discoloring, staining, and associative odors.

#### 2.3 Groundwater

Groundwater in the X-10 and X-11 Crossing corridors can typically be distinguished according to its chemical constituency and can be sub-divided into fresh, mineral, and brine waters.

#### 2.3.1 Fresh Groundwater

Fresh water is water that is free of any deleterious, naturally-occurring chemicals or dissolved salts or solids that otherwise would make it inconsumable. Fresh water aquifers generally exist in the upper glacial drift, as well as the lower Middle Devonian rocks (Sylvania Sandstone, Bois Blanc, Garden Island, and Bass Islands Group). In the project area, the fresh water aquifer is discontinuous, and often contaminated as a result of human activities. Where the fresh water aquifer is present in the glacial drift, groundwater generally flows toward the Detroit River, which generally behaves as a regional discharge/recharge feature.

#### 2.3.2 Mineral Groundwater

Mineral water is any water that contains dissolved minerals or constituents that may alter to gas upon being exposed to the atmosphere. The dissolved compounds of interest expected for this investigation consisted of hydrogen sulfide, methane and carbon monoxide, which exist naturally in some mineral ground waters. Mineral waters are common in the lower glacial drift (hardpan) and upper Middle Devonian bedrock (Dundee, Lucas, and possible Amherstburg Formations). In the project area, mineral waters typically exhibit flowing artesian conditions when penetrated.

#### 2.3.3 Brackish Groundwater (Brine)

Brine waters are those that contain a significant amount of dissolved salts, having been formed during the historical solutioning of rocks with higher-than-normal natural salt levels (i.e. sodium, potassium, etc. ions). Brine may naturally occur in the deeper formations of the project area, including the lower portions of the Salina Group.

# 3. HISTORICAL INFORMATION

# 3.1 Salt Mining

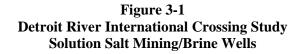
The Michigan Basin is one of the largest areas of halite (rock salt-NaCl) deposition in the world. Salt has historically been mined either directly in solid form as rock salt or as natural or artificial brine pumped through solution mining wells. The area beneath Detroit and Windsor within the Michigan Basin is currently mined using both solution mining techniques and conventional room-and-pillar excavation methods. Generally, the solution wells extended to depths of 335 to 490 m (1,100 to 1,600 feet).

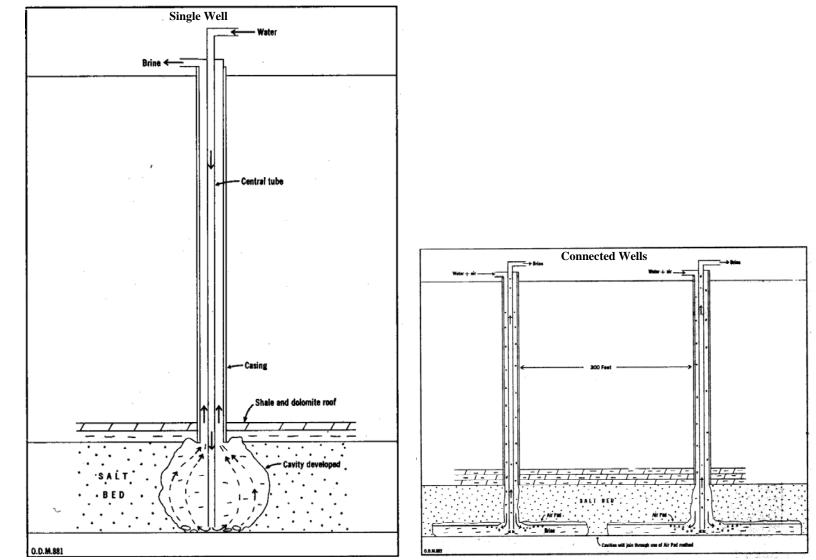
Solution mining consists of introducing fresh water from the surface down a well casing between an outer casing and a central tube (Figure 3-1). The brine produced from the salt dissolving in the water is recovered through the central tube. Cavities using this method are usually larger at the top of the stratum than at the bottom, because the fresh water tends to stratify above the denser salt brine in the cavity. The fresh water dissolves salt more rapidly near cavity roofs than at the base of the cavities which are in contact with saturated brine. This results in an inverted coneshaped or "morning glory" shaped cavity.

Solution cavities often coalesce with adjacent cavities to form composite cavities called "galleries." When this occurred historically, one or more of the wells were then converted to water inlet wells and the brine was pumped out through other wells in the interconnected system (Figure 3-1). As production continued in the gallery, large spans of unsupported roofs were sometimes created, which, in turn, could cause sagging, downward flexure, and local delamination of rock units resulting in local roof collapse and eventual surface subsidence in some instances. This surface subsidence is commonly known as a "sinkhole."

Subsidence and/or collapse often progresses upwards as a "chimney effect" on a relatively steep angle (generally 15-degrees or steeper) from the outside edges of the brinefield (Figure 3-2). Several theories have been published on the subsidence progression to the surface, the more notable of which attributes surface "daylighting" to failure of the Sandstone Sylvania Formation at a depth of approximately 152 to 183 m (500 to 600 feet). According to the theory, the sandstone disintegrates under the induced compression from rock mass sagging, and the fragments filter downwards as granular material into cavities below. This results in a cavity at a depth at approximately 152 m (500 feet) instead of the original cavity depth. This mechanism would explain why theoretical "bulking" of broken rock pieces would not be sufficient to fill the cavities before daylighting occurs.

The solution mining areas are of concern for this project, as they present the potential for future ground collapse and related adverse effects on elements of the proposed crossing structure. Additionally, at least two previous collapses have occurred in the region: at Point Hennepin near Grosse Ile, Mich., and in Windsor, Canada, in the X-10 crossing zone as shown in Figure 1-1. Significant settlements have also occurred beneath a known well field in Wyandotte, Mich. Both Grosse Ile and Wyandotte are several miles downriver from the DRIC crossing corridors. All of the known collapses have been in large interconnected brine fields (galleries).





Source: Professional Paper

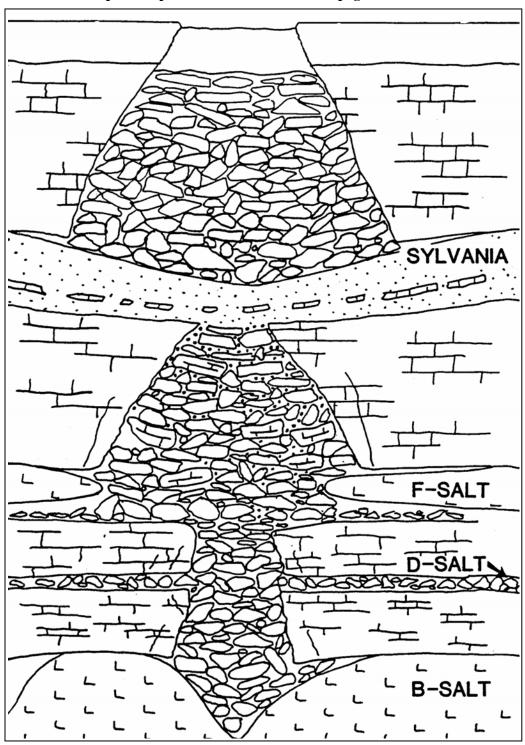


Figure 3-2 Detroit River International Crossing Study Conceptual Representation of Sinkhole Propagation to Surface

Source: Ontario Geological Survey

While no solution mining activities were identified in the U.S. bridge foundation zones in earlier DRIC work, it can be reasonably concluded that, if salt cavities exist, they were created before record keeping was standardized. Therefore, it is suspected that mining would have been of the older, uncontrolled methods and most likely before circa 1940. These early solution mines generally were of the uncontrolled type, mining salt from the F and D Units of the Salina Group, with formation tops at approximate depths of 274 to 305 m and 375 to 393 m (900 to 1,000 feet and 1,230 to 1,290 feet), respectively. Some mining may also have occurred in the lower B Unit of the Salina Group at depths of 427 to 503 m (1,400 to 1,650 feet), Specific records indicate this having occurred in the general area of crossing corridors X-10 and X-11, but mainly in the downstream locations. Nonetheless, it is not expected that mining would have occurred in the B Unit without mining in the upper F and D layers. For mining to be accomplished exclusively in the B (lower) unit, the brine well would need to be entirely cased through the D and F (upper) units, which is doubtful for that period based on historical evidence gathered for the region.

# **3.2** Solution Mining Overview

#### 3.2.1 X-10 Corridor

At least six probable brine well locations are documented within the 800 to 1,400 meter (2,500 to 4,500 feet) wide X-10 corridor. Three of these wells are documented by specific solution mining company records, which are considered reasonably reliable. At least three other brine well locations are documented through reference on historical maps and figures, which are considered somewhat less reliable. These locations are shown on the aforementioned Figure 1-7.

The three better-documented wells are within the parcel formerly owned by the Solvay Processing Company (Solvay), and are located toward the southern end of the corridor, adjacent to the existing Rouge River cut channel. The extreme southern Solvay parcel along the Rouge may be indirectly affected by the known solution mining wells that blanket Zug Island, immediately to the south.

Historic information indicates that Franklin Salt, also known as Swift Salt, performed solution mining activities within the corridor. The location of this company's solution mining wells is thought to be adjacent to the Fort Wayne Parcel on the northeastern edge of the corridor on the parcel that contained the former Edison Illuminating Company. Franklin Salt is thought to have drilled at least two solution mining wells, likely operating at a distance of approximately 100 to 120 meters (300 to 400) feet apart from one another. Information obtained after release of the "Preliminary Report on Historical Solution Mining Activities, Crossing X-10 and X-11 Corridors," dated March 24, 2006, indicates Franklin Salt drilled at least one well in 1901 and subsequently produced 72,618, 53,939, and 16,662 barrels of brine from 1902 through 1904, respectively. The same source also indicated the Edison Illuminating company had at least two wells commonly referred to as the "Edison-Ft. Wayne Wells." It is not known if the later Edison wells were simply the original Franklin-Swift wells or newly drilled wells. As the exact locations and existence of these wells are not known, the area surrounding this location (as shown on Figure 1-7) may be considered as containing at least two solution mining wells. Lithologic logs of the Franklin (Swift) solution mining wells have not been located, although historical lithology and formation references to the wells in other area well logs have been found in sources reviewed during this study. The well log commonly referred to as the Edison-Ft. Wayne #2, believed to be one of the Franklin wells has been located, subsequently evaluated, and documents bedrock lithology similar to that determined during this investigation.

Historic information indicates that prior to 1916, at least one solution mining well was located to the west of the better-documented Solvay wells, also on the Solvay parcel (near the edge of the X-10 corridor). This well is documented on only one relatively early historic map, so the location is not considered as reliable as the other documented locations on the site. Further, it is certainly possible that more than one brine well existed at this location.

Research for this project has also uncovered the existence of three previously-operated deep disposal wells on the former Solvay parcel. The wells were drilled from 1969 to 1978 to depths of greater than 1,200 meters (4,000 feet). The wells were used to inject hazardous waste into permeable rock formations (Munising Formation) deep within the ground. The wells are thought to have been plugged in 2004 based on court proceedings in which the operators of the hazardous waste injection operation were prosecuted for illegal activities. Disposal Well #2 was subsequently plugged in December 2007. Available lithologic logs do not indicate the existence of solution cavities encountered during drilling of any of these wells, one of which is apparently about 120 meters (400 feet) from a documented Solvay brine well. From this information, it appears that, at least in this location, the brine mining activities did not create cavities more than 120 meters (400 feet) from the actual brine well. However, it should also be noted that the logs of the injection wells do not contain great detail, and may not have documented small cavities that were encountered during drilling. Some loss of circulation is also noted in these logs in the upper Devonian strata, indicating that natural solutioning of the limestone and dolomites may have taken place historically (as later concurred in the drilling program).

#### 3.2.2 X-11 Corridor

At least four brine well locations and/or solution mining areas are documented within the approximately 900-meter-wide (3,000-foot-wide) X-11 corridor. All are documented through relatively older sources, which may be less reliable with respect to exact location or existence of the wells. All of the documented brine wells and/or processing areas are reported to have been located in the western half of the X-11 corridor (as shown on Figure 1-7).

Historic information indicates that both JI Carter Salt Company (Carter Salt) and Solvay may have participated in solution mining operations within the corridor. Carter Salt is thought to have operated a salt block (brine processing) plant between present day Dragoon and Military Avenues, on the north side of the existing railroad tracks. The exact numbers and locations of potential solution mining wells are not known, although various records appear to locate wells and/or brine processing in at least three locations. In any case, it is likely the wells would have been drilled to approximately 350 to 375 meters (1,100 to 1,200 feet) and located adjacent to or in close proximity to the block location.

Other early maps show Solvay maintained a salt block (brine processing) plant near present-day Campbell and Cavalry Streets, just south of Jefferson Avenue. Although exact numbers and locations of potential solution mining wells are not known, they would have likely been located adjacent to or in close proximity to the salt block location.

## 4. THE INVESTIGATION APPROACH

## 4.1 Program Concept Development

Based on the estimated bridge corridor locations, expected depth of brine well cavities, and estimated angle of cavity propagation to the bedrock surface, an investigation zone width was established. The zone was established such that, if a cavity/gallery were present immediately outside the investigation zone, it could not propagate to the surface location of the bridge foundations. Using a maximum angle for cavity propagation established through forward modeling, a cavity depth of 390 to 520 m (1,200 to 1,600 feet), and a bridge foundation width of less than 35 m (100 feet), an investigation zone width of approximately 320 m (1,000 feet) centered on the bridge foundation was established. This approach for establishing the cross-well seismic imaging spacing is presented Figure Nos. 1 and 2, in Appendix E. In addition, a corridor length extending about 640 m (2,000 feet) inland from the Detroit River edge was established.

Based on the required investigation zone and the limitations of current technology, a plan was developed to investigate the required zone for brine well cavities relying mainly on geophysical investigation methods combined with rock mechanics analysis. Physical investigation of the entire area was determined to be impractical, as boreholes to search for cavities could only be drilled at point locations, while brine wells potentially exist anywhere in the investigation zone. Therefore, several geophysical investigation methods were evaluated, with cross-well seismic imaging as the primary basis for the investigation. The cross-well method generally consists of drilling boreholes at selected locations to an engineered depth below the investigation depth/zone of interest, in the range of 480 to 560 m (1,500 to 1,750 feet) in this case. Seismic source and receivers are then installed in the boreholes and profiles are then acquired between boreholes.

The investigation was designed to search for both existing cavities and "rubbleized" zones, which indicate ongoing collapse of cavities. The issue of future potential for instability of a given cavity or rubbleized zone that might be discovered is addressed in terms of evaluating for potential propagation of existing cavities to the ground surface. The stability of the cavities is evaluated using numerical methods for rock mechanics analysis and determination of whether existing cavities have potential to propagate to the surface or result in settlement or sagging at the ground surface.

Combining this information with the cross-well seismic data to detect the existence of cavities within the zones under study confirmed whether a specific alignment will be subject to instability from the future collapse of existing salt cavities.

Solution mining activities in the area on the U.S. side ended approximately 50 years ago. Because the brine cavities are already filled with salt-saturated solution, and there is no known source of fresh water to the cavities, any salt cavities are considered very stable in terms of lateral spread. Because any future mineral mining or other man-made introduction of fresh water to this depth would require a permit from the Michigan Department of Environmental Quality or Michigan Department of Natural Resources and would be subject to public review, such activities would be prevented if a new crossing is built in the area.

## 4.2 Forward Modeling

During the early stages of this project, an extensive forward modeling program was initiated to assist in the development of the program and in particular to provide confidence with respect to

the investigation methods being used. "Forward modeling" is defined as using numerical methods to estimate physical rock mass behavior and the detection capabilities of geophysical exploration methods within the rock mass. This modeling is without the benefit of physical data from the field in this region. It is based heavily on historical geophysical data under similar geologic and urban characteristics. These evaluations were presented in the "Draft Technical Report on Comparison of Pseudo 3-Dimensional (3-D) Surface Seismic, Vertical Seismic Profiling (VSP/RVSP) and Cross-Well Reflection Tomography Geophysical Methods", dated March 21, 2006 (previously transmitted under separate cover and summarized herein). Both a Phase I and Phase II program were conducted as a part of this modeling program.

Rock mass modeling was initially performed to provide a sense of rock mass behavior above and adjacent to a solution mining cavity or series of interconnected cavities. Using historical data obtained for the Detroit/Windsor area, geometries and characteristics of regional geology and solution mine cavities were evaluated. The geometries and characteristics of the brine well cavities were evaluated for the potential angle of draw as well. The "angle of draw" is defined as the incidence angle between the edge of an underground cavern or gallery and the edge of the area on the bedrock surface where impacts from the cavity are detectible (i.e., ground subsidence). Finally, propagation characteristics given selected geometries and dimensions were modeled. The results of this investigation are summarized below.

A comprehensive geophysical forward modeling program was developed to inform the process in preparing and implementing the physical investigation and the analysis that followed with respect to three potential geophysical methods being considered. Modeled images included normal unaltered lithology, 30.5 m (100-foot) diameter brinewell cavities, and 91.4 m (300-foot) brinewell cavities. During Phase I modeling (surface seismic, vertical seismic profiling (VSP) / reverse vertical seismic profiling (RVSP), and cross-well methods), the pre-mentioned cavities were further modeled as open (containing brine) and filled with rubblized material. During the Phase II Modeling, cross-well seismic imaging techniques were modeled to produced data sets with three probable orientations/geometries, namely normal lithology, cavity directly imaged by the cross-well technique, and a cavity outside the direct image plane, but within the first Fresnel zone. A Fresnel zone is simply stated as the amount  $(1/4 \lambda)$  of constructive energy that is reflected from am impinging spherical wavefront striking an interface.

To account for the lateral resolution of each imaging method, a lateral smoothing filter was applied to both the velocity and the density model with a length of  $\frac{1}{2}$  the Fresnel diameter. The smoothing filter was applied as the synthetic seismogram generation method computes seismic reflectivity in a point-by-point manner and does not account for the fact that real-world seismic responses are the combination of responses within the Fresnel zone of the particular seismic method. The lateral smoothing filter approximates this effect by averaging the responses from model points that are within the Fresnel zone.

The relevance and importance of the Fresnel zone in the forward modeling as well as in the planning of the program is presented in Appendix J.

#### 4.2.1 Forward Modeling of Surface Seismic Techniques

Surface seismic techniques refer to the method of data gathering whereby the seismic source and receivers are located at or near the ground surface. A seismic image is produced by integrating the source-receiver arrival times over a series of data gathers. A set of logs was extracted for each offset in the 2-D velocity/density models to produce the forward modeling seismic section representative of the surface seismic method. These logs were used as input to a seismic

generation program that produces a synthetic amplitude versus angle (AVA) gather. A band-pass wavelet was then used and the resulting AVA gather was stacked over a selected range of incidence angles to produce a single seismic trace representative of that offset. Surface seismic data sets were produced using 15 to 55 Hz, which is typical for surface methods in this type and quantity of glacial till, with angle stacking equivalent to 10 to 30 degrees. The resulting section was smoothed laterally to emulate the lateral resolution of the imaging (migration) operation by applying a smoothing filter of length 0.75 X wavelength. Random noise was then added with peak amplitude equal to the peak reflectivity amplitude in the section to model real-world urban conditions.

The results of the surface seismic modeling indicated moderate-to-severe noise present within the same frequency content bands as expected during the actual survey. In the lithology-only model, select formation tops were resolvable, but clear contacts were indistinguishable. The model of the 91.4 m (300-foot) brine filled cavity was not resolvable (although a slight anomaly existed) in terms of size and geometry, but provided only slight indication that an anomaly existed in the image. The 91.4 m (300-foot) diameter rubble-filled cavity and a 30.4 m (100-foot) diameter (brine and rubble filled) cavity were not detectable in the modeled images.

#### 4.2.2 Forward Modeling for VSP/RVSP Seismic Techniques

Vertical seismic profiling (VSP) refers to the geophysical data gathering technique where the seismic source is located on the ground surface and receivers are located within a borehole. Reverse vertical seismic profiling (RVSP) refers to the method where the source is located in the borehole and the receivers are located on the ground surface. A seismic image is produced for both methods in a similar manner as surface seismic techniques by integrating the source-receiver arrival times over a series of data gathers. A set of logs was extracted for each offset in the 2-D velocity/density models to produce the forward modeling seismic section representative of the VSP/RVSP seismic methods. These logs were used as input to a seismic generation program that produces a synthetic AVA gather. A band-pass wavelet was then used and the resulting AVA gather was stacked over a selected range of incidence angles to produce a single seismic trace representative of that offset. VSP/RVSP data sets were produced using 15 to 80 Hz, with angle stacking equivalent to 10 to 45 degrees. The resulting section was smoothed laterally to emulate the lateral resolution of the imaging (migration) operation by applying a smoothing filter of length 0.75 X wavelength. Random noise was then added with peak amplitude equal to the peak reflectivity amplitude in the section to model real-world urban conditions.

The results of the VSP/RVSP forward modeling indicated moderate noise present within the same frequency content bands as expected during the actual survey. In the lithology-only model, select formation tops were resolvable, but clear contacts were indistinguishable. The model of the 91.4 m (300-foot) brine-filled cavity was not resolvable in terms of size and geometry, but provided only slight indication that an anomaly existed in the image. The 91.4 m (300-foot) diameter rubble-filled cavity and the 30.4 m (100-foot) diameter (brine and rubble filled) cavity were not detectable in the modeled images.

#### 4.2.3 Forward Modeling for the Cross-well Seismic Imaging Technique

Cross-well seismic imaging refers to the geophysical method whereby the seismic source is located in one borehole (well) and a series of receivers are located in another borehole. A seismic image is produced by integrating the source-receiver arrival times over a series of data gathers at various depths.

#### 4.2.3.1 Phase I Modeling for the Cross-well Seismic Imaging Technique

A set of logs was extracted for each offset in the 2-D velocity/density models for the Phase I forward modeling of a seismic section representative of the cross-well seismic imaging technique. These logs were used as input to a seismic generation program that produces a synthetic amplitude versus angle gather. A band-pass wavelet was used and the resulting AVA gather was stacked over a selected range of incidence angles to produce a single seismic trace representative of that offset. Cross-well data sets were produced using 100 to 2000 Hz, with angle stacking equivalent to 45 to 60 degrees. The resulting section was smoothed laterally to emulate the lateral resolution of the imaging (migration) operation by applying a smoothing filter of length 0.75 X wavelength. Because cross-well uses a downhole sensor and a frequency band above 100 Hz, near-surface attenuation was determined to remove most of the surface noise from the cross-well seismic records. Noise was then added to produce a post-stack signal-to-noise ratio (SNR) in the cross-well data of 20 dB relative to the peak reflectivity amplitude.

The results of the cross-well seismic imaging modeling indicated that the modeled anomalies identified during Phase I were detectable and resolvable (higher frequency equals greater resolution). Additionally, the slight bump on the top of the B-Salt was also resolvable in the images (specific to cross-well imaging modeling), as B-Salt unconformity is known in the region.

#### 4.2.3.2 Phase II Modeling for the Cross-well Seismic Imaging Technique

Phase II forward modeling of a cavity and/or absence of a cavity was performed in two dimensions (2-D), relevant to field data acquisition procedures. A 2-D finite difference model with 5 or more model cells per wavelength to avoid dispersion affects was created. For 2 kHz data at 18,000 feet per second, a 2.7 m (9 foot) wavelength was anticipated in the DRIC project corridor, with each cell approximately 0.61 m by 0.61 m (2 feet by 2 feet). For the cross-well program, with borings spaced approximately 305 m (1,000 feet) apart and about 60 percent padding to avoid edge effects, an 800 by 800 cell model was anticipated. A cross-well data set was then produced and processed to produce velocity/reflection images for use in data acquisition.

The second portion of the Phase II forward modeling program consisted of a 3-D model of a cavity outside the image plane but within the first Fresnel zone in the cross-well seismic imaging profile. The number of cells was limited to reduce the size of the model in the 3<sup>rd</sup> dimension, both by reducing the upper frequency below 2kHz and allowing greater dispersion of the data. Several common shot gathers from the 3-D model were produced and inspected by hand because the computer resources to generate a complete cross-well data set in a reasonable timeframe were not feasible. The results indicated a cavity could be resolved if acquired outside the direct image plane, but within the first Fresnel zone as indicated.

#### 4.2.4 Rock Mechanics Forward Modeling

The rock mechanics task was performed largely through the work of Dr. Edward Cording. A forward modeling program was created to yield useful data that advised the overall field investigation program through knowledge of the project and subsurface conditions. The desired outcome of the rock mechanics forward modeling effort was to predict the size, geometry, and location of brine well cavities of concern for our project (i.e., what size cavity might potentially propagate to, or have impact on, the bearing surface for the bridge foundations). In addition, the rock mechanics forward modeling was to provide insight into an appropriate angel of draw, for the purposes of spacing the test borings to provide for an appropriate offset from the cleared corridor (see Section 4.1, above). The field effort and/or cross-well seismic program was then adjusted, as appropriate, in consideration of the target void of concern as determined both through

the forward modeling process (through peer discussions and discussions between the project team, MDOT, and other parties also as appropriate) and early field results.

The proposed modeling plan consisted of evaluating the rock stability above solution-mined cavities using existing data and assumed rock mass-over-void conditions, similar to those known for coal mining. Conclusions and evaluations were based on parametric analysis and logic-based conclusions. The next portion included computer analysis that advanced the work using 2-D finite element analysis for more refined conclusions. Once suspected geometries and lithology were determined, a 3-D model was produced using the computer program 3DEC, which produces a computational model for rock fractures, movements, etc. The analysis was then calibrated with known stable and unstable conditions for the brine well cavities in the Detroit area. Later, the model was also calibrated with respect to feature geometry observed during the cross-well investigation.

# **4.2.4.1** Rock Mechanics Modeling Phase I: Characteristics of Subsidence and Sinkholes in Salt, Detroit Area

During the initial modeling phase, a report entitled "Interim Report: Characteristics of Subsidence and Sinkholes in Salt, Detroit Area – A Study for the Proposed Detroit River International Crossing," dated May 27, 2006 and presented under separate cover, was produced by Dr. Edward Cording to evaluate rock stability above solution mined cavities using existing historical and lithologic data. The characteristics of subsidence and sinkholes above solution mined cavities and room-and-pillar mines in salt deposits in the Detroit Area were reviewed, with references made to experience with tunnels in soil, room-and-pillar mines in coal and other bedded rocks, and long-wall mines in coal. The primary focus of this report was to evaluate the lateral extent of the near-surface subsidence and sinkhole effects with respect to the locations of the wells, cavities, and brine fields. The gathered information was intended to aid in the development of guidelines for bridge foundation elements and establishing safe stand-off distances from known or suspected solution mining cavities in the salt. Specifically, the modeling predicted that for larger caverns or brine well galleries, an "angle of draw" of approximately 15 to 20 degrees would be appropriate for use in planning the investigation.

#### 4.2.4.2 Rock Mechanics Modeling Phase II: Stability of Cavities in Salt, Detroit Area

During the second phase of rock mechanics modeling, a report entitled "Stability of Caverns in Salt, Detroit Area; A Rock Mechanics Study for Bridge Foundations for Proposed Detroit River International Crossing," dated December 28, 2006 and presented under separate cover, was produced by Dr. Edward Cording to focus on the stability of cavities in salt and the size of stable cavities extending into overlying bedded deposits. This report commented on the stability of the roof of individual solution cavities and the condition which would allow large roof falls to initiate and progress upward without being arrested by overlying thicker beds. The evaluation was based on the review of existing solution cavities in the Detroit-Windsor area and analysis of cavity stability conducted using three-dimensional (3-D) distinct element methods. The results of the modeling indicated that the cross-well seismic imaging technique would be applicable for detecting cavities that are much smaller than any existing cavities that would be potentially unstable.

## 4.3 Geotechnical Advisory Group

The Geotechnical Advisory Group (Group) met five times in the Detroit-Windsor area and five times by teleconference between June 2006 and February 2008.

The Group was formed and initially met over a span of three days in late June 2006 to discuss the planned investigations and work that had been completed to that point. Topics discussed included an overview of the project, the purpose and mission of the group, status of the investigation thus far including a history of brine wells in the Detroit-Windsor area, description and objectives of the geotechnical program, planning and development of the program (including consideration of risk), decision making protocol for the investigation, and schedule. The Group also discussed the required criteria for placement of bridge foundations in the subject corridors, including size of voids of concern, angle of draw, the concept of void mitigation, and relative certainty of the void detection program. The Group also took a tour of the project alignments in Windsor and Detroit.

In consideration of the risk of potentially being unable to detect a cavity within certain "shadow areas" based on the cross-well investigation alone, vertical seismic profiling and borehole gravity surveys were recommended by the Group and were later formally added to the program. These additional measures were intended to provide independent methods to confirm the findings based on the cross-well investigation, as well as to provide a greater level of detection in the vicinity of the primary foundation elements for the proposed bridge alignments.

The Group met in Windsor in March 2007 to discuss results-to-date from the ongoing investigation. Both the U.S. and Canadian DRIC Teams presented a summary of progress and results with respect to the drilling and data gathering. The meeting ended with discussion of project goals and schedule.

The Group also met in Windsor in December 2007 to discuss the preliminary results and conclusions from the completed investigation. Both the U.S. and Canadian DRIC Teams presented a summary of investigation, results, conclusions, and recommendations with respect to the drilling and data gathering. The meeting ended with the group addressing a set of questions derived from the results presented by both DRIC Teams.

The Group also teleconferenced five times during the drilling and data acquisition processes, to discuss progress, and to provide insight on proposed changes to the program that were initiated because of encountered field conditions. These changes are discussed in subsequent sections of this report.

#### 4.3.1 Decision Protocol

A decision-making protocol was developed at the first Group meeting. The protocol was refined and presented in a memorandum entitled "Decision Process in Brine Well Cavity Program", as summarized herein. The memorandum presented a flow chart to define the decision criteria concentrating on the drilling, initial geophysics, and final geophysics analysis, with the day-today technical decisions being made by the respective DRIC teams on each side of the border.

### 4.4 Final Program Details

Development of the final brine cavity investigation program was an iterative process that involved research, forward modeling, consultation with various experts (including the Group), and practical issues such as site access, right of entry, etc. Based on these efforts, the final program was developed with boreholes to be generally 480 m (1,500 feet) deep, with selected holes to be as much as about 560 m (1,750 feet) deep. Imaging coverage, resolution, angle of draw, and signal-to-noise ratio were considered in determining the intended boring spacing and depth, as well as proposed piezoelectric source and frequency sweep selection. The final program was bolstered based on specific team member experience in similar deeper rock conditions at the Michigan Technological University test site, using the same equipment as proposed for this investigation. At that site, a higher frequency was used (3khz) with borehole spacing of 2,000 feet. A conservatively lower frequency was proposed for this investigation, for the purpose of allowing an increase in the allowable borehole spacing, if actual site conditions required such.

Figure 4-1 Detroit River International Crossing Study Final Test Boring Location Plan – Site X-10



Source: NTH Consultants, Ltd.

Figure 4-2 Detroit River International Crossing Study Final Test Boring Location Plan – Site X-11



Source: NTH Consultants, Ltd.

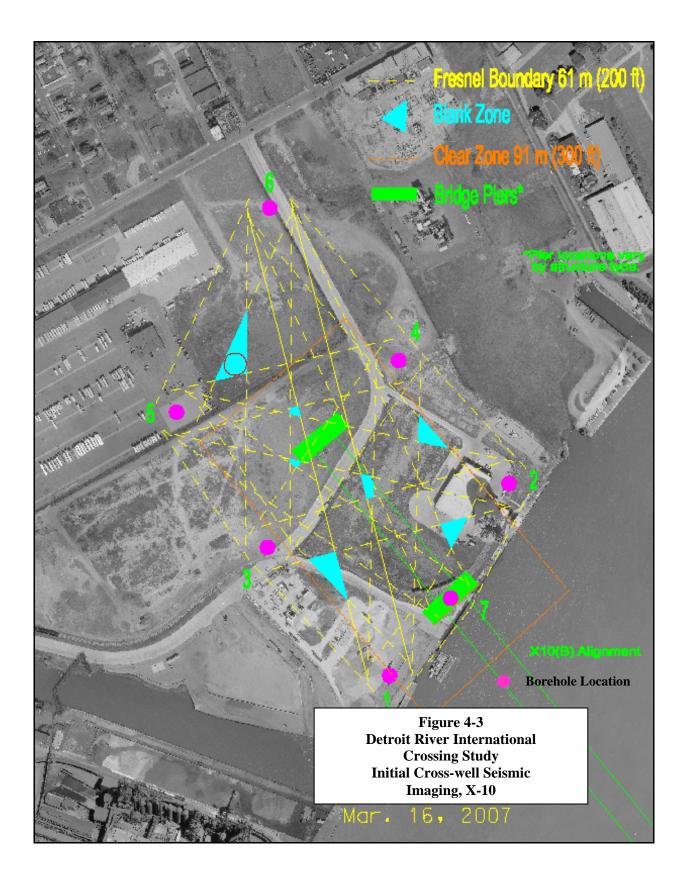
The final borehole configurations for each crossing are shown on Figures 4-1 and 4-2, which include six boreholes for the X-10 crossing corridor and seven boreholes in the X-11 crossing corridor. The program included gyroscopic deviation surveys within each borehole, to be performed before the cross-well profiles were imaged. The logs were run to establish a very accurate borehole azimuth and dip. The final investigation program included two boreholes on each crossing to be cored, at least partially. The remaining boreholes were rotary drilled and logged based on drill cuttings, combined with an extensive suite of downhole geophysical logging.

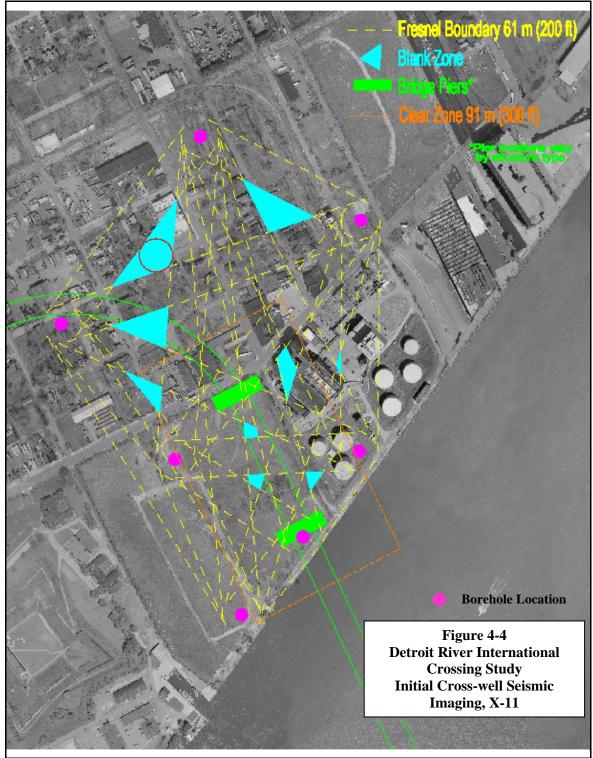
The proposed investigation was planned with an understanding of the practical limitations on the size and resolution of solution cavities or other natural cavities in the investigation zone, as only the area directly between the boreholes was to be investigated by a cross-well profile. Based on the forward modeling effort, the profiles were initially estimated (for program planning purposes) to have an effective width (Fresnel zone) of approximately 50 to 65 m (150 to 200 feet) at 1,000Hz, essentially thinning as the spectral frequency increases. Based on later analysis, using the actual data acquired in the field, the Fresnel zone was estimated to be about 65 to 75 m (200 to 225 feet), creating an investigation zone of at least about 30 m (100 feet) on each side of the profile.

Because the profiles can only detect cavities within a given Fresnel zone width (assumed to be about 50 m (150 feet) during the initial planning), the probability of missing a given cavity was examined in terms of cavity diameter (assumed circular) for a constant Fresnel width, and ignoring "thinning" effects of the Fresnel zone in close proximity to the boreholes. Given a random placement of that circle within the study area, the probability of any part of that circle being intersected by one of the 2-D profiles was examined. The problem can be simplified by recognizing that the centerpoint of any circle that would be missed by the cross-well profiles would be at least one radius away from the edge of the 50 m (150 foot) effective width of the cross-well profile. The issue is further simplified in terms of a ratio of the total area within the perimeter of the interconnected boreholes to the area of a swath of width equal to one diameter plus 50 m (150 feet) along each profile.

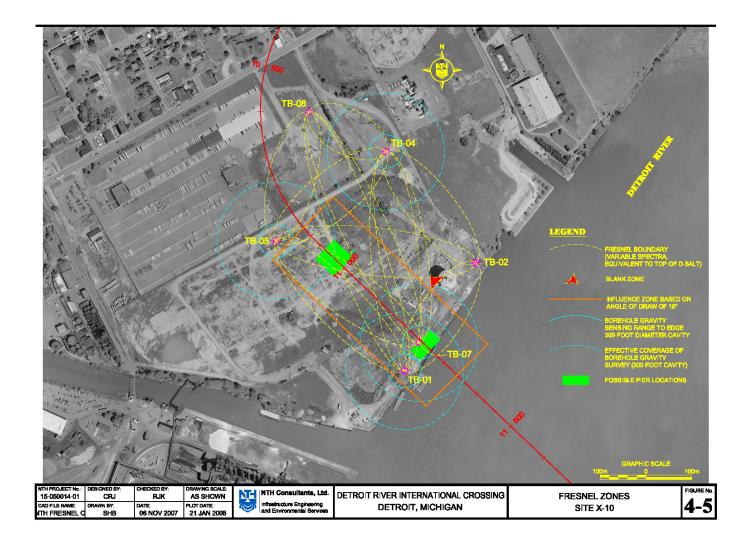
Based on the boring configuration and cross-well profile width of about 65 m (200 feet), the effective cavity size that could be missed by the investigation for the X-10 crossing was determined to be about 30 m (98 feet), with the coverage of the clear zone estimated at about 97.6 percent (Figure 4-3). Likewise, the effective cavity size that could be possibly missed by the investigation for the X-11 crossing was determined to be about 55 m (170 feet), although this large shadow zone was located outside the zone that could impact the proposed bridge foundations. Within the influence zone (determined based on angle of draw and depicted in Figures 4-3 and 4-4 by an orange dashed line), the effective cavity size that could be possibly missed by the investigation for the X-11 crossing was determined to be about 28 meters (90 feet), with the coverage of the clear zone estimated at about 94.9 percent.

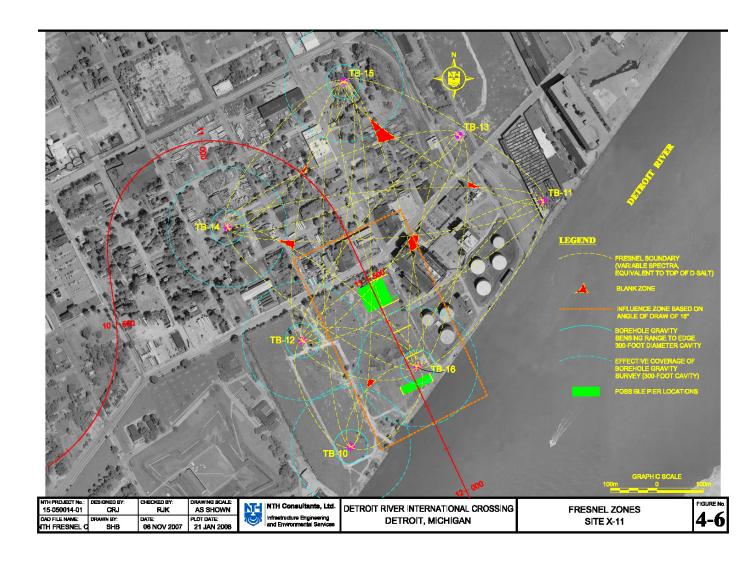
Final cross-well and gravity coverage based on actual spectra obtained during the investigation are presented as Figures 4-5 and 4-6. It is observed that near complete coverage of both crossing corridors was obtained. The cross-well fresnel zone widths are representative at the top of the D-Salt, where the wavelengths are the longest (approximately 100 feet at 200 Hz). The source and receiver depths are approximately 335 m (1,100 feet). The gravity detection limits are representative of the radius detection zone around a borehole to the edge of a potential cavity. In this case, a 91.4 m (300 foot) diameter cavity was used, with a detection zone away from the borehole of approximately 37 m (120 feet). This provides a theoretical detection radius of approximately 128 m (420 feet) from the borehole.





Source: NTH Consultants, Ltd.





## 5. FIELD INVESTIGATION

The field investigation was conducted beginning February 1, 2007, and continued 7 days per week, 24 hours per day until completed in early April 2007 (drilling). The subsequent geophysical investigation (cross-well seismic, BHGM, and VSP surveys) were also conducted 24/7, and were completed in early August 2007. As the field investigation program was implemented, the actual individual property conditions and restrictions were integrated with the program, resulting in an evolution of the actual borehole coverage and depth, boring configurations, drilling procedures, etc. These issues are discussed in the following section of the report.

## 5.1 Site Conditions

Site conditions and borehole locations as they evolved into the final program for both crossings are summarized as follows.

#### 5.1.1 X-10 Corridor

The investigation sites for the X-10 crossing corridors may be generally characterized as former industrial lots, with the exception of boreholes TB-1, TB-2 and TB-7, which were drilled in current industrial areas. The locations of the boreholes were typically on vacant land.

The original X-10 borings (TB-1, TB-2, TB-3, TB-4, TB-5, TB-6 and TB-7) were located on the former Detroit Coke site (Figure 4-1).

The former Detroit Coke property, used for coke oven and coke oven gas by-products operations from early 1900 until 1991, has been the subject of environmental investigations performed by others not related to this project. Due to the presence of regulated deep-underground injection wells on the property, it was identified as a Resource Conservation and Recovery Act (RCRA) facility. Associated environmental impacts with the coke oven and coke oven gas by-products operations included tar, free-phase hydrocarbons (free product), and soil and groundwater contamination. The entire Detroit Coke site has been reportedly impacted by former industrial operations.

Site soils are reportedly contaminated with volatile organic compounds (VOCs), semi-volatile organic compounds (SVOCs), ammonia, cyanide and metals at concentrations exceeding the MDEQ industrial criteria for indoor and ambient air, direct contact, particulate inhalation and surface water protection. Likewise, site groundwater is reportedly contaminated with VOCs, SVOCs, ammonia, cyanide and metals at concentrations exceeding the MDEQ industrial criteria for indoor are and metals at concentrations exceeding the MDEQ industrial criteria for indoor and metals at concentrations exceeding the MDEQ industrial criteria for indoor air, direct contact and surface water protection.

Honeywell, the primary responsible party, has installed a demarcation membrane in certain areas, and approximately 15.2 to 30.5 cm (6 to 12 inches) of clean fill material has been placed over the membrane to prevent contact with the impacted soil. However, this membrane and clean fill layer may not be present throughout the entire site. Honeywell has also installed groundwater collection trenches and an associated force main that runs parallel to West Jefferson Avenue to limit impacted groundwater from discharging to the Rouge and Detroit Rivers. Section 5.3 of this report explains special measures taken at this site.

Boring TB-1 was drilled on what is now the McCoig Aggregate property located at 1441 Springwells Court along the Detroit River northeast of its confluence with the Rouge River. The McCoig property is currently used for receiving, storing, and shipping aggregate for concrete production. The work area was located adjacent to aggregate stockpiles on the property, with the final borehole location shifted about 45 meters (150 feet) from its originally planned location, dictated by operational considerations within the property itself.

Boreholes TB-2 and TB-7 were located on the Lafarge North America Detroit Cement Terminal facility located at 1301 Springwells Court along the Detroit River northeast of the McCoig Aggregate facility. The Lafarge property consists primarily of open land with a storage silo and related structures adjacent to the Detroit River. Similar to the McCoig property, the boring locations for TB-2 and TB-7 were adjusted up to about 30 meters (100 feet) from the originally planned locations to accommodate existing infrastructure and operational considerations at the Lafarge facility.

Boring TB-6 was drilled on the Yellow Transportation Detroit Terminal property located at 7701 West Jefferson, and was also adjusted to allow for minimal disruption of Yellow's facility. The remaining X-10 boreholes, TB-4 and TB-5, were drilled on property that is currently vacant land owned by the Detroit Economic Growth Corporation (DEGC). Due to the various modifications to the locations of borings TB-5 and TB-3,<sup>3</sup> and in consideration of the existing railroad tracks, overhead power lines, and underground utilities on the DEGC property, the locations of these two borings had incrementally changed such that the final locations were less than 90 m (300 feet) from each other. At this spacing, the relative value of drilling TB-3 was reduced such that it provided very little in terms of coverage within the overall program. After consideration by the Geotechnical Advisory Group, boring TB-3 was eliminated from the program.

#### 5.1.2 X-11 Corridor

The X-11 borings (TB-10, TB-11, TB-12, TB-13, TB-14, TB-15 and TB-16) were drilled both north and south of Jefferson Avenue between South McKinstry Street and Cavalry Street (Figure 4-2).

Test borings TB-10, TB-12, and TB-16 were drilled on the former Revere Copper and Brass site located at 5851 West Jefferson Avenue. The Revere site exists as vacant, unoccupied land with some lightly wooded areas south of West Jefferson, to the southeast of the Mistersky Power Plant. Previous environmental investigations conducted by others not related to this project have identified the Revere site as impacted with the byproducts of former industrial use.

In addition to the environmental issues, a Detroit Water and Sewerage Department (DWSD) triple-barrel outfall sewer runs from a 4.9 m (16-foot) diameter interceptor sewer under Jefferson Avenue to the Detroit River, extending through the center of the Revere property. The outfall sewer in this section is approximately 1.8 m (6 feet) tall, 5.5 m (18 feet) wide, 366 m (1,200 feet) long, at an average depth of approximately 3 m (10 feet) below ground. Piling supports the last third of the outfall before the river. The current easement alignment, together with the size and shallow depth of the outfall, limit potential development opportunities for the site.

Test borings TB-11 and TB-13 were drilled on parcels currently owned by Renaissance Logistics/Motor City Building Materials. These sites are located at 4738 West Jefferson, northeast of the Mistersky Power Plant. The work areas are vacant, former industrial property.

<sup>&</sup>lt;sup>3</sup> Identified when the program was initially laid out.

Borehole TB-14 was drilled on City of Detroit properties at 5851, 5857, 5861, and 5867 Harvey Street, between Campbell and Cavalry Streets. The area is characterized as vacant residential property in a mixed residential and industrial neighborhood. These properties are generally vacant and unoccupied with scarce vegetation.

Test boring TB-15 was drilled at 125 Junction Street on property owned by PVS Chemicals. The PVS site is vacant with several building foundations evident on parts of the site.

## 5.2 Site Preparation Activities

The existing conditions at each drilling location were modified to accommodate drilling equipment. A level pad was built at each boring location, using crushed stone to create a stable platform on which to place the drill rigs and support equipment. Existing conditions were protected to the degree possible at each site in order to facilitate site restoration at the completion of the DRIC Brine Well Investigation Program.

#### 5.2.1 MDEQ Permitting

To conduct the geotechnical analysis, the Michigan Department of Environmental Quality (MDEQ) required a Natural Resources and Environmental Protection Act, Act 451, Part 625 Mineral Wells permit for each of the boring locations. The borings were permitted as test wells with stringent regulations for site preparation, drilling methods, fluid management, casing, well abandonment, restoration, and a hydrogen sulfide contingency plan, which will be discussed in later sections of this report. The MDEQ also required the permit locations to be bonded until completion of abandonment and restoration activities.

#### 5.2.2 Right of Entry

Drilling operations were conducted on sites owned by six separate entities. Because none of the sites were under MDOT ownership, the DRIC team was required to secure Right-of-Entry agreements prior to starting the work. Agreements were reached with each landowner to allow time for drilling operations, downhole testing, borehole abandonment, and site restoration. The agreements were made with specific requirements as detailed by each of the individual owners.

#### **5.2.2.1** Communications Planning

A plan was developed to document communications protocol for the field activities of the DRIC Brine Well Investigation Program. The procedures outlined in the protocol governed all communications among the project team, governmental agencies, the media, and the public during the time at which field activities were conducted. The protocol addressed the communications guidelines that would be used for everyday communications through emergency situations and a possible evacuation of workers and the general public. The plan covered topics such as individual personnel responsibilities, site access protocol (including site limits, safety requirements, and project orientation), media and public relations, and emergency procedures. The protocol is subdivided into several sections as discussed below.

**Hydrogen Sulfide Contingency Plan** – Based on historical information, the lower glacial drift and bedrock to be investigated, beginning with the Illionoian "hardpan" and Dundee Limestone, respectively, at approximately 29.0 to 30.5 meters (90 to 100 feet) below ground surface, is known to contain hydrogen sulfide dissolved in groundwater. It was considered possible that significant hydrogen sulfide could have been encountered and released to the atmosphere during drilling operations. As such, the borings were subject to Michigan's Oil and Gas Regulations, Natural Resources and Environmental Protection Act No. 451 of the Public Acts of 1994, Rules 324.1101 through 324.1130. These rules required preparation of a Hydrogen Sulfide Contingency Plan that provided a "plan of action for alerting and protecting personnel at the well site and the public in the event of an emergency involving release of hydrogen sulfide gas." This contingency plan was in place until the drilling program was completed. It was not necessary to enact the contingency plan, as no releases of  $H_2S$  covered by the plan were observed.

**Project Safety Training and Orientation** – This plan and document were prepared and utilized during the field investigation to provide a summary of the procedures and content of the DRIC Project Safety Orientation Program, which was established and utilized during the deep drilling program. Site personnel and visitors were required to take this orientation prior to entering the drilling sites. Topics covered during this orientation included site access and security, health and safety plans, personal protective equipment, hydrogen sulfide management, first aid, housekeeping, illumination, drilling safety, fall protection, flammable hazards, hot work permits, confined space entry, excavations, lock out/tag out, hazard communication, weekly safety briefings, and accident reporting.

Fifteen safety orientation training sessions at multiple sites were conducted involving 300 people with the Detroit Fire Department, Police Department, Department of Homeland Security, Department of Environmental Affairs, Mistersky Power Plant, and other City of Detroit agencies. The Mistersky Power Station Emergency Procedures checklist was included in the communications protocol to familiarize project personnel and first responders with the evacuation procedures of the adjacent Mistersky Power Station.

**Evacuation Checklist** – An evacuation checklist for hydrogen sulfide exposure was produced to streamline the decision making process should an accidental release be detected during the drilling. The checklist provided communications protocol and evacuation guidelines for actions to be taken given specific hydrogen sulfide concentrations.

#### 5.2.2.2 SCBA Training for City of Detroit

Special equipment and training was provided as follows: 1) 8-hour SCBA respirator training for 28 Mistersky Power Plant personnel in two sessions; 2) SCBA fit tests and medical evaluations of all Mistersky Power Plant personnel; 3) nine self-contained breathing units, together with associated tanks and other equipment and a detailed respirator protection program; 4) two gas meters and training for Mistersky personnel; and 5) three gas meters and training to Fire Department personnel.

#### 5.2.2.3 Evacuation Planning

An evacuation plan for the surrounding area was prepared. An automated telephone evacuation system was at the ready throughout the drilling operation. The consultant and multiple City of Detroit and public agencies were required to gain approval of the plan from the Detroit Fire Department, Police Department, Homeland Security, Department of Environmental Affairs, Emergency Dispatch, hospital representatives, and the Michigan Humane Society. A "mock disaster" drill was conducted where potential real-world situations were addressed. Evacuation plans for site personnel, local business personnel, residents, and pets (cats and dogs) were adopted.

#### 5.2.2.4 Special Insurance Requirements for Right-of-Entry

In order to obtain right of entry on selected parcels, the team was required to have the driller, Advanced Energy Services, obtain \$10 million of pollution liability, additional contractor's general liability, and control-of-well insurance for the project.

## 5.3 Additional Protocol for Investigation Program

A number of protocols were developed and implemented as part of the field investigation program, including environmental protocols for drilling, noise monitoring and control, vibration monitoring and control, and environmental contingency plans.

#### 5.3.1 Environmental Protocol for Drilling

A specific plan detailing the potential contaminants present at the site and work procedures for limiting exposure and potential for exacerbation of contaminants was prepared for each drilling location. Each worker was required to read, sign, and abide by the overall health and safety plan.

#### **5.3.1.1 Detroit Coke Site (TB-1 through TB-7)**

As mentioned previously, environmental contamination had been previously identified at the former Detroit Coke site boring locations (X-10 Crossing). Based on the existing conditions, it was necessary to prevent migration of contaminated near-surface groundwater through the boreholes into uncontaminated aquifers and exposure of clean soils to contaminated near-surface soils displaced during drilling. To address the near-surface contamination, an additional environmental casing was employed. Drilling equipment was steam cleaned and the environmental casing was cleaned prior to advancing the boring.

Borings on the Detroit Coke site resulted in contaminated waste generated during environmental casing installation. Cuttings and drilling fluid from these borings were placed in half-tanks on site and solidified. The solidified waste was then shipped to a Class II landfill for disposal. Liquid generated during environmental casing cleaning was pumped into a frac tank at the site and disposed at both hazardous and non-hazardous liquid disposal facilities, as appropriate. Waste generated from installing the conductor casing, which will be described in following sections, was solidified in a half-tank and shipped to a Class II landfill for disposal.

#### 5.3.1.2 Crossing X-11 City of Detroit Parcels (TB-10, 12, 14, and 16)

Environmental contamination had been previously identified on the City of Detroit property at Crossing X-11. As part of the environmental protocol for these borings, a single conductor casing was installed to bedrock, in the manner discussed above for the Detroit Coke property. Soil cuttings and drilling fluid were also placed in a half-tank on site and solidified. The material was then shipped to a Class II landfill.

#### 5.3.1.3 Crossing X-11 Private Owners (TB-11, 13, and 15)

Environmental contamination had not been identified on the X-11 crossing private properties (TB-11, TB-13, and TB-15), and near-surface fill soils on these properties were not characterized. Borings on these properties were drilled to a depth of 1.5 meters (5 feet) into the native clay layer

as an initial step. Drilling fluid and cuttings were then replaced, with the initial material pumped to a pit on site, which was then stabilized and capped. Drilling resumed with fresh drilling fluid to a depth of 1.5 meters (5 feet) into bedrock prior to installing the conductor casing. Fluid and cuttings from this second interval were placed in a half-tank and stabilized prior to shipment to a Class II landfill.

#### 5.3.2 Noise Monitoring and Control

Requirements to control sound levels were implemented during drilling. The drillers installed hospital-type sound mufflers on the drilling rig and generators to limit sound. Noise threshold levels were established per MDOT specifications. Sound monitoring was conducted during the field operations by measuring the sound intensity at various intervals around the drilling site. If values over 66 dBa were detected for more than ten minutes, the readings were then taken adjacent to the nearest residence. During the investigation, sound levels exceeding 66 dBa were not recorded adjacent to any structures used for business or residential purposes.

#### 5.3.3 Vibration Monitoring and Control

An assessment of vibration levels at nearby structures was performed during the field investigation by monitoring vibration levels at the outer limits of each drilling site. BlastMates II and III seismographs were used during the drilling to monitor vibration levels on a per shift basis. The setup consisted of geophones placed on the ground surface and connected to a microprocessor within the seismograph. The devices recorded peak particle velocity and velocity time history, with a threshold value set at 2.54 cm/s (1.0 in/s). Vibration levels recorded during the investigation did not exceed the threshold value at the confines of the site.

A structure condition survey was incorporated into the field investigation procedure, should threshold values have been exceeded, as a tool for mitigating and resolving potential claims. Structure condition surveys were not performed during the field investigation because the vibration threshold was not exceeded.

#### 5.3.4 Environmental Contingency Plan and Procedures

As a byproduct of the drilling activities, drilling fluids were produced with a potentially high pH (above 12), which would technically classified them as liquid industrial waste. A high pH was maintained during drilling to inhibit dissolved hydrogen sulfide gas present in the groundwater from becoming volatilized upon being exposed to the atmosphere. Testing was conducted during drilling activities to monitor the pH. Specific procedures were developed to minimize the potential for generation of hazardous waste and are discussed as follows:

#### 5.3.4.1 Training Requirements

At least one person was on site who had received training equivalent to 40 CFR 265.16 for the duration of drilling activities that could potentially produce hazardous waste. The training was provided to ensure that hazardous waste generator requirements, including personnel training, and contingency plan and emergency procedures were in place before commencement of field activities, in case of hazardous waste production. Records were kept onsite for verification.

#### 5.3.4.2 Preparedness and Prevention

The drill site was maintained in such a manner that there were no releases of hazardous waste to the soil or surface water. This was completed by directly transferring waste from boreholes to tanker trucks for transportation to an approved disposal facility. Communication between the shift supervisor and the project team was maintained by cellular telephone. Detroit Police and Fire Department personnel were briefed on the operations numerous times and were aware of the activities. As previously mentioned, staff from the police and fire department received project safety orientation prior to beginning field activities. Security was also maintained at the drilling sites at all times.

#### 5.3.4.3 Contingency Plan

A Contingency Plan was created to address the potential release of hazardous waste as a result of the drilling process. It complemented the existing Hydrogen Sulfide Contingency Plan, as previously discussed. An emergency coordinator (and alternates) were selected who were familiar with the operation and had received the proper training. An emergency responder was also contracted in the event an unattended release were to happen. Emergency procedures were developed that specifically detailed the steps to be taken in the event of a release, with respect to notification protocol, identification, assessment, reporting, containment, monitoring, clean-up, ensurance that equipment and procedure are online before resuming activities, and submittal of documentation to the Environmental Protection Agency.

## 6. DRILLING PROGRAM

The drilling program for the DRIC Brine Well Cavity Investigation Program consisted of advancing thirteen test borings from the ground surface to total depths (TD) ranging from 457.2 to 533.4 meters (1,500 to 1,750 feet) below ground surface. The borings were advanced using a combination of rotary drilling and coring methods.

As borehole drilling was subject to Michigan Department of Environmental Quality permitting rules cited previously, the boreholes were designed for each stage of drilling: 1) borehole diameters to be used to certain depths; 2) casing requirements for each section of the boring; 3) casing cementing requirements; and 4) drilling fluid weights necessary for each stage. The permits also required that the borings comply with the Hydrogen Sulfide Rules of Act 451, as previously discussed.

Drilling through the overburden to approximately 1.5 meters (5 feet) into the bedrock for conductor casing installation was performed using an Ingersoll-Rand TH-55 truck-mounted topdrive water well drilling rig. Following conductor casing installation (discussed in the following section), each drilling location was prepared for rock drilling rig by placing a crushed stone pad at the boring location to provide a level and stable platform for drilling operations. A cellar was then excavated around the conductor casing to provide space to position a Blowout Prevention Device (BOP) required for drilling operations. Access roads were constructed as necessary and fencing was installed around the drilling sites that were not already secured by the property owner. Surface soils were isolated from drilling operations for their duration using an HDPE liner under the equipment. After installation of the cellar, a Walker-Neer Apache 150-25 top-drive, hydraulically-operated rotary drill rig was mobilized to the site and set up along with the necessary pumps and tanks to perform drilling operations. Both types of drill rigs operated continuously during drilling operations were underway.

Daily Field Reports (DFRs) were prepared that describe the site activities as the investigation program progressed and are presented on CD-1, attached to this report.

### 6.1 Casing Program

Steel casings were installed and cemented in the borings in order to keep the boreholes open during drilling and subsequent downhole work, and to prevent cross-contamination of the various aquifers present in the investigation zones. Four separate casing types were used during the investigation.

#### 6.1.1 Environmental Casing

Soil and groundwater contamination identified on the former Detroit Coke site necessitated the use of an environmental casing as the first stage of the casing program. A 508-mm (20-inch) outside diameter (O.D.) steel casing weighing 142.9 kg/m (96 lb/feet) was socketed into a gray silty clay layer below the contaminated zone to prevent exacerbation and contaminant migration. Environmental casing was installed in boreholes TB-1, TB-2, TB-4, TB-5, TB-6, and TB-7.

#### 6.1.2 Conductor Casing

The initial casing for sites other than the former Detroit Coke locations and the second casing for Detroit Coke sites consisted of a 340-mm (13 3/8-inch) O.D. steel casing weighing 71.4 kg/m (48

lb/feet). This casing, also referred to as conductor casing, extended from the ground surface to a minimum of 1.5 meters (five feet) into bedrock. This casing was installed to protect groundwater aquifers and to prevent potential exacerbation and cross-contamination of any contaminants in the overburden (drift).

#### 6.1.3 Surface Casing

Surface casing was also installed during the drilling process. This 244-mm (9 5/8-inch) O.D. steel casing weighing 53.6 kg/m (36 lb/feet) was installed from the ground surface to a depth of approximately 30.5 m (100 feet) into bedrock. The surface casing was installed to protect the lowest (potentially) freshwater aquifer, which in this investigation corresponds to the bottom of the glacial drift.

#### 6.1.4 Final Casing

Sloughing and localized swelling of the shale bedrock in some horizons was noted during downhole testing at TB-1 and TB-6. A removable packer assembly, in combination with a 140-mm (5 1/2-inch) O.D. steel casing weighing 25.3 kg/m (17 lb/feet) was installed in TB-1 and TB-6 to prevent further gauging (borehole restriction or enlargement) issues within the borehole. Steel casing was used to limit the effect on the cross-well data acquisition that was to follow, as the final casing was not cemented into place.

#### 6.1.5 Casing Cementing Methods

Each casing installation was cemented in place using Class A cement and was fully cemented to surface with the exception of the 140-mm (5 <sup>1</sup>/<sub>2</sub>-inch) casing. Prior to cementing, a fitting and valve were installed on the top of the casing, cellophane flake (Cello Flake) and cement were then pumped into the interior of the casing. Pressurized water was then used to displace the cement into the annular space between the borehole and the outside of the casing, leaving a plug of cement at the bottom of the interior of the casing. The cellophane flake was pumped as a spacer ahead of the cement to better gauge cement return to the surface and partially act as a lost circulation additive.

At TB-10, TB-13, and TB-16, cement or flake return to the surface was not observed during displacement. In order to ensure that the casing was fully cemented, additional cement was then placed into the annular space from the outside of the casing following displacement (backside grouting). When cement was observed to be lost into the rock formation during this process, a mechanical device called a cement basket was then placed in the annular space to facilitate cementing in the proper horizon.

Schematics illustrating well construction have been prepared and are included in Appendix A.

#### 6.1.6 Blowout Prevention (BOP) Device

A BOP was used on each borehole during drilling to ensure and maintain pressure control of the wells, as required by the MDEQ permit. The BOP used during drilling was considered "full stack" and included one set of pipe rams, blind rams, and an annular preventer. The stack configuration used for this project had the pipe rams and blind rams configured and bolted to the 244-mm (9 5/8-inch) O.D. steel surface casing. The pipe and blind rams were configured to clamp, secure, and shear the drill string (hang) in the case of a downhole pressure situation. An

annular BOP was then placed on the top of the stack to control a wide variety of tubing, casing, core barrels, etc., that were used in the construction of the well.

The MDEQ also required pressure testing of the BOP units prior to advancing the borehole beyond surface casing. After the surface casing had been cemented and the cement had cured for the required time, the pipe and blind rams were tested to 20.68 Mpa (3,000 pounds per square inch (psi)) for a period of 30 minutes. Pressure on the backside of the conductor casing was monitored, as well as the BOP connections. When the BOP maintained the required pressure for the required time frame, the pressure test was concluded. The drill string was then dropped into the hole to tag the cement column inside the casing in preparation for the annular BOP pressure test. The annular BOP was then tested with the drill string in the hole to 6.895 Mpa (1,000 psi) for 20 minutes. When the annular BOP maintained pressure for required time frame, the pressure test was also concluded.

## 6.2 Drilling Fluids

Boring operations required the use of drilling fluids to lubricate and cool the drill bit, promote borehole stability, and remove cuttings from the borehole. Drilling fluid minimum requirements were outlined in the MDEQ permits for each boring, but were substantially changed during drilling based on environmental field conditions.

For environmental casing installation on the former Detroit Coke site, drilling fluid, commonly known as "spud mud," was formulated to have a density significantly higher than water (between 1.2 to 1.6 kg/L (10 to 13 lb/gal)) and sufficient viscosity in order to prevent fluid and groundwater migration into and out of the borehole. These characteristics were also desired in order to remove cuttings from the borehole while maintaining low annular velocities, allowing the borehole to be cut to true diameter and minimize washout. The drilling fluid was composed of chlorinated fresh water mixed with bentonite clay and then flocculated with lime. Additives to compensate for naturally occurring calcium and to adjust the pH of the drilling fluid were also used during environmental casing installation.

Drilling fluid used during conductor casing installation was comprised of chlorinated, potable, fresh water mixed with bentonite clay (spud mud). Permit requirements for this fluid weight were 1.08 kg/L (9.0 lb/gal), with the mud weight typically above 1.20 kg/L (10 lb/gal) during drilling operations. From the top of the hardpan to the top of the bedrock, the fluid was enhanced with barite to provide additional weight to control potential artesian groundwater conditions at the soil/rock interface and granular zones within the hardpan.

Once drilling operations reached bedrock, fully-saturated brine (1.22 kg/L, 10.2 lb/gal) with drilling additives was used to well completion. Several additives were also used to control fluid loss into the rock formation, such as Kwik Seal and bentonite gel. Lost Circulation Material (LCM-coarse, medium, and fine) sweeps, or pills, consisting of paper, cellophane, and ground pecan shells mixed in with the drilling fluid, were pumped where fluid loss was observed in the fluid stream. The use of lime to effectively raise the pH and hydrogen sulfide scavengers, i.e. zinc oxide and zinc sulfide, were used to treat the gas-laden groundwater. Hydrogen sulfide escapes to the atmosphere less readily in high pH environments and hydrogen sulfide scavengers effectively react with the three noxious sulfurous compounds (H2S, H-2, and HS-) to change them to neutral inert compounds. Salt gel was also added to increase viscosity when the pump was crippled during partial lost circulation episodes.

## 6.3 Rotary Drilling Methods

Rock borings were generally advanced from the bottom of the 340-mm (13 3/8-inch) conductor casing (socketed and cemented 5 feet into rock) to the designated total depth (TD) using rotary drilling methods. However, borings TB-7 and TB-11 were performed using a combination of rotary drilling and coring methods. Rotary drilling was performed using tri-cone roller bits of various sizes to drill through the rock formations. A 311 mm (12 1/4-inch) diameter Smith bit was used to drill to the required depth for the surface casing. A 222 mm (8 3/4-inch) Smith bit was then used to drill out the surface casing. Some borings were advanced to TD using this bit. At other boring locations, the drillers changed to a 200 mm (7 7/8-inch) Hughes bit at approximately 360 meters (1,180 feet) below ground surface (BGS) to complete the boring. The bit change correlated to advancing the boring out of the last salt bed of the upper salt horizons. Below this depth, known anhydrite beds required the change to the 200 mm (7 7/8-inch) bit, which was a harder bit (Type 2) with smaller buttons to better handle the anhydrite.

The rig's drill string initially consisted of a combination of 203 mm (8-inch) and 152 mm (6-inch) diameter drill collars, each nominally 9.1 meters (30 feet) in length. Drill collars provided additional weight relative to drill pipe to advance the boring, as no pull-down pressure was applied by the rig to advance the bit. As the boring was advanced, additional drill collars and 114 mm (4 <sup>1</sup>/<sub>2</sub>-inch) diameter drill rods were added to the drill string. The rate at which the boring was advanced depended primarily on the weight of the drill string (weight on bit) and the characteristics of the rock formation being drilled. For optimal drilling efficiency, the drillers maintained a "drill string pendulum" where the weight on the bit equaled approximately 9,070 kilograms (20,000 pounds), while maintaining little to no pull-down from the rig. Typically, advance rates averaged 10 to 26 minutes per meter (3 to 8 minutes per foot) throughout the borings.

Drilling fluid was circulated through the drill string, out of the bit, and up the annular space between the sidewall of the borehole and the drill string. Samples collected during rotary boring consisted of drill cuttings (rock chips) produced during the drilling process. Rock chips broken off the formation as the bit progressed downward were brought to the surface in the drilling fluid. The field engineer collected cuttings from the drill rig's shale shaker as the cuttings came to the surface. Samples were collected for each ten-foot interval and logged by the field engineer (See Logs of Rotary Boring in Appendix B). Field logging consisted of identifying the constituents of the sample, as well as describing the color, particle size, and grain size of these constituents. A full description of the terminology used to classify and log rock core samples is included in Appendix E. The samples were bagged and labeled with the boring and depth interval at which they were collected. Lithology was determined by the field engineer based on drilling fluid annular velocity, drilling fluid weight, depth of the borehole, cutting size, and particle density, and the lag between the production of the cuttings and return at the surface, which typically increased as the borehole depth increased. The depth of origin of samples as logged is therefore approximate, and where applicable, has been checked against downhole logging to develop the subsurface profile.

Bagged samples of the cuttings were delivered to a warehouse in Detroit, Mich., for additional examination and description. Geologists then performed secondary logging of the samples at the warehouse to verify field identification. The sample logs were then checked against previously-developed formation and group descriptions.

## 6.4 Coring Methods

Sampling by cutting rock cores was performed in TB-7 and TB-11. Coring was performed continuously from 31.4 to 448.4 meters (103 to 1,471 feet) below ground surface (BGS) in TB-7. Rock coring in TB-11 was performed: 1) from 150.9 to 169.2 meters (495 to 555 feet) BGS to provide a representative sample of the Sylvania Sandstone; 2) from 281.9 to 315.5 meters (925 to 1,035 feet) BGS to sample the potential roof rock above the Salina F-Salt unit (F4 Bed) and to identify the top of the Salina F-Salt; and 3) from 376.4 to 395.0 meters (1,235 to 1,296 feet) BGS to sample the potential roof rock above the Salina D-Salt. A total of 70.4 meters (231 feet) of coring was performed in TB-11.

Continuous rock core samples were obtained in general accordance with ASTM D2113. Rock coring was performed with 20.0-centimeter O.D./10.2-centimeter I.D. (7 7/8" O.D./4" I.D.) core tooling. A double-tube, solid core barrel with diamond-impregnated matrix bottom-discharge bits capable of obtaining a core run length of 9.1 meters (30 feet) was used. The bit and barrel were attached to the rig's drill string, used to turn the bit and barrel and supply the bit with drilling fluid. As with rotary drilling, the drill string was comprised of drill collars and drill pipe to provide the required weight to advance the boring. The average rate of advancement during coring was approximately 114 minutes per meter (35 minutes per foot). During the advance of the core run, the field engineer recorded the elapsed time required to complete the core run, rate of advancement, and other pertinent drilling information.

Each core run consisted of tripping the bit, barrel, and drill string into the boring, performing coring operations, and then tripping the drill string back out of the borehole. The core was then removed from the barrel and placed in boxes. Core boxes were three feet in length, generally necessitating 10 to 11 boxes for the average core run, based on natural and mechanical (handling-induced or intentional) breaks in the core occurring at irregular intervals.

The field engineer then field logged the recovered core. Logging consisted of calculating the percent recovery, fractures per foot of run, and the Rock Quality Designation (RQD) value for each core run, as well as documenting the rock type and the location and type of discontinuities present in the core run. The recovery is defined as the total length of core retrieved from the barrel divided by the total distance the barrel was advanced during coring. The RQD is defined as the total length of rock core pieces greater than four inches in length divided by the total advanced distance of the core run. Fractures-per-foot measurements were determined by the field engineer immediately after sampling by counting fractures that appeared natural and were not obviously caused by handling of the core or by intentional breakage. Potential test samples were selected by the field engineer and were immediately wrapped with plastic wrap and the ends taped to delay moisture loss.

The core boxes were then delivered to the warehouse for secondary logging by geologists and for final test sample selection. The Logs of Core Boring are presented in Appendix C. The logs include information for each core run, including a detailed rock description, lithology marker beds noted by the geologist, a description of fractures and mechanical breaks noted by the field engineer at the time of sampling, results of sampling for hydrogen sulfide gas and methane during sampling. Photographs of Rock Cores are presented in Appendix D (CD-2 for this report).

## 6.5 Downhole Logging Program

As a basis for correlation between test borings and lithology determination, an extensive and supplemental downhole geophysical logging program (i.e., wireline logs, electric logs, downhole logs) was developed and implemented to better document borehole conditions and lithology. The results of the downhole investigation have been included on CD-1. A description of the logging program is detailed in the following paragraphs.

#### 6.5.1 Directional/Deviation Surveys

Directional borehole deviation surveys (gyroscopic) were performed to determine borehole dip and azimuth. Boreholes as they progress downward through the earth are not typically straight and some lateral variation is expected. This log determines the deviation from a straight path at depth intervals along the borehole. The actual path of the borehole is critically important when performing subsequent cross-well seismic surveys, as any deviation from vertical, i.e. horizontal position from well to well, can cause artifacts in the processing phase. Borehole deviation surveys were conducted in each of the DRIC test borings.

#### 6.5.2 Natural Gamma Logging

Natural gamma logs are run in combination with each of the other downhole logs. The gamma logs measure the natural radioactivity of the bedrock formations. Rock units that contain argillaceous (clayey) or shale portions in their rock matrix will exhibit a naturally higher gamma reading. Rock units such as clean sandstones, clean limestone/dolomite, and salt layers will exhibit very low natural gamma readings. Gamma logs were also run by the cross-well contractor before acquisition to "tie-in" the downhole logging with the eross-well equipment for correlation of formation/depth information in the borehole. Gamma ray logs provide a relatively inexpensive way to correlate bedrock information among boreholes and were run in each of the DRIC test borings in conjunction with the other types of downhole logs that were performed.

#### 6.5.3 Digital Acoustic Logging (Delta-t)

Digital Acoustic Logs (DAL) were performed to determine the acoustic properties of the bedrock formations as a function of seismic velocity change over a two-foot interval (delta-t). These logs provide the cross-well seismic program with a foundation for expected travel times through the rock units during cross-well seismic acquisition. They also provide the direct information for the tomography (travel time/velocity) portion of the cross-well seismic survey (background). These logs were run in TB-2, TB-6, TB-7, TB-10, TB-11, and TB-14 prior to the acquisition of the cross-well seismic data.

#### 6.5.4 Photoelectric (PEF)/Density Logging (Z-Densilog)

Z-Densilogs or a combination of PEF and density logs, are performed to evaluate the natural bulk density of the rock formation, which, in turn, aids lithological classification. The wireline density tool uses a Ce-137 source that emits gamma rays. The formation density is, therefore, based on the reduction in gamma ray flux between the source and the detector due to Compton Scattering. The density logs performed during this investigation were compensated, i.e., having multiple receptors to correct for the effects of drilling mud and potential mudcake in the borehole. Halite or rock salt will typically have a bulk density of approximately 2.0 grams per cubic centimeter (g/cc) (124.8 pcf), while anhydrite will approach 3.0 g/cc (187.3 pcf). Limestone, dolomites,

sandstones, and shale will typically have bulk densities of approximately 2.4 to 2.8 g/cc (149.8 to 174.8 pcf).

Photoelectric logging, or PEF, is another sensor that is run on the density sonde. The PEF log records the photoelectric absorption factor of the rock matrix. When the energy transferred from a gamma ray being fully absorbed by a bound electron exceeds the binding energy to the atom, the electron is ejected. Typically, the ejected electron is then replaced within the material and a unique X-ray will be emitted with energy that corresponds to the atomic number of the material. Typical values for PEF include sandstone (1.7), dolomite (3.0), salt (4.6), and limestone (5.0). Lithologies exhibiting higher PEF values (>5) are typical of matrices that contain heavier atoms, such as barium, iron, etc. The combination of PEF and density values is used to aid in lithological classification of individual beds and formation properties.

Compensated Z-Densilogs were performed in test boring numbers TB-1, TB-2, TB-4, TB-5, TB-7, TB-10, TB-11, TB-14, TB-15, and TB-16 prior to the acquisition of the cross-well seismic and borehole gravity data.

#### 6.5.5 Circumferential Borehole Imaging Log (Acoustic Televiewer - CBIL)

The CBIL log provides a visual "image" of the borehole wall by scanning with a narrow-pulsed acoustic beam from a rotating transducer while the tool is pulled up the hole. The transducer rotates to scan the entire circumference of the borehole wall, providing sharp images and boundary delineation. CBIL logs were run within TB-2, TB-7, and TB-10 to better delineate naturally-occurring fractures, improve borehole wall knowledge (i.e. wash-outs and lost-circulation zones), and fracture delineation.

## 6.6 Geophysical Investigation

#### 6.6.1 Cross-well Seismic Imaging Investigation

For the cross-well seismic imaging, a high-power, variable frequency piezoelectric seismic source was placed in pre-determined wellbores, with receivers located in adjacent wellbore(s) to provide high resolution reflection and tomographic images of the bedrock between wells. The direct seismic wave arrivals were used in travel time tomography to estimate the formation velocity in the interwell region at the depths logged. The reflected rays, both downgoing and upgoing, were used to provide a seismic reflection image in the same region as the tomographic image, as well as above and below the depths logged for each of the subject images between boreholes (termed "profiles").

Typically, the multiple-level receiver string (TARS Receiver Array) was positioned in the receiver wellbore(s). The source was then activated while in motion, making a traverse through the selected logging interval past the receivers. The receivers were then moved to the next position (or fan) in the data acquisition plan and the source again was activated. Each fan that was acquired produced several common receiver gathers that were correlated. Quality control was checked on site to assure that adequate data had been recorded.

Raw field records for swept-source acquisition were cross-correlated with a reference sweep and the data was then edited and organized into a cross-well data set. The direct arrival travel times were measured in first-break picking to provide input for the initial travel time tomographic inversion. The initial result was a velocity image between boreholes.

The initial velocity image from travel time tomography was then the starting point to produce a velocity model for the mapping operation. Mapping then transformed wave field-separated data from a time-domain representation to a depth-domain reflection image. Wave field separation was then used to separate other coherent arrivals present in the data from the primary reflection events needed in the mapped image. To produce interpretable images, the mapped data were then sorted by reflection incidence angles and stacked over a limited range of angles. The resulting stacks were then enhanced and produced for interpretation. Composite images from these data are included in a report by Dr. Roger Turpening and Carol Asiala, entitled "Final Report, Detroit River International Crossing, United States Side, Cross-Well Seismic Imaging", dated January 18, 2008, and included herewith in Appendix G.

Once cross-well images were processed and prepared, they were interpreted for signs of bedrock distress, as well as documentation of lithological anomalies. Anomalies important to this study include unconformities, formation top reflectivity, formation top continuity, formation dip, potential cavities and localized rubblized zones. The images are evaluated singly, as well as combined to form a composite subsurface bedrock profile.

#### 6.6.2 Vertical Seismic Profiling

Two vertical seismic profiles (VSP) were planned as part of the investigation for the following reasons: 1) investigation within shadow zones not covered well by the cross-well imaging; 2) to provide redundancy and confirmation of the cross-well imaging; or 3) to investigate apparent anomalies detected from the cross-well imaging.

After obtaining the initial cross-well and downhole information, two VSP profiles were gathered. A VSP profile was acquired into TB-2 to provide an image through the adjacent shadow zone as shown in Figure 4-3 (considered a concern because it is so near the anticipated main pier location). VSP was also conducted into TB-4 to investigate a suspected anomaly between TB-4 and TB-6, as well as to provide redundancy and confirmation of the cross-well results.

For the VSP imaging, a high power, variable-range frequency seismic source (vibrator truck) induced seismic energy at the ground surface at a selected offset, with receivers in a nearby wellbore to provide high resolution images and estimates of selected bedrock structure between the source and the well. The reflected energy was used to provide a seismic reflection image in the structure of interest. The source location for the VSP was chosen to optimize coverage of the upper salt layers in the above-mentioned anomalies.

Single-offset VSPs were performed for this project, with a source offset of approximately 215 to 275 m (700-900 feet) in the X-10 Crossing Corridor. A vibrator sweep of 10 to 150 Hz was utilized, but the attenuation in the glacial till yielded a bandwidth of approximately 20 to 90Hz (20db down points).

Composite images for the two VSP profiles are presented in Section 8.4.4 of this report

#### 6.6.3 Borehole Gravity Investigation

The Borehole Gravity (BHGM) technique is a method of determining subsurface densities over discreet vertical intervals in a drill hole. BHGM measurements are not influenced by conditions around the hole, such as washouts, mud cake, etc. Therefore, the BHGM logging tool gives more reliable density estimates than standard density logs, as the method samples a larger volume of

the rock mass. The BHGM measurements, therefore, provide a better estimate of density that is more representative of the formation. The difference in densities measured by the standard density log and calculated by the BHGM method can be used to infer nearby 3-D structures, e.g. solution cavities in salt (or elsewhere in the formation).

Typically, BHGM remote sensing applications require a remote body with sufficient contrast in density to be detected by the BHGM no farther from the wellbore than one or two times the height of the body. A spherical brine filled solution cavity 91.4 m (300 feet) in diameter may be detectable by a BHGM survey about 36.6 m (120 feet) from the edge of the cavity, effectively clearing an area approximately 128 m (420 feet) from the borehole. Local geology, and in particular the thickness of local density units, may also influence the effective radius of investigation of the BHGM.

Computer modeling of BHGM measurements was used to develop a relatively detailed model giving rise to the BHGM anomaly. Modeling has been particularly effective where seismic data can be integrated into the modeling process. As such, a model was used that is consistent with both data sets. The underlying assumption in computing apparent density is that of an earth model made up of a "layer cake" of horizontal infinite slabs. For such a model, the density of any slab is exactly given by the gravity gradient through that slab; the gradient measured at any point within the slab is constant; and, the slabs above and below it have no effect on the gradient within it.

BHGM measurements were acquired in 9 of the 13 holes drilled in the X-10 and X-11 areas. This represents a total of 1,800 measurements for the nine holes. In the X-11 corridor, BHGM measurements were acquired in boreholes TB-10, TB-12, TB-16, TB-13, and TB-15. Measurements in holes TB-10, TB-16, and TB-12 were needed to evaluate whether solution cavities exist in the region of the main pier for the bridge. Special attention was given to these three boreholes because of the observation of only 1.83m (6 feet) of B-salt in TB-10. BHGM measurements in TB-13 and TB-15 form a basis to explore for cavities underlying the location of several piers supporting the approach to the bridge.

In the X-10 corridor, BHGM measurements were acquired in holes TB-1,TB-4, TB-5, and TB-7. The measurements for TB-1and TB-7 were necessary to evaluate possible solution cavities underlying the proposed main pier, while TB-4 and TB-5 will help determine whether cavities lie beneath the location of the secondary piers.

To increase the signal to noise ratio, 4 measurements per level were taken at each borehole between 243.8m (800 feet) below ground surface and total depth (TD) using a measurement interval of 6.1m (20 feet). This provides excellent coverage through the 3 salt units (F, D and B) and through the Salina G roof rock above the F-Salt. From 243.8 to 76.2m (800 to 250 feet below ground surface), a 15.24m (50 feet) measurement interval with 2 measurements per level was utilized.

Details of the BHGM survey and results are presented in a report produced by Dr. Jimmy F. Diehl of Michigan Technological University, under subcontract to NTH Consultants, Ltd. This report is entitled "Final Report, Detroit River International Crossing, Borehole Gravimeter Surveys" dated January 18, 2007. Dr. Diehl's report provides a characterized explanation of the procedures for borehole gravity data collection, theory, and the relation to the structural geology characterized by the deep borings and downhole geophysical logging program. Dr. Diehl's report is provided in Appendix H.

## 6.7 Site Restoration

After completing field activities in the boreholes, each site has subsequently been (or is currently in the process of being) restored to original (before site activities) condition. Roads, drilling pads, and associated surface materials are currently being removed. The borings have been plugged and abandoned per MDEQ specifications. Associative repair of demarcation layers/membranes, topsoil, and grass seed will be spread were appropriate or as desired by the property owner, in the spring of 2008.

## 7. ROCK CORE HANDLING AND TESTING

## 7.1 Rock Testing

A total of 27 representative rock core samples (17 from TB-7 and 10 from TB-11), obtained during the investigation, were subjected to an array of laboratory tests including: Uniaxial Compressive Strength (UCS), Indirect (Brazilian) Tensile Strength (BTS), and Slake Durability Index (SDI). As part of the UCS tests, the static elastic constants (Young's Modulus and Poisson's Ratio) were determined as well as the density. The UCS test measures the intact compressive strength of the rock. A total of 27 UCS tests were performed generally covering the major formations encountered. The BTS test evaluates the tensile (indirect) shear of intact rock core. A total of six BTS tests were performed on selected shale and sandstone samples. The SDI test evaluates the durability of weaker/softer rock such as shale and sandstone when subjected to cycles of wetting and drying. A total of nine SDI tests were performed on selected shale and sandstone samples.

The tests were performed at the Earth Mechanic Institute (EMI) of the Colorado School of Mines, Golden, Colorado, in general accordance with ASTM D2938, ASTM D2845 ASTM D3967, and ASTM D4644. The samples were tested at their "as-received" moisture content. The tests were performed on a limited number of selected rock core samples and were intended to determine the general nature of intact properties for the formations encountered. The results of the rock tests are presented in EMI reports included in Appendix F. The EMI report also includes detailed test results, sample measurements, procedures, and photographs of the specimens after testing. A table, included in Appendix F as Table F-1, has been provided to correlate test specimen identification (i.e. depth and formation) with reference to TB-7 and TB-11.

## 7.2 Rock Core Storage

### 7.2.1 Short Term

Rock core recovered from TB-7 and TB-11 during the field portion of this investigation was temporarily stored in a climate-controlled warehouse located near the drilling sites. The core was transported to the warehouse facility after being initially logged in the field. Once arriving at the storage facility, the core was archived, stratigraphically arranged, and descriptively logged by project personnel. Final testing samples were then selected and sent to EMI.

### 7.2.2 Long Term

After being fully logged and evaluated by project personnel, the rock core was transported to the Michigan Geological Repository for Research and Education (MGRRE), a division of the Michigan Basin Core Repository located at Western Michigan University (Kalamazoo, Michigan) for long term storage. Ownership of the core has been retained by NTH Consultants, Ltd. and MDOT.

## 8. SUBSURFACE CONDITIONS AND EVALUATIONS

Based on the information gathered during this investigation, subsurface conditions within the explored areas are generally consistent, with a few exceptions. Subsoils generally consist of fill soils overlying a thick layer of generally medium-to-soft lacustrine silty clay. The silty clay soils are then underlain by a highly over-consolidated glacial till, which is underlain by bedrock. Soil, groundwater, and bedrock conditions encountered are detailed in the following sections.

## 8.1 Soil Overburden

The test boring locations selected for this project are, generally, previously-developed, but nowvacant areas. This previous use has resulted in a typical near-surface profile of fill and some abandoned foundation elements. While the extent and types of foundation elements are highly variable from site to site, the fill present typically consists of sand, frequently mixed with some percentage of clay, ranging from approximately 0.5 to approximately 5 meters (1.6 to 16.4 feet) in depth. The fill type and depth can, therefore, be expected to vary considerably over the project area.

Native materials underlying the fill generally consist of glacially deposited soils. The upper layers of native soil at the X-10 crossing were characterized by borings performed during previous projects. Based on this information, the soils at the X-10 corridor typically consist of sand to a depth of approximately 16.7 meters (55 feet), with clay layers occasionally present at some locations. Underlying the sand, soft to medium-gray silty clay is generally present to the depth at which hardpan is encountered, typically around 25.9 to 30.5 meters (85 to 100 feet). At the X-11 crossing, the upper layers of native soil were generally observed to be silty clay during the conductor casing installation. Some sand and gravel seams or layers can be expected in various locations based on local experience.

Hardpan consisting of hard gray silty clay was encountered at a depth of approximately 25.9 meters (85 feet) during conductor casing installation at the X-10 crossing. At the X-11 crossing, hardpan was typically encountered at depths ranging from 27.4 to 30.5 meters (90 to 100 feet). A thin (0.3 meter/1 foot) gravel layer was also encountered between hardpan and bedrock at the TB-13 location. In general, bedrock was encountered directly below the hardpan and will be discussed in the following sections.

## 8.2 Hydrogeologic Conditions

The use of drilling fluids during installation of casing through the glacial overburden precluded groundwater measurement during the drilling program. During rock drilling, the use of brine based drilling fluids generally precluded observations and measurements of known or suspected artesian groundwater conditions. However, flowing artesian conditions were observed during several periods of drilling after loss of circulation or brine shortages.

Lost circulation zones (partial and full) were encountered in almost all of the borings. Significant loss zones were generally encountered in the upper layers of the bedrock. Zones expected to produce artesian groundwater flow may be encountered during any future drilling at depths below ground surface of approximately 35.1 meters (115 feet), 64.0 to 67.1 meters (210 to 220 feet), 76.2 to 79.2 meters (250 to 260 feet), 83.8 to 91.4 (275 to 300 feet), 97.5 to 102.1 meters (320 to 335 feet), and 103.6 to 104.2 meters (340 to 342 feet).

During drilling, artesian conditions at TB-16 were measured after completion using a pressure gauge fitted to the well casing. Artesian pressures equivalent to a static water level at 4.3 meters (14 feet) above ground surface were recorded. After completing the drilling, the water level in this boring was initially measured at 4.6 meters (15 feet) below ground surface due to the drilling fluid that remained in the hole. Several other borings were observed after drilling to have static water levels greater than 3 meters (10 feet) above ground surface. At the end of the investigation, the observed artesian groundwater level in each of the borings ranged from 3.0 to 6.4 meters (10 to 21 feet) above ground surface.

## 8.3 Gas Conditions

Hydrogen sulfide gas is typically dissolved in the groundwater encountered during drilling at both the X-10 and X-11 areas. Atmospheric levels of hydrogen sulfide from gas escaping from the groundwater during drilling operations were generally undetectable with gas meters at the ground surface. The drill rods were typically stained black when tripped out of the borehole, indicating the presence of dissolved H<sub>2</sub>S in the formation waters. Slight hydrogen sulfide odors were noted in the drilling area at times, but were not detected when measured using a quad-gas meter that has a detection level of 1 ppm. Hydrogen sulfide gas was measured directly at the borehole after completion of drilling when riser pipes were installed to control artesian conditions. Measured concentrations during riser installation were low to not detectable; however, during bleeding off of accumulated gas within the risers (prior to cross-well seismic acquisition), hydrogen sulfide and carbon monoxide levels exceeding 20 and 35 parts per million (ppm), respectively, were measured. Based on this, and on the understanding of historical construction efforts in the area, it should be expected that major construction operations may release significant volumes of gas from the groundwater, which will need to be considered for the final design and construction.

## 8.4 Bedrock Conditions

Based on the information gathered during this investigation, the overburden (drift) is generally conformably underlain by non-clastic sedimentary rocks of the Middle Devonian Period (Dundee Limestone and Detroit River Group). Conformably underlying the non-clastic rocks is a clastic sedimentary formation (Sylvania Sandstone) of the same period, which, in turn, unconformably overlies the oldest non-clastic sedimentary bed of the period (Bois Blanc Formation). The Bois Blanc Formation is then generally underlain by a non-conformable clastic sedimentary formation of Early Devonian time (Garden Island Formation).

As the transition is continued into the Late Silurian Period, the massive, undifferentiated nonclastic sedimentary beds of the Bass Islands Group then underlie the Garden Island Formation. The successive clastic and non-clastic beds of the Salina Group, in which non-clastic members of this group include both chemical and evaporate formations, then underlie the Bass Islands Group to the explored depths of the DRIC drilling program. The encountered depths in the following sections are referenced to meters (feet) above/below mean sea level (BMSL). Mean sea level is the equivalent of 0 m (0 feet) elevation. Ground surface elevations for the borings range from 176.6 to 180.6 m (579.4 to 592.5 feet), and average 178.4 m (585.3 feet) above mean sea level (AMSL).

#### 8.4.1 Direct Investigation Results

#### 8.4.1.1 Dundee Limestone

The Middle Devonian Dundee Limestone (Dundee) is encountered at EL 146.3 to 152.5m (EL 480 to 500.4 feet) AMSL directly below the glacial drift in each of the thirteen borings performed for this investigation. The Dundee was observed as a non-clastic sedimentary rock consisting of laminated to massive interbedded light-gray-to-gray limestone and dolomitic limestone. Carbonaceous partings are common between the laminated beds as styolites. The Dundee was also noted to be petroliferous, fossiliferous, and vuggy. Fossils of brachiopods, crinoids, and horn corals were noted.

During rotary drilling operations, the Dundee typically drilled at 3.3 to 9.8 minutes per meter of advancement (mpm) (which equates to approximately one to three minutes per foot (mpf)). Rotary drilling produced rock cuttings (chips) on the order of 3 to 6 mm (1/8 to ¼-inch). During coring operations, typical advancement rates on the order of 32.81 to 114.8 mpm (10 to 35 mpf) were noted. In several of the rotary borings, penetration into the formation allowed a slight sheen to appear on the surface of the drilling fluid circulation pits (frac tanks), indicative of very small pockets of naturally occurring petroleum. The 1 to 3 mm (1/32 to 1/8-inches) pockets of petroleum correlate with those observed in the core recovered from TB-7.

The Dundee has an abbreviated thickness of approximately 8 to 30 meters (26 to 98 feet). Conformably underlying the Dundee are the clastic and non-clastic formations of the Detroit River Group (DRG). The DRG is detailed in the section below.

#### 8.4.1.2 Detroit River Group

The rocks of the overlying Dundee grade conformably into the older non-clastic rocks of the DRG. These formations begin the rock sequence otherwise known as the Mackinac Breccia. The individual formations and members of the DRG are described below.

**Lucas Formation** - The Middle Devonian Lucas Formation (Lucas) is the youngest of the Detroit River Group series of rocks (encountered during this investigation) and is encountered at EL 120.2 to 138.5m (394.3 to 454.4 feet) AMSL directly below the Dundee in all thirteen borings performed for this investigation. The Lucas was observed as a non-clastic sedimentary rock consisting of laminated to finely-interbedded light-gray-to-gray limestone, dolomitic limestone, and dolomite. The Lucas was also noted to be pitted, vuggy, and sometimes argillaceous. Fossils of brachiopods and corals were noted.

During rotary drilling operations, the Lucas typically drilled 3.3 to 9.8 mpm (1 to 3 mpf) and produced rock cuttings (chips) on the order of 3 to 6 mm (1/8 to <sup>1</sup>/<sub>4</sub>-inch). During coring operations, typical advancement rates on the order of 32.81 to 114.8 mpm (10 to 35 mpf) were noted. Several lost circulation zones were noted during drilling within this formation, most notably in TB-2, TB-4, TB-5, TB-6, TB-14, and TB-15. TB-2 and TB-4 were drilled completely blind (without fluid/cutting return) below this formation as circulation through multiple loss circulation. The transition from the overlying Dundee was not easily discernable from rock chip size, color, or rate of advancement as the contact is conformable and gradual.

From the wireline logging, a very small stringer of halite is apparent in TB-14 from approximately 85.6 to 86.6m (281.0 to 284.0 feet) depth. Salt is common in the DRG rocks in other portions of the basin, but not thought to generally exist in this region.

At a depth of approximately 57.4 m (188.4 feet), an apparent ash layer (bentonite) is present within the core sample. The deposit is believed to be the Kawkawlin Bentonite, otherwise known as the Tioga Bentonite in other adjacent sedimentary basins. The ash deposit is thought to have been formed by eruption of an ancient volcano near present day West Virginia at or near the beginning of the Erian North American Geologic Stage.

The Lucas has an observed thickness of approximately 61.3 to 89 m (201.1 to 292.0 feet). Conformably underlying the Lucas are the non-clastic beds of the Amherstburg Formation (Amherstburg). The Amherstburg is detailed in the section below.

<u>Amherstburg</u> Formation - The Middle Devonian Amherstburg Formation (Amherstburg) is the middle of the Detroit River Group series of rocks and is encountered at EL 104.2 to 132.9m (341.9 to 436.0 feet) AMSL directly below the Lucas in each of the thirteen borings performed for this investigation. The Amherstburg was observed as a non-clastic sedimentary rock consisting of laminated to thinly-bedded buff to buff-brown dolomitic limestone and dolomite. The Amherstburg was also noted to be occasionally fossiliferous and sometimes pitted. Fossils of small corals were sometimes noted.

During rotary drilling operations, the Amherstburg typically drilled 6.5 to 19.7 mpm (3 to 6 mpf) and produced rock cuttings (chips) on the order of 0.4 to 1.6 mm (1/64 to 1/16-inch). During coring operations, typical advancement rates on the order of 32.81 to 114.8 mpm (10 to 35 mpf) were noted. The transition from the overlying Lucas was not easily discernable from rock chip size, color, and rate of advancement. However, in borings where wireline density and natural gamma logs were completed, the contact between the Amherstburg and upper Lucas Formation is easily discernable.

The Amherstburg has an observed thickness of approximately 14 to 36 m (45.9 to 118.1 feet). Conformably underlying the Amherstburg are the clastic beds of the Sylvania Sandstone (Sylvania). The Sylvania is detailed in the section below.

**Sylvania Sandstone** - The Middle Devonian Sylvania Sandstone is the oldest formation of the Detroit River Group series of rocks and is encountered at EL 25.3 to 43.0 m (83.0 to 141.2 feet) AMSL directly below the Amherstburg in each of the thirteen borings performed for this investigation. The Sylvania was observed as a clastic sedimentary rock consisting of white to light gray fine-grained sandstone, dolomitic sandstone, and sandy dolomite. The sand grains (quartz) are noted to be sub-rounded, sometimes frosted, and generally very well sorted (saccharoidal). The Sylvania was also noted to be fairly-well to well-cemented in the case of the non-dolomitized horizons.

During rotary drilling operations, the Sylvania typically drilled 1.6 to 3.3 mpm (0.5 to 1 mpf) and produced cuttings (sand grains) on the order of 0.4 mm (1/64-inch). During coring operations, typical advancement rates on the order of 16.41 to 32.81 mpm (5 to 10 mpf) were noted. The transition from the overlying Lucas was easily discernable from rate of advancement and characteristics of the cuttings (change in rock facies).

The Sylvania has an observed thickness of approximately 15.2 to 25.6 m (49.9 to 84.0 feet). Conformably underlying the Sylvania are the non-clastic beds of the Bois Blanc Formation (Bois Blanc). The Bois Blanc is detailed in the section below.

#### 8.4.1.3 Bois Blanc Formation

The Middle Devonian Bois Blanc Formation is encountered at EL 23.5 to -0.34m (77.1 to -1.1 feet) AMSL directly below the Sylvania in each of the thirteen borings performed for this investigation. The Bois Blanc was observed as a non-clastic sedimentary rock consisting of light-gray-to-gray amorphous to fine-grained cherty dolomite and dolomite. The cherty inclusions were typically 5 to 20 cm (1.97 to 7.87 inches) in diameter, vitreous, and characterized by conchoidal fracture and rounded (nodular). Anhydrite was also noted filling the vugs in the cored portions and was also encountered in the chips recovered during rotary drilling.

During rotary drilling operations, the Bois Blanc typically drilled 9.8 to 32.8 mpm (3 to 10 mpf) and produced chips on the order of 0.4 to 1.6 mm (1/64 to 1/16-inch). During coring operations, typical advancement rates on the order of 131.2 to 196.9 mpm (40 to 60 mpf) were noted. The transition from the overlying Sylvania was easily discernable from rate of advancement and characteristics of the cuttings (change in rock facies).

The Bois Blanc has an observed thickness of approximately 14.0 to 25.9 meters (45.9 to 85.0 feet). Unconformably underlying the Bois Blanc is the clastic bed of the older Garden Island Formation (Garden Island), and is described as follows.

#### 8.4.1.4 Garden Island Formation

The Early Devonian Garden Island Formation is encountered at EL 9.5 to -19.5m (31.2 to -64.1 feet) AMSL, directly below the Bois Blanc in each of the thirteen borings performed for this investigation. The Garden Island was observed as a clastic sedimentary bed consisting of white to light gray fine-grained dolomitic sandstone and is believed by some to be a separate facies of the overlying Bois Blanc or underlying Bass Islands Group. The sand grains were noted to consist almost predominately of bleached and frosted quartz grains. They were also observed to be well-rounded and contain little discernable bedding structure.

During rotary drilling operations, the Garden Island typically drilled 1.6 to 3.3 mpm (0.5 to 1 mpf) where discernable. Rock chips produced during rotary drilling operations were not discernable. During coring operations, typical advancement rates on the order of 16.4 to 32.8 mpm (5 to 10 mpf) were noted. The upper contact with the Bois Blanc was easily discernable in TB-7 (core samples), but was indistinguishable in the rotary borings, except for a noted increase on the drill-o-graph (which measures penetration in feet versus time) on the drill rig. However, in borings where wireline density and natural gamma logs were completed, the contact between the upper Bois Blanc and Garden Island formations is easily discernable.

The Garden Island has an observed thickness of approximately 0.6 to 2.8 m (1.9 to 9.2 feet). Unconformably underlying the Garden Island are the non-clastic beds of the older Bass Islands Group (Bass Islands). The Bass Islands are detailed in the section below.

#### 8.4.1.5 Bass Islands Group

The rocks of the overlying Garden Island (Early Devonian) lie unconformably on the older nonclastic rocks of the Bass Islands Group (Late Silurian). The individual formations of this group (Raisin River Dolomite, Put-in-Bay Dolomite, and St. Ignace Dolomite) are typically undifferentiated in the subsurface and are presented as such in this report. The Bass Islands are encountered at EL 7.7 to -22.3m (25.3 to -73.1 feet) AMSL, directly below the Garden Island in each of the thirteen borings performed for this investigation. The Bass Islands were observed as non-clastic sedimentary rocks consisting of light gray, gray, and buffbrown amorphous to fine-grained inter-bedded dolomite, cherty dolomite, and minor dolomitic limestone. Secondary anhydrite, calcite, and selenite are commonly observed as infilling in some of the vuggy portions. In the basal beds, anhydrite and shaley anhydrite layers become dominant as the transition is approached to the underlying Salina Group.

During rotary drilling operations, the Bass Islands typically drilled 13.1 to 39.4 mpm (4 to 12 mpf) and produced chips on the order of 0.4 to 6.4 mm (1/64 to 1/4-inch). During coring operations, typical advancement rates on the order of 196.9 to 295.3 mpm (60 to 90 mpf) were noted. The upper contact with the Garden Island was easily discernable in TB-7 (core samples), but was indistinguishable in the rotary borings, except for a noted decrease on the drill-o-graph on the drill rig. However, in borings where wireline density and natural gamma logs were completed, the contact between the overlying Garden Island and Bass Islands is easily discernable.

The Bass Islands have an observed thickness of approximately 76.2 to 85.0m (250.0 to 278.9 feet). Conformably underlying the Bass Islands are the highly variable and clastic and non-clastic sedimentary formations of the Salina Group.

#### 8.4.1.6 Salina Group

The rocks of the overlying Bass Islands lie conformably on the older sedimentary rocks of the Salina Group (Late Silurian). Rocks of the Salina Group are generally described as dolomites and shaley dolomites with shale interbedding in the upper units. Evaporative rocks (halite, gypsum, and anhydrite in thin to massive bedded formations) generally appear and are separated by exceptionally pure limestone and dolomitic shales in the lower formations. The individual formations of this group are highly variable and well differentiated in the subsurface and are presented separately by unit in the following paragraphs.

**Salina G-Unit** - The Salina G-Unit (G-Shale) is encountered at EL 71.57 to 98.48 m (234.8 to 323.1 feet) BMSL, directly below the Bass Islands in each of the thirteen borings performed for this investigation. The G-Shale was observed as both a clastic and non-clastic sedimentary rock consisting mainly of green, reddish green, and greenish black carboniferous shale. The bedding is mostly indistinct and massive, with characteristics of high-horizontal stress at or soon after deposition (cupping). Near the bottom of the formation, anhydrite and shaley anhydrite beds become dominant as the transition is approached to the underlying Salina F-Unit.

During rotary drilling operations, the Salina G-Shale typically drilled 13.1 to 39.4 mpm (4 to 12 mpf) and produced chips on the order of 0.4 to 6.4 mm (1/64 to 1/4-inch). During coring operations, typical advancement rates on the order of 32.81 to 114.8 mpm (10 to 35 mpf) were noted. The transition from the overlying Bass Islands was discernable from the characteristics of the cuttings (change in rock facies). In borings where wireline density and natural gamma logs were completed, the contact between the upper Bass Islands and the G- Shale is easily discernable.

The G-Shale has an observed thickness of approximately 26.2 to 34.75 m (86.0 to 114.0 feet) and marks the end of the sequence previously referred to as the Mackinac Breccia. Conformably underlying the G- Shale are the beds of the Salina F-Unit.

<u>Salina F-Unit</u> - The Salina F-Unit (F-Unit, F-Salt) marks the uppermost formation where massive beds of evaporative rocks, including halite (rock salt) are encountered in this investigation. The salt beds are typically interbedded with layers of dolomite, shaley limestone, and dolomitic shales. The F-Unit salt beds encountered in this investigation are typically present in four distinct layers, commonly referred to as the F1, F2, F3, and F4 beds, although younger beds (F5 and F6) also are observed in some borings. The beds are numbered in accordance with their age, where F1 to F6 is the oldest to youngest in succession, respectively.

During rotary drilling operations, the F-Salt typically drilled 1.6 to 3.3 mpm (0.5 to 1 mpf) and 6.5 to 19.7 mpm (3 to 6 mpf) for the salt and chemical precipitate beds respectively, and produced chips on the order of 3.2 to 6.4 mm (1/8 to 1/4-inch). During coring operations, typical advancement rates on the order of 16.41 to 32.81 mpm (5 to 10 mpf) and 32.81 to 114.8 mpm (10 to 35 mpf) were noted for the salt and chemical precipitate beds respectively. The transition from the overlying G-Shale and between the layers in the F-Unit are discernable from the characteristics of the cuttings (change in rock facies) and based on drilling advancement rates. In borings where wireline density and natural gamma logs were completed, the contacts between these facies are easily discernable. The overall thickness of the F-Unit (not including the anomalous borings where F5 and F6 salt beds are encountered) is approximately 33.7 to 40.5m (110.0 to 133.0 feet).

**<u>F6 Salt Bed</u>** – The F6 bed was encountered in boreholes TB-5, TB-6, TB-14, and TB-15. The top of the F6 bed was typically encountered from EL 97.8 to 113.3m (320.8 to 371.6 feet) BMSL and had an average thickness of 7.6 to 13.7m (25 to 44.9 feet). Underlying the F6 bed in the above referenced boreholes was approximately 1.2 to 3.1m (4 to 10 feet) of dolomitic limestone and shaley dolomite. The F6 bed was not encountered in either of the wells with core recovery (TB-7 or TB-11).

**F5** Salt Bed – The F5 bed was encountered in boreholes TB-5, TB-6, TB-13, TB-14, and TB-15. The top of the F5 bed was typically encountered from EL 111.6 to 126.6m (366.3 to 415.3 feet) BMSL and had an average thickness of 1.8 to 4.3 m (6.0 to 14.0 feet). Underlying the F5 bed in the above referenced boreholes was approximately 4.9 to 5.2 m (16 to 17 feet) of dolomitic limestone and shaley dolomite. The F5 bed was not encountered in either of the wells with core recovery (TB-7 or TB-11).

**F4 Salt Bed** - The top of the F4 bed was typically encountered from EL 118.6 to 133.2m (389.0 to 437.0 feet) BMSL and had an average thickness of approximately 2.6 to 13.7m (8.9 to 44.9 feet). The halite layers were typically interbedded with trace anhydrite and gypsum, causing banding in the depositional structure. Small limestone and anhydrite interbeds were also observed within the TB-7 core samples. The salt was generally dark in color indicating a purity of approximately 85%, based on visual samples. Underlying the F4 bed was approximately 3.1m (10 feet) of dolomitic limestone and shaley dolomite.

**F3 Salt Bed** - The top of the F3 bed was typically encountered from EL 126.8 to 141.9m (416.0 to 465.4 feet) BMSL and had an average thickness of approximately 8.5 to 10.4m (27.9 to 34.0 feet). The halite layers were typically

interbedded with trace anhydrite and gypsum, causing banding in the depositional structure. Small limestone and anhydrite interbeds were also observed within the TB-7 core samples. The salt was generally dark in color indicating a purity of approximately 98%, based on visual samples. Underlying the F3 bed was approximately 10m (32.8 feet) of shaley limestone, dolomitic limestone, and massive anhydrite.

**F2 Salt Bed** - The top of the F2 bed was typically encountered from EL 144.2 to 159.5m (473.0 to 523.4 feet) BMSL and had an average thickness of approximately 4.3 to 8.5m (14.1 to 27.9 feet). The halite layers were typically interbedded with trace anhydrite, causing very distinct banding in the depositional structure. Small limestone and anhydrite interbeds were not observed within the TB-7 core samples, indicating a purity approaching 100% based on visual samples. Underlying the F2 bed was approximately 1 to 2m (3.2 to 6.4 feet) of shaley dolomite.

**F1 Salt Bed** - The top of the F1 bed was typically encountered from EL 153.6 to 167.5m (504.0 to 549.6 feet) BMSL and had an average thickness of approximately 13.7 to 15.9m (44.9 to 52.2 feet). The halite layers were typically interbedded with trace anhydrite and gypsum, causing banding in the depositional structure. Small limestone and anhydrite interbeds were also observed within the TB-7 core samples. The salt was generally dark in color indicating a purity of approximately 85 to 90%, based on visual samples. The F1 bed is underlain by the dolomitic rocks of the Salina E-Unit in all of the borings performed for this investigation.

**Salina E-Unit** - The Salina E-Unit (E-Dolomite) is encountered at EL 168.9 to 183.0m (554.2 to 600.3 feet) BMSL, directly below F-Unit in each of the thirteen borings performed for this investigation. The E-Dolomite was observed as a non-clastic sedimentary rock consisting mainly of competent gray and tan amorphous to fine-grained dolomite and shaley dolomite. The bedding is mainly laminated to massive, with occasional anhydrite stringers prevalent throughout. The rock shows indications of pitting and vugs that are commonly filled with secondary anhydrite, selenite, and gypsum. A volcanic episode, recorded as an ash deposit, is noted in the core from approximately EL 182.37 to 182.46m (598.4 to 598.7 feet) BMSL.

During rotary drilling operations, the E-Dolomite typically drilled 26.2 to 39.4 mpm (8 to 12 mpf) and produced chips on the order of 0.4 to 3.2 mm (1/64 to 1/8-inch). During coring operations, typical advancement rates on the order of 196.9 to 295.3 mpm (60 to 90 mpf) were noted. The transition from the F-Unit was easily discernable from the characteristics of the cuttings (change in rock facies) and rate of drilling advancement. In borings where wireline density, sonic, and natural gamma logs were completed, the contact between the upper F-Unit and the E-Dolomite is easily discernable. The E-Dolomite exhibits the fastest acoustic formation velocity of the rock units, with P-Wave velocities in excess of 7,000 meters per second (23,000 feet per second) measured.

The E-Dolomite has an observed thickness of approximately 27.1 to 37.8 m (88.9 to 124.0 feet). Conformably underlying the E-Dolomite are the beds of the Salina D-Unit.

<u>Salina D-Unit</u> - The Salina D-Unit (D-Unit, D-Salt) marks the second formation where massive beds of evaporative rocks, including halite (rock salt) are present in this

investigation. The D-salt beds are typically interbedded with layers of dolomite, shaley limestone, and dolomitic shales. The D-Unit salt beds in this investigation are typically present in two to three distinct layers, commonly referred to as the D3 and D2 beds. The beds are numbered in accordance with their age, where D1, D2 and D3 is the oldest to youngest in succession, respectively.

During rotary drilling operations, the D-Salt typically drilled 1.6 to 3.3 mpm (0.5 to 1 mpf) and 6.5 to 19.7 mpm (3 to 6 mpf) for the salt and chemical participate beds respectively, and produced chips on the order of 3.2 to 6.4 mm (1/8 to 1/4-inch). During coring operations, typical advancement rates on the order of 16.41 to 32.81 mpm (5 to 10 mpf) and 32.81 to 114.8 mpm (10 to 35 mpf) were noted for the salt and chemical participate beds respectively. The transition from the overlying E-Dolomite and the D-Unit is easily discernable from the characteristics of the cuttings (change in rock facies) and based on drilling advancement rates. In borings where wireline density and natural gamma logs were completed, the contacts between these facies are also easily discernable. The overall thickness of the D-Unit (not including where the D-1 beds is encountered) is approximately 5.2 to 12.3m (17.0 to 40.5 feet).

**D3 Salt Bed** - The top of the D3 bed was typically encountered in each boring from EL 199.9 to 217.7 m (656.0 to 714.1 feet) BMSL and had an average thickness of approximately 2.7 to 6.4m (8.9 to 21.0 feet). The halite layers were typically interbedded with trace anhydrite and gypsum, causing banding in the depositional structure. Small irregular banding consisting of limestone and anhydrite interbeds were also observed within the TB-7 core samples. The salt was generally dark in color indicating a purity of approximately 80 percent, based on visual samples. Underlying the D3 bed was approximately 3.1m (10 feet) of dolomitic limestone and shaley dolomite.

**D2** Salt Bed - The top of the D2 bed was typically encountered from EL 204.8 to 225.7m (672.0 to 740.6 feet) BMSL and the bed had an average thickness of approximately 0.9 to 7.3m (3.0 to 24.0 feet). The halite layers were typically interbedded with trace anhydrite and gypsum, causing banding in the depositional structure. Small irregular banding consisting of limestone and anhydrite interbeds were also observed within the TB-7 core samples. The salt was generally dark in color indicating a purity of approximately 80%, based on visual samples. Underlying the D2 bed was approximately 3.1m (10 feet) of dolomitic limestone and shaley dolomite, where the D1 Salt Bed was encountered. In borings where the D1 bed was absent, the D2 Salt Bed was underlain by rocks of the Salina C-Unit.

**D1 Salt Bed** - The top of the D1 bed was encountered in TB-11 and TB-15 from EL 224.7 to 227.8m (731.7 to 747.4 feet) BMSL and the bed had an average thickness of approximately 0.6 to 1.5m (2.0 to 5.0 feet). Underlying the D1 bed, where present, are the rocks of the Salina C-Unit.

<u>Salina C-Unit</u> - The Salina C-Unit (C-Shale) is encountered at EL 208.2 to 229.1 m (683.1 to 751.7 feet) BMSL, directly below D-Unit in each of the all thirteen borings performed for this investigation. The C-Shale was observed as both a clastic and non-clastic sedimentary rock consisting mainly of green and greenish brown sometimes carboniferous shale. The bedding is mostly indistinct and massive, with characteristics of high-horizontal stress at or soon after deposition (cupping). In the basal beds of the C-

Shale, anhydrite and shaley anhydrite beds become dominant as the transition is approached to the underlying Salina B-Unit.

During rotary drilling operations, the C-Shale typically drilled 13.1 to 39.4 mpm (4 to 12 mpf) and produced chips on the order of 0.4 to 6.4 mm (1/64 to 1/4-inch). During coring operations, typical advancement rates on the order of 32.8 to 114 mpm (10 to 35 mpf) were noted. The transition from the overlying D-Unit was easily discernable from the characteristics of the cuttings (change in rock facies) and rate of advancement. In borings where wireline density and natural gamma logs were completed, the contact between the upper D-Unit and the C-Shale is also easily discernable.

The rock core cut from the TB-7 well indicated large vertical fractures present within this unit extending upward of 19.8 to 21.3 m (65 to 70 feet) from the top of the underlying Salina B-Unit. The vertical fractures varied from 0.64 to 5.1 cm (1/4-inch to greater than 2 inches) in width and were in filled with halite and anhydrite of varying purity.

The C-Shale has an observed thickness of approximately 41.5 to 60.4 m (136.2 to 198.2 feet), with the exception of TB-10 and where the borings did not fully penetrate the unit (TB-12 and TB-16). The C-Shale at TB-10 had an increased thickness of 89.6 m (294 feet.)

Salina B-Unit - The Salina B-Unit (B-Unit, B-Salt) marks the lowermost formation where massive beds of evaporative rocks, including halite, are present in this investigation. The B-Salt beds are typically interbedded with shale stringers present in four to eight distinct horizons. The stringers can be used as marker beds from well to well and as indications of cavity development in the Cross-well imaging.

During rotary drilling operations, the B-Salt typically drilled 1.6 to 3.3 mpm (0.5 to 1 mpf) and produced chips on the order of 3.2 to 6.4 mm (1/8 to 1/4-inch). During coring operations, typical advancement rates on the order of 16.4 to 32.8 mpm (5 to 10 mpf) were noted. The transition from the overlying C-Shale to the B-Unit is discernable from the characteristics of the cuttings (change in rock facies) and based on drilling advancement rates. However, in borings where wireline density and natural gamma logs were completed, the contacts between these facies are easily discernable. The overall aggregate thickness of the B-Unit is approximately 44 to 66 m (144.4 to 216.5 feet), not including the anomalous borings (as mentioned below) or those that did not fully penetrate the formation.

The B-Salt is fairly uniform within the X-10 Corridor, but varies considerably in the X-11 Corridor, and also varies widely between corridors. An anomaly also was observed in the depositional history of this unit at TB-10 in the X-11 corridor. For the purposes of this report, the B-Salt will be discussed in three distinct categories, namely the X-10 Corridor, the X-11 Corridor, and the B-Salt Anomaly at TB-10, which are presented as follows.

**<u>B-Salt in the X-10 Corridor</u>** - The top of the B-Salt in the X-10 Corridor was typically encountered from EL 250.5 to 256.1 m (821.9 to 840.0 feet) BMSL and the unit had an average thickness of approximately 61.0 to 66.0 m (200.1 to 216.5 feet) where the borings fully penetrated this formation. The halite layers were typically interbedded with trace anhydrite and gypsum, causing banding in the depositional structure. Small irregular banding consisting of limestone and

anhydrite interbeds were also observed within the TB-7 core samples. The salt was generally light to dark in color indicating a purity of approximately 85 to 95 percent, based on visual samples.

**B-Salt in the X-11 Corridor** - The top of the B-Salt in the X-11 Corridor was typically encountered from EL 270.8 to 282.4 m (888.5 to 926.6 feet) BMSL and the unit had an average thickness of approximately 35.7 to 56.0 m (117.1 to 183.7 feet) where the borings fully penetrated this formation (TB-11,13, 14, and 15). The halite layers were typically interbedded with layers identified in cuttings that contain trace anhydrite and gypsum, causing banding in the depositional structure. The salt was generally light to dark in color indicating a purity of approximately 90 to 95 percent, based on visual samples.

**B-Salt Anomaly** - At TB-10, an anomaly exists in the depositional history of the B-Salt where only 1.83m (6 feet) of salt was encountered. From the rotary logs, in conjunction with extensive wireline logging, this feature appears to be conformable as solutioning and re-deposition does not appear to have occurred. The lack of salt deposition is balanced by the earlier and increased deposition of transgressive/regressive cycle rocks very similar to those of the overlying C-Shale. An anomaly similar to this is noted in historical documents and is believed to have occurred to the south at Pt. Hennepin and in the Trenton, Michigan area. The additional "pinch-outs" may be the fingers of an ancient deltaic environment where water was eroding sediment from the east (Algonquin or Findlay Arch) and was flowing into the basin from the east.

**B-Salt Shale Stringers** - As observed in the wireline logging, several very distinct shale stringers are present in the main salt bed with the B-Unit. The shale stringers are typically 1 to 3 m (3 to 10 feet) in thickness and are sometimes continuous as discussed later in the cross-well interpretation section of this report. There are predominantly 4 to 8 stringers that appear in borings that contain the full B-Salt thickness of at least 45m (150 feet). The stringers can and will be used later in the imaging portion of this work to potentially document cavity formation or propagation.

Data from the wireline logging in TB-11 and TB-13 also indicate that the shale stringers within the upper portion of the B-Salt are not present at these two boring locations, indicating a natural erosional or non-depositional event in the geologic record.

Salina A2 Carbonate - The Salina A2 Carbonate (A2 Carb) is encountered at EL 317.1 to 330.9 m (1,040.4 to 1,085.7 feet) BMSL, directly below B-Unit in eight of the thirteen borings performed for this investigation. The A2 Carb was observed as a non-clastic sedimentary rock consisting mainly of very competent gray and tan amorphous to fine-grained dolomite.

During rotary drilling operations, the A2 Carb typically drilled 26.2 to 39.4 mpm (8 to 12 mpf) and produced chips on the order of 0.4 to 3.2 mm (1/64 to 1/8-inch). Core of the A2 Carb was not recovered during this investigation. The transition from the overlying B-Unit was easily discernable from the characteristics of the cuttings (change in rock facies) and rate of drilling advancement. In borings where wireline density, sonic, and natural gamma logs were completed, the contact between the upper B-Unit and the A2 Carb is

also easily discernable. A large anhydrite stringer (also referred to as the False A2) was present and visible in most of the downhole logs and marked the transition from the Salina B to underlying A2 Unit.

The A2 carbonate was not fully penetrated in this investigation, but exhibited an observed thickness of at least 10.4 m (34.1 feet) based on the results of TB-10. However, historical information in this area indicates the A2 Carb is approximately 30.5 m (100 feet) thick.

#### 8.4.2 Cross-Well Seismic Imaging Evaluation

The cross-well images are of excellent quality with vertical resolution of 2.1 to 3.0 m (seven to ten feet) and horizontal resolution of approximately 6.1 to 10.7 m (20 to 35 feet). Details of the cross-well processing and evaluation, along with the actual cross-well images are included in a report entitled "Final Report, Detroit River International Crossing, United States Side, Cross-Well Seismic Imaging", by Dr. Roger Turpening and Carol Asiala, dated January 18, 2008., included for reference in Attachment G. In a broad sense, these data are exceptional because they provide the first-ever images in Michigan of detailed structure, beneath glacial till, that geologists previously had only inferred from widely-separated well logs or very poor resolution surface seismic reflection data.

When looking for cavities in the upper salt layers, namely the Salina F and D-Units, the investigation focused on the occurrence of broken, discontinuous reflectors that indicate that the integrity of the roof rock has been compromised. A clear indication of brine cavities would be the subsequent migration of the cavity upwards into the rock as the roof becomes wide and unstable.

When looking for salt cavities in the B-Unit structure, the investigation focused on many of the small shale stringers, as previously discussed, to be broken in a systematic manner, i.e. a vertical "column" of broken stringers with a bright reflection (the top of the "morning glory") at the top of the column. If the image plane does directly not cut the salt cavity, but is within the Fresnel zones of several stringers, then the vertical column "feature" would still be noted in the image, but, instead of completely breaking each stringer, it would reduce the amplitude of each stringer. The reflector on top would still exist but, again, not be as bright. In summary, the investigation focused on a "vertical feature" with a seismically brighter top.

#### 8.4.2.1 X-10 Corridor Cross-well Imaging

The 12 cross-well profiles obtained within the X-10 corridor have been processed and interpreted. The tops of the F-Salt layers are continuous and not broken in the seismic profiles and are clearly evident for the F5 through F1 salt layers. Seismically, the F4 and F3 layers are viewed as one continuous bed, therefore only five layers are recognized in the profiles. The tops of the D-Salt layers also appear to be continuous and not broken in the seismic profiles and are clearly evident for the D3 through D2 salt layers. The D1-Salt layer, where encountered in the drilling program, is not observed in the cross-well data.

The top of the B-Salt appears to be continuous and not broken in the seismic profiles. From the information derived from the direct and downhole geophysical investigation, the conclusion is that approximately 61 m (200 feet) of B-Salt exists in the X-10 corridor. In both profiles TB-4 to TB-2 and TB-4 to TB-5 (4-2 and 5-4 respectively), the top of the B-Salt is noted at a depth of approximately 445 m (1,460 feet). Furthermore, the wireline logs show that six or seven stringers exist in the B-Salt and, again, six or seven (or more) clear reflectors in the B-Salt in images 4-5 and 4-2. In fact, one of those stringers is observed in both images to be continuous and several others are nearly continuous.

<u>X-10 Bedrock Unconformity</u> - In the case of the TB-1 to TB-5 and TB-4 to TB-5 profiles, a relative thinning of the G-Shale toward the north is observed, corresponding to the same relative dip of the overlying strata in the same direction. The dipping G-Shale corresponds to the thinning or pinching of the F5 and F6 salt layers as observed in the borehole TB-5 lithology. This event is apparently naturally occurring and occurred at or very near the deposition of the overlying rocks.

**<u>TB-6 to TB-4 Profile</u>** - In the TB-6 to TB-4 profile, there are two different events that warrant increased discussion. These items are presented in the following sections.

#### Potential Morning Glory Shaped Feature

At approximately 107 m (350 feet) from TB-6 toward TB-4 at a relative depth of 338 m (1,110 feet), an anomaly appears in the data. It is approximately 38 m (125 feet) wide and 3 to 6 m (10 to 20 feet) high (Figure 8-1). The feature is observed in the top of the F2 bed and appears to have migrated upwards and ends in the overlying shaley roof rock. A possible "crooked" stem can be viewed below the main portion of the feature, ending at the underlying E-Dolomite.

The reflectors associated with the underlying D and B-Units are unbroken below this anomaly, indicating that if the feature is the result of historical solution mining, that mining was not performed in the lower salt units. Most notably, the shale and carbonate stringers in the B-Salt are visible and continuous.

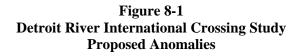
The profile from TB-6 to TB-4 was reprocessed using true-amplitude techniques to determine if the observed feature is brine or rubble filled. For this analysis, the true-amplitude processing produced data displays with an initial gain on a trace-by-trace basis prior to the mapping, migration, and stacking of traces. The data were then plotted with an overall section normalization and as a post-migration trace by trace normalization. The trace-amplitude method produced data displays with an applied initial gain to balance the "noise level," calculated on a deep window below the lowest data arrivals, i.e., ambient noise. The results illustrate the amplitude at the upper contact of the feature and overlying roof rock is not bright, which appears to indicate the feature is probably partially (maybe fully) filled. Such probable partial filling may have occurred through several processes, such as filling with bulked debris fallen from the roof, and, to a much lesser extent, from silt or mud from above through the original well opening (or through natural fissures), or through crystallization of brine.

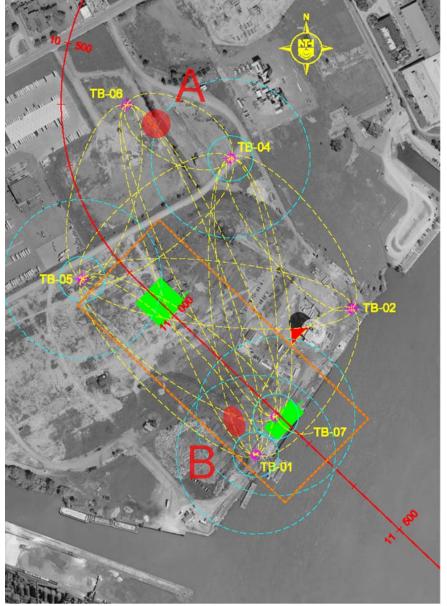
**Solution Features Adjacent to TB-4** - As viewed in the TB-6 to TB-4 profile, several features adjacent to the TB-4 borehole are of geophysical interest. Initial observation indicates the G-Shale in TB-6 at a depth of approximately 284 m (930 feet), is dipping towards TB-4, together with an associated pinching-out of the underlying F6 and F5 beds.

At depths of approximately 296 m (970 feet), 314 m (1,030 feet), 325 m (1,065 feet), and 338 m (1,110 feet), directly adjacent to the TB-4 borehole, many relatively small anomalies exist that could potentially be interpreted as cavities, but may also be features associated with disturbance in the nearby rock from the drilling itself (wash-out and borehole rugosity). The potential anomalies have been examined with the BHGM method and true-amplitude processing techniques, as addressed later in this report.

<u>**TB-1 B-Salt Anomaly</u>** - A second, and relatively small anomaly with a downward concave roof is present in cross-well profiles TB-1 to TB-4 and TB-1 to TB-6. It is at the top of the B-Salt/C-Shale contact (Anomaly B on Figure 8-1). The anomaly is approximately 37 to 52 m (120 to 170 feet) wide and 6 to 8 m (20 to 25 feet) high. The feature appears to have a tabular (i.e., "hockey-</u>

puck") shape. Upon initial examination, it was unclear if the anomaly has been caused by natural solutioning (and possibly natural recrystallization), artificial solutioning (mining), or is simply a naturally-occurring variation in the roof rock above the B-Salt. Reprocessing the data using true-amplitude techniques indicates this feature is apparently filled with material. As this anomaly is very close to a borehole, it was also examined using Borehole Gravity techniques (BHGM), which indicates a very small feature, or less-dense zone. A more-refined estimate of the B-Salt density has been obtained to re-calculate apparent density values for the TB-1 BHGM survey. This value in the apparent-density calculations makes the anomaly coincide with measured noise levels within the borehole, essentially making it a non-detectable feature. This further supports the conclusion that this feature is in-filled, or simply a naturally-occurring deviation in the top of the B-Salt. Because the roof rock appears to be intact (although sagged slightly), it appears that infilling into a naturally-occurring solution feature is the result of recrystallization or the result of silt/mud infilling through joints. Because such infilling would generally be expected over hundreds of thousands to millions of years, this anomaly is likely naturally occurring and not solution mining related.





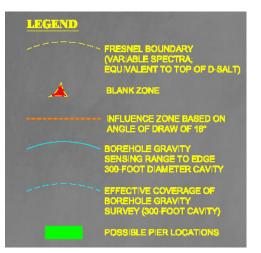
#### Anomaly "A":

- Size: About 20 to 25 feet high, about 125 ft diameter.
- Depth: Centered at about 1100 feet BGS.
- Shape: Round in Plan, "Morning Glory" in profile.
- Probably fully or partially "bulked-up," Possible limited recrystallization

#### Anomaly "B":

- Size: About 20 to 25 feet high, about 120 by 170 feet in diameter.
- Depth: Centered at about 1410 feet BGS.
- Shape: Elliptical in Plan, Hockey Puck in profile.
- Most likely a density anomaly within the B-Salt, where density is lower due to geological deposition (recrystallization, high purity, etc.)

Note: No anomalies greater than those illustrated in this figure were detected in the X-11 Crossing area investigated.



Source: NTH Consultants, Ltd.

#### 8.4.2.2 X-11 Corridor

The 16 profiles obtained within the X-11 crossing corridor have been processed and interpreted. For this report, the focus is directly on the Salina F, D, and B-Salt layers and the corresponding roof rock.

The tops of the F-Salt layers appear to be continuous and not broken in the seismic profiles and are clearly evident for the F5 through F1 salt layers. The tops of the D-Salt layers also appear to be continuous and not broken in the seismic profiles and are clearly evident for the D3 through D2 salt layers.

Contrasting the X-10 images with the surveys from profiles 10-16 and 10-12 illustrates how the rock strata has changed. The density log in borehole TB-10 shows that there is only 1.8 m (6 feet) of B-Salt present with a thin layer of anhydrite overlying it. Directly beneath the B-Unit is the A2 Carbonate. The individual impedance contrasts provided by the dense anhydrite and the light salt in a shale background produces strong reflections that are easily observed in the images. However, the B-Salt is not flat and is not a strong reflector everywhere between the boreholes. The overlying anhydrite is not flat and dips down towards TB-10.

This alludes to the possible deltaic environment, mentioned previously. Another indication in the literature states that the B-Salt was possibly dissolved in small regions by water flowing through fractures. The dipping anhydrite layer here also supports that assertion.

The investigation also looked directly above this area, adjacent to TB-10, for any indication of a massive salt cavity. No such indications are evident. The anhydrite layer is continuous as well as all of the layers in the C-Shale and D-Salt.

#### 8.4.3 Borehole Gravity Evaluation

Borehole gravity meter (BHGM) surveys are used to determine subsurface formation density values over minute intervals within a borehole. BHGM values differ from standard wireline density logs with respect to obtaining a measurement of formation density at distances of tens, or potentially hundreds, of meters (feet) from the borehole (depending on the size of the density anomaly). Wireline density logging samples the area close to the borehole. As a result, BHGM measurements are not affected by conditions within the borehole that may affect the readings on a standard density log, such as drilling fluid invasion, mud cake, or washout zones. The differences in the standard density log and the BHGM values are then used to determine nearby formation structure away from the borehole. Structural geology, or a variation in blocky structure, as well as the thickness of local density units, also affect the way a BHGM survey is utilized. The precision of the gravity equipment, and the vertical separation between density units, can also affect the BHGM results.

#### 8.4.3.1 Results of BHGM Modeling

When computing gravity values from BHGM observations, it is assumed the earth is made up of horizontal slabs of infinite length. The density of individual slabs is exactly given by the density gradient through the slab, which is assumed constant throughout and not affected by slabs above or below. Deviation from this assumption occurs when a 3-D structure is present adjacent to the borehole, but can be recognized by plotting differences from standard density logs run in the same borehole. Due to the overlying assumptions and the possibility of a nearby structure(s) within the surrounding formation, the density determined by the BHGM method is termed "apparent density."

Using the modeling software, Hol-o-grav®, synthetic apparent-density logs and corresponding density-difference profiles were generated to model several magnitude and shapes of prospective solution cavity geometries. The types of modeled cavities included multiple geometric variations of "morning-glory" and tabular-shaped bodies. The models assumed geology similar to that of TB-1, which corresponds fairly well to the average geological cross-section of both corridors. Previous modeling indicated that morning-glory shaped cavities were detectable if the distance from the borehole to the center of the cavity was less than the overall diameter of the cavity.

Two additional models have been prepared that include both tabular and morning-glory shaped geometries. Both assume a location in the F-Salt, with the roof extending into the overlying G-Shale, and are 91.4 m (300 feet) wide. The morning-glory shaped cavity extends downward to the contact of the underlying E-Dolomite, while the tabular shaped cavity has an assumed thickness of 18.3 m (60 feet), entirely within the F-Unit. Based on the modeling, the apparent density differences are large in both cases and easily recognizable when the edge of the cavity is in close proximity to the borehole, but less recognizable as the cavity is moved away from the relative borehole location. However, the negative-density anomaly increases, i.e. negative density differences are apparent over a larger number of measurement intervals. Previous detectability estimates are further enforced when random observation error is added to the data. It remains true that cavities are still likely detectable near the borehole, but likely become undetectable as the cavity diameter becomes less than that of the distance between the borehole and the edge of the cavity.

#### 8.4.3.2 BHGM Data Analysis

Gravity measurements were taken every 6.1 m (20 feet) at selected boreholes from the total depth to a depth of 243.8 m (800 feet), with each location being re-sampled four (4) times to give four (4) independent values for observed gravity. From 243.8m (800 feet) to the bottom of surface casing, the measurements were taken at 15.2m (50 feet) intervals, with each location being re-sampled two (2) times to give two (2) independent values for observed gravity. Average uncertainty in observed gravity readings, which are used to correlate data quality, ranged from 0.009 mgals at TB-16 to 0.027 mgals at TB-7. As standard density logs tend to underestimate salt density, salt densities used in the analysis where adjusted to 2.2 grams per cubic centimeter  $(g/cm^3)$  (137.3 pounds per cubic foot (pcf)).

<u>General Crossing X-10/X-11 Observations -</u> BHGM measurements show a prominent swing in positive to negative density differences, which tend to occur when a cavity is present near the borehole. In fact, most of the processed borings in the X-10 and X-11 corridors show this relationship at or near the contact with the F-Salt and G-Shale units. Further evaluation indicated that most of these readings correspond to increase Z-Scores and larger uncertainties in observed gravity. Z-Scores are defined as the standardized misfit from the inversion between the mean of the observed gravity and the calculated gravity.

It is surmised from further analysis that the inversion used to calculate the density differences was responsible for the erroneous density swings (data noise), due to the blocky nature of the local geology (anhydrite-salt-shale). The density differences were then recalculated using equations that were not affected by the blocky nature of the geology. Computed density differences in the re-calculated data have diminished the noise significantly, except in the case of TB-7 and TB-10, where noisy data remain, and could not be improved though reprocessing.

The density difference plots show little to no correlation to the results that would be anticipated in the presence of either the tabular or morning-glory shaped geometries developed during the modeling portion of this investigation (refer to Appendix I).

#### 8.4.4 Vertical Seismic Profile Evaluation

#### 8.4.4.1 General Method Evaluation

Vertical Seismic Profiling (VSP) was added to this investigation program to provide for checks and balances of the cross-well seismic reflection data and to provide a secondary method to help image any shadow zones developed during the cross-well imaging portion of the investigation. In the VSP profile design for this project, receivers (hydrophones) were placed in the boreholes with a large vibratory source located an engineered distance away (offset). Two single-offset VSPs were performed for this project with a source offset of approximately 215 to 275 m (700-900 feet) in the X-10 Crossing Corridor. A vibrator sweep of 10 to 150 Hz was utilized, but the attenuation in the glacial till provided a bandwidth of approximately 20Hz to 90Hz (20db down points). The VSPs are discussed below.

#### 8.4.4.2 Imaging the TB-2 Shadow Zone

During the cross-well program, due to subsequent modifications of final boring locations, a shadow zone was created near the location of TB-2, which is near the location of a potential future bridge foundation. A single offset VSP was performed at this location to image the upper salt layers. During the initial VSP data collection, a large "tube wave" was evident in the raw data gathers. The tube wave was caused by the large vibratory source creating a large surface wave (Rayleigh Wave), which traveled through the drift and contacted the installed casing in the borehole. This event was then transferred to the fluid within the borehole and translated to the hydrophone receivers. This effectively limited the receivers' ability to measure reflected arrivals from the formations of interest. The tube-wave event had massive amplitude and corresponded to the desired and expected frequency content of the data, thus making the data suspect. The issue was resolved by re-installing clamped receivers within the borehole, which were then clamped to the formation. The clamped receivers then measured actual particle movement within the rock, corresponding to the reflected wave arrivals from the formations of interest.

The migrated VSP image is shown in Figure 8-2. We see the top of the F-Salt at 305 m (1,000 feet) dipping toward TB-2. This is also observed in great detail in cross-well surveys TB-1 to TB-5 and TB-5 to TB-4. More precisely, an additional layer of salt in the F-Salt formation has been deposited there as previously discussed.

VP(Kft/sec) 10.0 ZDEN(g/cc) 24.0 ZDEN(g/cc) TB-2 <----> VP-1 10.0 24.0 4.0 GR(A GR(API) 0.0 0.0 200.0 200.0 Tomo(Kft/se 10.0 10.0 24.0 24.0 200 300 400 500 24.00 22.60 600 21.20 700 19.80 18.40 Kft/sec 800 Depth re project datum (ft) 17.00 15.60 900 14.20 1000 12.80 11.40 1100 10.00 1200 1300 1400 1500 1600 1700 0.0 100.0 200.0 300.0 400.0 500.0 600.0 700.0 800.0 900.0 VP1 TB2 Interwell distance (ft)

Figure 8-2 Detroit River International Crossing Study Offset, P-wave VSP Shot into Borehole TB-2<sup>a</sup>

Figure 8-2 <sup>a</sup> The A2 Carbonate is observed at 487.7 m (1,600+ feet). The high velocity base of the C-Shale, overlying the B-Salt, is noted at 426.7m (1,400 feet) and the top and bottom of the E-Dolomite is noted on either side of the light red, high velocity tomographic band at 365.8 m (1,200 feet). The top of the F-Salt is noted at approximately 304.8m (1,000 feet).

Source: Z-Seis / NTH Consultants, Ltd.

At a depth of approximately 244 m (800 feet) and a distance of 91 m (300 feet) from TB-2, the reflection takes a step upward, corresponding to the G-Shale/Bass Islands interface moving up to overlie the additional layer of F-Salt. The downhole logs in the X-10 area also show this (Figure 8-3) as indicated below.

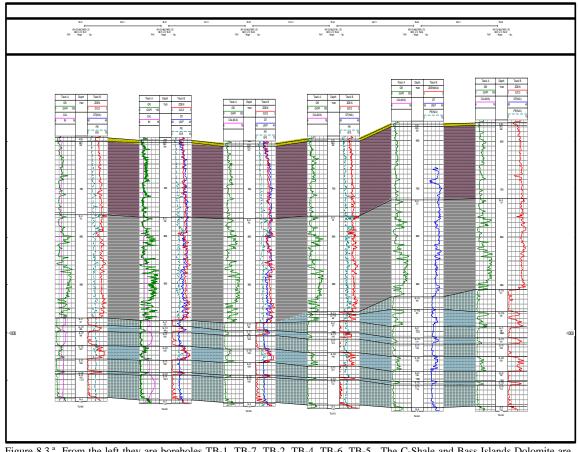


Figure 8-3 Detroit River International Crossing Study Logs from the X-10 Corridor<sup>a</sup>

Figure 8.3 <sup>a</sup> From the left they are boreholes TB-1, TB-7, TB-2, TB-4, TB-6, TB-5. The C-Shale and Bass Islands Dolomite are 9.14 m (30 feet) higher in hole TB-5 when compared to TB-1 and TB-7. The vertical seismic profile was shot between holes TB-5 and holes TB-1 and TB-7. The VSP shows this step.

Source: Michigan Technological University.

The frequency content of the VSP is low (Figures 8-4 and 8-5) compared to the spectral content of the cross-well reflection surveys. No variation in the amplitudes along the F-Salt reflection was observed, nor were reflections from the base of the E-Dolomite (top of the D-Salt), or the reflection from the base of the C-Shale (top of the B-Salt). The smooth change in amplitude that each reflection shows is merely the change in fold improving as you move away from the borehole.

Figure 8-4 displays the spectra of the downgoing first arrivals for the TB-2 VSP. The first arrivals have the highest frequency content because they have traveled the direct path (shortest distance) from source to receiver. The peak can be observed at approximately 30 Hz with 150Hz being nearly 40 db down.

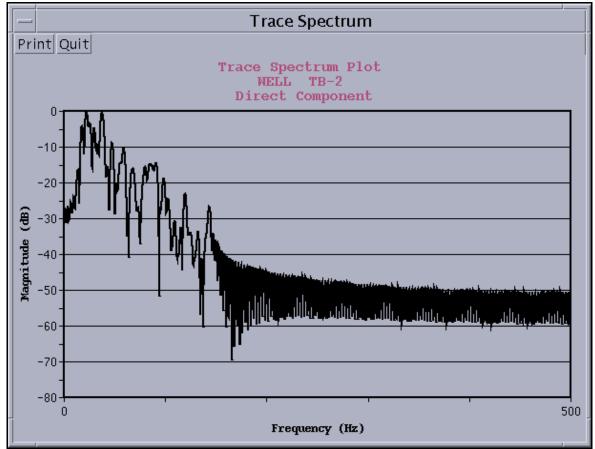


Figure 8-4 Detroit River International Crossing Study Raw Spectrum of Direct, First Arrivals in the VSP<sup>a</sup>

Figure 8-4 <sup>a</sup> Vibrator sweep was 10Hz. to 150Hz. The 150Hz portion of the spectrum is approximately 40 db down from the peak at approximately 30Hz. Deconvolution has not been applied to this signal.

Source: NTH Consultants, Ltd.

Figure 8-5 is the spectra of a reflected arrival after deconvolution. The deconvolution operator flattens the spectrum between 10Hz and 150Hz but that does not mean that the upper portion of that band is necessarily signal.

The VSP shot into TB-2 was intended to search for anomalies along the tops of the B-Salt and the F-Salt. No anomalous feature was observed along these two reflections, nor elsewhere in the image.

Figure 8-5 Detroit River International Crossing Study Spectrum of Reflections, After Deconvolution, for the TB-2 VSP<sup>a</sup>

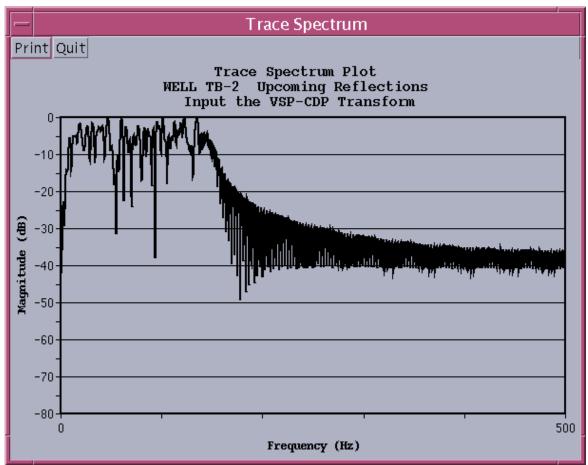


Figure 8-5 <sup>a</sup> Because this spectrum was obtained after the deconvolution operator was run the flat band of energy does not imply that the entire band is signal.

Source: NTH Consultants, Ltd.

#### 8.4.3.3 Imaging the TB-4 Cross-well Anomaly

Figure 8-6 displays the migrated image for the VSP that was shot into borehole TB-4. Here the effort was directed towards imaging the small "morning-glory" feature that was observed in the TB-4 to TB-6 cross-well survey at the major, thick stringer approximately 332 m (1,090 feet) deep in the F-Salt near TB-4. The F-Salt event is at approximately 305 m (1,000 feet).

In Figure 8-6, no anomalous features are present in the image, within the range of resolution of the VSP technology (due to the low frequency nature of the VSP data). This was expected because the feature detected by the cross-well imaging was identified as being only about 125 feet across. However, the VSP data appear to rule out a feature in this area much larger than that indicated by the cross-well data.

Figure 8-6 Detroit River International Crossing Study Offset P-wave VSP Shot into Borehole TB-4<sup>a</sup>

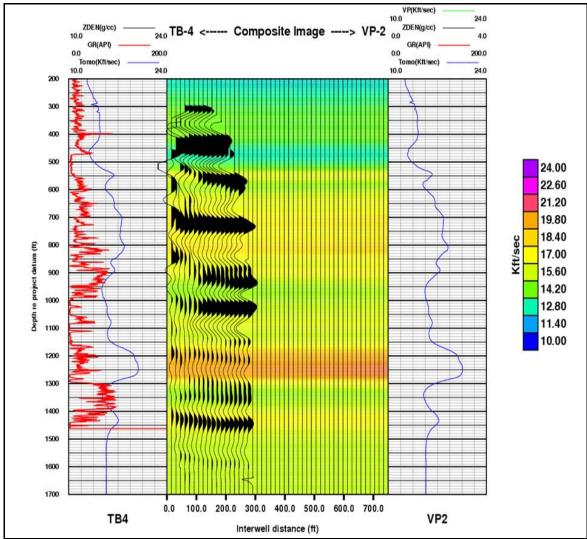


Figure 8-6 <sup>a</sup> Note that all interfaces are flat. The top of the F-Salt at TB-4 is at approximately 301.7 m (990 feet) and the top of the B-Salt is at 435.9 m (1,430 feet). The "morning glory" shaped feature observed in the Cross-well reflection is 61.0 to 91.4 m (200 to 300 feet) away from TB-4 at a depth of 332.2 m (1,090 feet). No indication of that feature is observed here either in depth or amplitude.

Source: Z-Seis / NTH Consultants, Ltd.

# 8.5 Combined Geophysical Investigation (Cross-well Seismic, BHGM, and VSP)

There is excellent correlation between the core and rotary drilling information, the downhole geophysical logs, and the geophysical data. There are no large features (possible cavities) observed in either crossing corridor greater than 37 m (120 feet) wide, on average, by 6 to 9 m high (20 feet to 30 feet).

Two specific cases are presented here that illustrate how the BHGM and cross-well surveys were used to compliment and establish correlation between observations drawn from both methods.

#### 8.5.1 BHGM Check on Suspected Solution Features

Several anomalies are observed in the TB-6 to TB-4 cross-well profile. These anomalies, due to their size and geometry, could potentially be interpreted (without the benefit of other data) as possible interconnected morning-glory type solution cavities/structures adjacent to the TB-4 borehole. The anomalies, as observed on the cross-well profile, could be interpreted in the worst case (again, without benefit of considering all the data), to range from 61 t 91 m (200 to 300 feet) in diameter, assuming circular dimension in plan view. Based on the results of the modeling, a solution cavity near the edge of the borehole would provide a large negative density difference when compared to the standard density log. The density differences in this case are all positive. Therefore, it is highly unlikely that such cavities exist. However, when all the data are considered, it is concluded that it is likely that a smaller feature exists in this area, probably in the top of the F2 bed and about 38 m (125 feet) wide and 3 to 6 m (10 to 20 feet) high as described in Figure 8-1.

#### 8.5.2 TB-1 to TB-4 and TB-1 to TB-6 Anomaly

A downward concave anomaly is observed on the top of the B-Salt at a depth of 427 m (1,400 feet). A corresponding negative-density difference is also observed at the same interval in the BHGM data. However, this calculated density difference is only -0.04 g/cm<sup>3</sup> (2.5 pcf). As such, it is considered likely that this feature is probably not a cavity and most likely a naturally occurring anomaly in the rock density at this location.

#### 8.5.3 Ground Truthing the Cross-well Results

The process of ground truthing geophysical data often involves verifying assumptions made during the initial modeling (see Section 4.2 for more details ), data acquisition, processing, or interpretation phases of a project. The collection of ground-truth data enables calibration of final images with known subsurface parameters (in this case lithology), and aids in the interpretation and analysis of what is being detected, e.g. bedrock structure. Ground truthing is important in the final interpretation of an image, when the identity and location of specific bedrock lithology, e.g. suspected anomalies, are thought to exist. To effectively ground truth the cross-well results in this program, two methods were selected to ground truth the results obtained from the TB-1 to TB-2 cross-well profile in Crossing X-10. TB-7 was drilled inline in this profile during the original investigation with continuous cored rock recovery and a complete suite of downhole geophysical logging (wireline). This allowed for ground truthing the seismic profile with the actual core data and downhole geophysical results. The TB-7 location is approximately 68.6 meters (225 feet) from TB-1. The results to correlate the cross-well profile with the wireline and direct data results (drilling) are discussed as follows:

#### 8.5.3.1 Ground Truthing the TB-1 to TB-2 Profile

The first method for ground truthing the cross-well profile performed from TB-1 to TB-2 used the data obtained from TB-7. The wireline logs performed during the initial investigation of TB-7 were superimposed on the cross-well profile at the scaled location of TB-7, analyzed for consistency, and then combined with the direct information obtained during the coring of the borehole. Prominent reflections or events were noted on the cross-well profile and then evaluated against the other data for continuity and general accordance.

The truthing points (A through V) that were chosen for analysis are presented in Appendix E, as Figure Nos. E-4 and E-5. The reference points were selected from the cross-well image and then compared with events on the core log, density, and sonic logs and generally fell into three categories. The categories consisted of:

- 1. Change in bedrock formation/ rock facies within the core log (i.e. truth point C, D, M, etc.),
- 2. Velocity change noted on sonic log with no direct variation noted in core (i.e. truth point A, B, H, etc.), and
- 3. Velocity / Density change noted on sonic and density logs respectively (i.e. truth point G, I, T, etc.).

The results of the analysis are presented in Appendix E as Table No. E-1, and are discussed below. As noted in Table No. E-1, the referenced truthing points (column I) are consistent at each reflection with differences (column VI) of 0 to 2.8 meters (0 to 9.3 feet) noted. Truthing distances are defined as the distance the reflection event in the cross-well profile differs with the event's occurrence in the core, density, or sonic logs.

For reference, a depth normalization constant (Column V) is subtracted from the cross-well event elevation before it is compared with that of the core, sonic or density logs. The subtraction of this constant normalizes the depths/elevations referenced on the cross-well profiles to that of the other logs (core, density, etc.). During cross-well processing, all top of well elevations are adjusted to the same datum to ease data migration and processing, in our case 182.9 meters (600 feet above mean sea level). In the case of TB-7, the actual elevation of the ground surface was subtracted from the normalized cross-well elevation to obtain 5.3 meters (17.3 feet).

#### 8.5.3.2 Synthetic Seismograms

Synthetic seismograms were produced using the compressional and bulk density well log data from wells TB-7 and TB-2. The synthetics were then correlated with the refection image for the profile performed between TB-1 and TB-2. Zoeppritz equations are considered to predict the angle-dependent reflectivity for AVA gathers, but the synthetics produce for this analysis do not consider vertical incidences.

The synthetic AVA gather incorporates the effect of Zoeppritz equations on amplitude as a function of angle, with the reflectivity computed in depth from the logs. The compressional, shear velocity, and density are available and extracted from the wireline logs. The reflectivity amplitude in depth is converted to time with a cos2 compression factor for imaging stretch as the function of the incidence angle, which results in a translation of the frequency spectrum. The data is then convolved in time with a selected wavelet and returned to depth with the cos2 stretch factor. The synthetics also simulate the wavelet stretch due to wide angle imaging methods as a function of angle. The effect of ray propagation through the velocity field is not computed.

The crosswell synthetic seismograms, presented as Figures E-6 and E-7 in Appendix E are stacked over a range of angles (50-60 degrees) to generate the synthetics stacks. Also shown are a comparison of synthetic seismograms generated using well information and the reflection image for the profile performed from TB-1 to TB-2. The figure includes relevant well log data such as sonic, gamma ray, and density curves.

Wells TB-1 and TB-2 are separated by approximately 295 meters (970 ft), with Well TB-7 located approximately 70 meters (230 ft) from well TB-1. TB-1 did not have a sonic log performed during the original investigation; therefore, the TB-7 and TB-2 synthetics were correlated with the reflection image for the profile performed between TB-1 and TB-2 (same AGC window and plot parameters). By comparing the "synthetic" seismic events in the seismogram with the major reflections on the reflection image; considering that sonic is acquired with distinctly more frequency range, we observed a relatively good tie, with a better performance in and near TB-2. A Vp/Vs ratio of 1.82 was selected for the analysis based on results of the same rocks at the Michigan Technological Test Site. The analysis was completed when the synthetics achieved a relatively close match to the existing data, with the Vp/Vs ratio held constant.

#### 8.5.4 Cross-Well Seismic Profile Quality Evaluation

As cross-well profile image quality varied from the X-10 to X-11 Crossing Corridors, a numerical analysis was performed to assign a numerical quality indicator for use in comparing specific profiles. Cross-well reflection image quality is a function of many factors and is discussed as follows.

#### 8.5.4.1 Signal Power

Although the strongest possible source signal would be gained by specifying many long sweeps, overall project schedule dictated that we use a value designed to maximize signal power, while maintain project schedule (i.e. eight sweeps per level and a sweep length of 1.1 seconds). To make the comparison of images possible, these acquisition parameters were used for all acquired cross-well profiles. With the source strength remaining constant, the strength of the signal impinging on a receiver is then a function of the distance between source and receiver boreholes.

#### 8.5.4.2 Noise, Ambient and Electrical

Two distinct types of seismic noise intruded on the cross-well imaging in this project and are discussed in the following paragraphs.

The first type, or ambient seismic noise, was typically very low in the boreholes due to the highly attenuative properties of the overlying glacial till. When the boreholes are relatively close together, the signal strength is strong enough that ambient noise is not observed to have an effect on the raw data. However, at large borehole distances and without the ability to increase sweep length and the number of sweeps in a stack, the ambient noise becomes noticeable.

The second type of seismic noise observed in the cross-well data consisted of electrical noise (60Hz and harmonics), which radiated from the electrical power plant and transmission lines in the survey region. Electrical noise can be reduced in the signal spectrum by passing the data through a low cut filter (200Hz in our case), and was done for several profiles in the X-11 Corridor. Passing the signal spectrum through the filter harms the spectra by narrowing the bandwidth and more importantly eliminating the low frequency portion of the signal, which propagates long distances with relatively little attenuation. Thus, the distance between the two boreholes in any given survey is an excellent proxy for signal strength. Furthermore, location of the boreholes relative to the Mistersky Power Plant is a direct measure of the amount of electrical noise encountered in the profiles in the X-11 Corridor. Visual examination of each of the cross-well images confirms this relationship.

#### 8.5.4.3 Signal to Noise Ratio

The signal to noise ratio is controlled by the signal strength, amplitude, and nature of the noise. Analyzing them separately, it is observed that signal strength at a seismic receiver is a function of the distance the seismic energy must travel to illuminate the image plane (i.e. total ray path length of reflected energy) and the "strength" of the source. The strength of the source is therefore a function of its intrinsic strength (i.e. the power in a single sweep or "pop") and the number of activations of the source, (number of sweeps in this project) summed in a stack. A swept source, as used in this project, has an additional parameter, i.e. length of sweep. For a given source, the greater the length of the sweep combined in conjunction with a greater number of sweeps produces a higher signal to noise ratio.

#### 8.5.4.4 Angle of Incidence

The angle the impinging seismic energy makes with a given interface is a critical parameter in cross-well profile quality. Rapid phase changes occur in the reflected energy (compared to the incident energy) when the angle of incidence, measured from the normal to the interface, is greater than approximately 45 degrees. When the interfaces of interest all lie at the same depth over the entire extent of the project, i.e. 305 to 460 meters (1,000ft. to 1,500ft.) in depth, this angle also becomes a function of the distance between the source and receiver boreholes.

#### 8.5.4.5 Index of Image Quality

For each profile, we examined the angle of incidence, the signal to noise ratio, and electrical noise; and assigned "quality index" between 0 and 5, with the lower end representing poor quality, and the upper end representing excellent quality. The combined average of these three indices was computed for each profile to produce a measure of the overall image quality. For overall image quality falling in the lower third (corresponding to an index in the range of zero to 1.7), the profile was considered somewhat poor, with some confidence that the image could detect an anomaly within the size range of 100 feet or less. For overall image quality falling in the middle third (from 1.8 to 3.4), the profile was considered acceptable, with good confidence that the image could detect an anomaly 100 feet in size. For overall image index in the upper third (from 3.5 to 5.0), the image was considered excellent, with very high confidence of detecting such an anomaly. Although somewhat subjective, this method was used to examine the relative quality of the images as compared to each other, and to allow for overall comparison of the confidence of our overall conclusions with respect to the image quality. A table and related summary of image quality figures have been produced and are included as Table No. E-2 and Figure Nos. E-8 and E-9 in Appendix E.

Within the X-10 corridor, ten of the twelve profiles are considered excellent quality with two images considered good quality. Based on this exercise alone, our confidence that we would detect an anomaly within one Fresnel zone of the image plane for the combined cross-well image pattern in the corridor is considered excellent. This confidence is further increased after considering the combined program, including the drilling and coring data, downhole geophysics, VSP, and gravity surveys.

Within the X-11 corridor, six of the sixteen profiles are considered excellent quality; eight are considered good quality; and two are considered somewhat poor quality. By examination of the cross-well image quality summary in the plan view, it is apparent that the proximity of the Mistersky Power plant is a primary influence in producing the poorer quality images, and that a secondary factor appears to be related to the distance between boreholes (and related large incidence angles). However, when combining the cross-well data with the drilling and coring data, downhole geophysics, and gravity surveys, we have a high level of confidence that the program is robust, and could detect an anomaly 100 feet in size within the X-11 corridor.

## 8.6 Confirmation of Rock Mechanics Model

Final evaluations of the geotechnical conditions in the various boreholes have been completed, a geotechnical model of the rock mass has been completed, and results are provided below.

#### 8.6.1 Geotechnical Rock Mass Simulations

A model of geotechnical rock mass characteristics has been completed and is presented in a report by Dr. Edward Cording, included in Appendix I. The results presented there are based on derived borehole lithology, direct observations of rock mass discontinuities from the TB-7 and TB-11 core samples, and modeled solution cavity characteristics. For this evaluation, cavities of the size and geometry similar to the Anomalies A and B described above in this report were used. This was conservative, since it is believed that Anomaly A is probably fully or partially filled, and that Anomaly B is probably a natural geologic feature, and almost certainly filled.

On these bases, an evaluation for the potential instability of such assumed solution cavities has been performed. The evaluation of the instability of solution cavities has also included a review of existing solutions in the vicinity and the results of a three-dimensional, distinct-element analysis (3DEC) of suspected solution cavity geometry.

#### 8.6.1.1 Stability of Existing Solution Cavities in the Vicinity of Crossing Corridors X-10 and X-11

For this analysis, two types of cavity geometry and characteristics were considered, namely cavities that have propagated into overlying rock layers and those completely contained within the respective salt layers. These are discussed as follows:

<u>Stable Cavities with Roofs Contained in Salt</u> - Modern solution cavities in this area were engineered and produced to meet a criterion, i.e., to leave salt in the top of the cavity and to prevent cavity migration into the roof rock. In this case, the overlying salt in the roof contains the internal pressure and provides a stabilizing effect. The roof of salt acts as a confinement layer for the saturated brine and provides support for the salt and bedded layers above. In this case (where there is no upward propagation), surface settlements are very small and can be calculated using elastic theory.

<u>Cavities with Roofs in Bedded Deposits Above the Salt</u> - For solution cavities subject to this characteristic, the fluid pressure acts on all sides of the roof blocks and does not provide stability. However, the buoyant effect of the brine does reduce the effective weight of the roof blocks and does provide a slight measure of stability.

**Upward Propagation of Solution Mining Cavities** - Locally, only settlements and large sags have occurred over areas where large, interconnected brinefields have been operated. These wells were often interconnected as fresh water was injected with brine being removed from other wells at significant distances across the brine well field. Wells and fields developed in this manner had little to no vertical or horizontal control of solutioning within the fields.

In order for surface settlements or large sags to occur, the solutioning must reach the bedded layers above the salt formation and cause localized roof collapse and progressive failure of overlying bedded formations until reaching the surface. The overlying rock layers begin to sag as localized pillar failure occurs in the remaining salt horizon. Bulking of rock fragments in this case is not enough to arrest the progression towards the surface. Eventual failure at the surface occurs.

#### 8.6.2 Preliminary Lithology and Cavity Geometry Failure Model

The report by Dr. Cording also addresses distinct types of solution cavities of the size and geometry similar to the features identified as "Anomaly A" and "Anomaly B". These geometries and conditions were evaluated using the 3DEC computer program to evaluate the potential for impacts at the ground surface. The cases examined are discussed as follows:

#### 8.6.2.1 Small Isolated Cavities

In this case, small cavities do not allow for loosening or excessive overbreak of the overlying bedded deposits. The 3DEC modeling, as well as closed beam and linear arch solutions, are used to estimate the stable cavity spans for bedding thicknesses observed in the TB-7 and TB-11 core samples.

#### 8.6.2.2 Cavity Roof is Stabilized Against an Overlying Bed

In this case, loosening and localized fallout of roof blocks occur where rock is thinly bedded, shaley, or where cavity spans are significant to allow sag and corresponding tension in the roof rock. Roofs developing in this manner often form arched geometry and continue to propagate upwards until a bed with sufficient thickness and strength is encountered to arrest further movement.

#### 8.6.2.3 Cavity Where Bulking of Rock is Sufficient to Arrest Upward Propagation

The roof span in this case is large enough that bedded layers above the salt are not thick enough to resist upward propagation. In this case, the bulked material falling from the roof is sufficient to fill the cavity and arrest upward propagation.

#### **8.6.2.4** Cavities Where Collapse Progresses to Surface

This case is similar to that mentioned immediately above, but bulking is not sufficient to arrest the upward propagation of the cavity. The upward propagation reaches the surface of the rock until a sinkhole is formed at the surface. In the modeling, cavities approaching 91 to 244 m (300 to 800 feet) in diameter are needed to form a sinkhole at the surface.

#### 8.6.3 Results of Rock Mechanics Modeling

Based on the observations made during the deep drilling and subsequent geophysical investigations, potential cavities were modeled with sizes of 37 to 52 m (120 and 170 feet) in diameter, 3 to 6 m (10 to 20 feet) high, and in a "morning-glory" shape. On the basis of the evaluations, such shapes are likely to exhibit the characteristics mentioned above for small, isolated cavities and, in the worst case, cavities where fallout is stabilized by roof rock.

In both cases, roof spans are estimated to be small with little-to-no localized overbreak or fallout of roof blocks. Cavities meeting these criteria, even if upward progression were to occur, would be arrested quickly as thicker bedded formations are encountered. Furthermore, due to the high width-to-height ratios, bulking would occur in the localized fallout and would arrest upward movement. As an additional evaluation based on the actual data, the angle of draw was re-examined using the actual rock test and core data, to confirm the basis of the borehole spacing for the program. Based on the analysis, the angle of draw was further refined and estimated to be 15 degrees or less. More importantly, it was determined that for isolated brine cavities as small as 37 m to 52 m (120 to 170 feet), the concept of angle of draw does not apply because such cavities would be stable and could not possibly progress any significant distance upward to the ground surface.

A further evaluation included in the analysis involves an examination of the potential impacts of seismic activity in the Detroit area on potential brine cavities. The analysis indicates that seismic impacts would be insignificant to the stability of any brine cavities that may exist in the U.S. project area.

## 9. CONCLUSIONS

### 9.1 Presence of Brine Cavities

Based on the data gathered and analyzed, there is no evidence of cavities larger than those discussed as Anomalies A and B shown on Figure 8-1, nor is there evidence of potential instability of the rock mass within the crossing corridors. In fact, the analysis shows that the observed anomalies have probably been, at least partly, filled by one or a combination of several mechanisms. More specific conclusions for each of the crossings are presented in the following sections.

#### 9.1.1 Crossing X-10 Corridor

There are two noticeable anomalies within the area studied for the X-10 crossing. One (previously identified as "Anomaly A") is approximately 38 m (125 feet) wide and 3 to 6 m (10 to 20 feet) high that appears to be morning-glory shaped and located between TB-4 and TB-6 at a depth of 338 m (1,110 feet). The feature is observed in the top of the F2 salt bed and appears to have migrated upwards slightly and terminates in the overlying shaley roof rock. It is concluded that this feature represents the remains of a historic brine well that is probably partially or fully filled, either by bulking, or to a much lesser extent by silt infilling or recrystallization of salt. A Franklin-Swift Salt well may have been located in the vicinity of this anomaly according to historic documents, and reportedly produced approximately 150,000 barrels of brine from 1901 through 1904. The apparent size of this anomaly is generally consistent with the volume of brine that was reported to have been removed.

A second anomalous feature is located northeast of TB-1 (identified previously as "Anomaly B"). It is located at the top of the B-Salt/C-Shale contact, and is approximately 37 to 52 m (120 to 170 feet) wide and 6 to 8 m (20 to 25 feet) high. The feature appears to have a tabular (i.e., "hockey-puck") shape. Based on the available data, it is concluded that this feature is probably a naturally occurring density anomaly, possibly formed during initial deposition or another naturally-occurring variation in the roof rock above the B-Salt. Based on the bulk of the geophysical evidence, the feature does not appear to be a cavity by any means. It is noted that no historical brine wells are known to be located in the immediate vicinity of this feature. Further, there is no evidence of solutioning in the salt beds above or below the anomaly. This would be expected if the features were related to solution mining, given the solution mining methods that were in use in the period when records were not well kept.

It should also be noted that there is no evidence that this feature is related in any way to the known solution mining that has occurred to the nearby west, on and near Zug Island. In addition to the previously discussed evidence, we offer several other points of evidence that the feature is not related to the Zug Island interconnected brine well galleries.

- The closest known solution well that is part of the Zug Island brine well gallery system is approximately 320 m (1,050 feet) to the west of Anomaly B. Based on historical records of various modern solution mining operations in the vicinity (particularly evidenced from the solution mining operations of Canadian Salt), it does not appear that there is evidence of solutioning having extended more than about 95 meters (300 feet) away from a gallery or singular brine well.
- There is no historical record of any other isolated wells in the vicinity of Anomaly B that could have connected to the Zug Island system or extended solutioning further

than the normal distance that could be expected (about 95 meters (300 feet) as indicated above). Further, there is no evidence in the cross-well profiles intersecting Anomaly B of solutioning in the salt beds above the anomaly, which would be expected if Anomaly B were due to brine extraction, given the solution mining methods that were in use in the period when records were not well kept (before 1940).

• Anomaly B is less than 30 meters (100 feet) from TB-1, and about 45.7 meters (150 feet) from TB-7. However, there was no loss of wash water from either of the test borings as they extended through the salt beds or potential roof rock, which would be expected if the feature were a cavity related to brine well solutioning and would almost certainly occur if the feature were connected so a brine well gallery system as extensive as the Zug Island system.

#### 9.1.2 Crossing Corridor X-11

In the X-11 profiles, there is a greater amount of seismic "noise" in the data, most likely the result of electrical interference (60 Hz and harmonics) from the Mistersky Power plant. The plant lies directly in the center of most of the cross-well profiles for X-11. Several of the cross-well profiles are further degraded due to longer than planned distances between the boreholes, which resulted because of site access and right-of-entry issues.

However, the tops of the F-Salt layers appear to be continuous and not broken in the seismic profiles and are clearly evident for the F4 through F1 salt layers (also F6 and F5, where they occur). The tops of the D-Salt layers also appear to be continuous and not broken in the seismic profiles and are clearly evident for the D3 through D2 salt layers. Although some of the images have relatively poor resolution, several small anomalies are present in the data which could possibly represent small features of interest. However, these anomalies are most likely the result of large angle of incidence and general reduced resolution of the images due to borehole spacing distance (when compared to the excellent resolution for the X-10 profiles). In any case, it can be stated that no significant cavities or other anomalies (i.e., greater than the size of anomalies discussed for the X-10 crossing) are evident in the X-11 seismic cross-well profiles.

## 9.2 Stability of Anomalies

As discussed in earlier sections of this report, the investigation and analysis performed for the X-10 and X-11 crossings indicates that there are no anomalies in either crossing greater than 120 to 170 feet in diameter, and that for the larger end of this size range (Anomaly B), the anomaly is almost certainly not a void and not related to historical brine mining. For both cases however, analysis was performed, conservatively modeling the anomalies as open cavities with sizes of 37 to 52 m (120 and 170 feet) in diameter, 3 to 6 m (10 to 20 feet) high, and in a "morning-glory" shape.

For both cases, the analysis concludes that there would be little-to-no localized overbreak or fallout of roof blocks, and that is any upward progression were to occur, it would be arrested quickly as thicker bedded formations are encountered, and/or bulking of the rock fallout occurs.

On this basis, it can be stated that the combined analysis indicates that any of the size of anomalies that are potentially present in the X-10 and X-11 crossings (based on the

investigation), would not propagate to the surface nor could they have impact at the surface in the foreseeable future.

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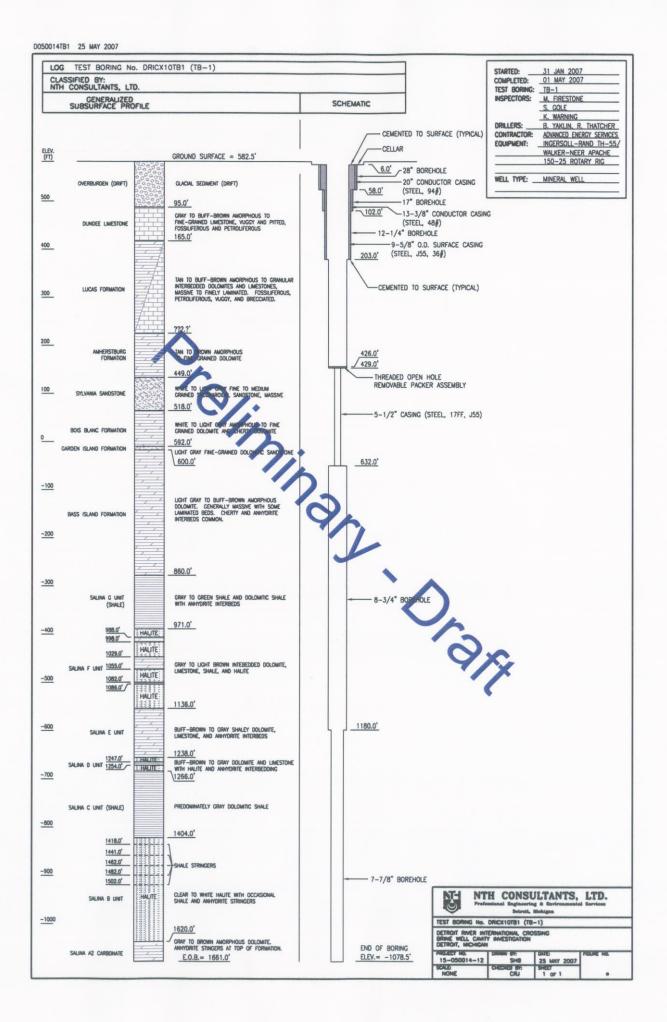
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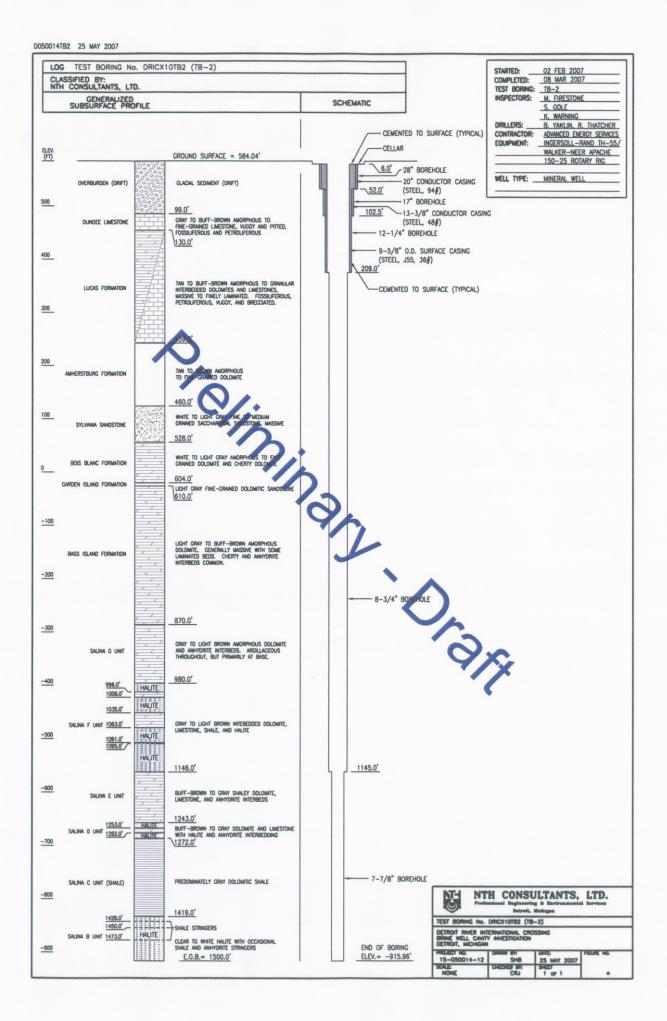
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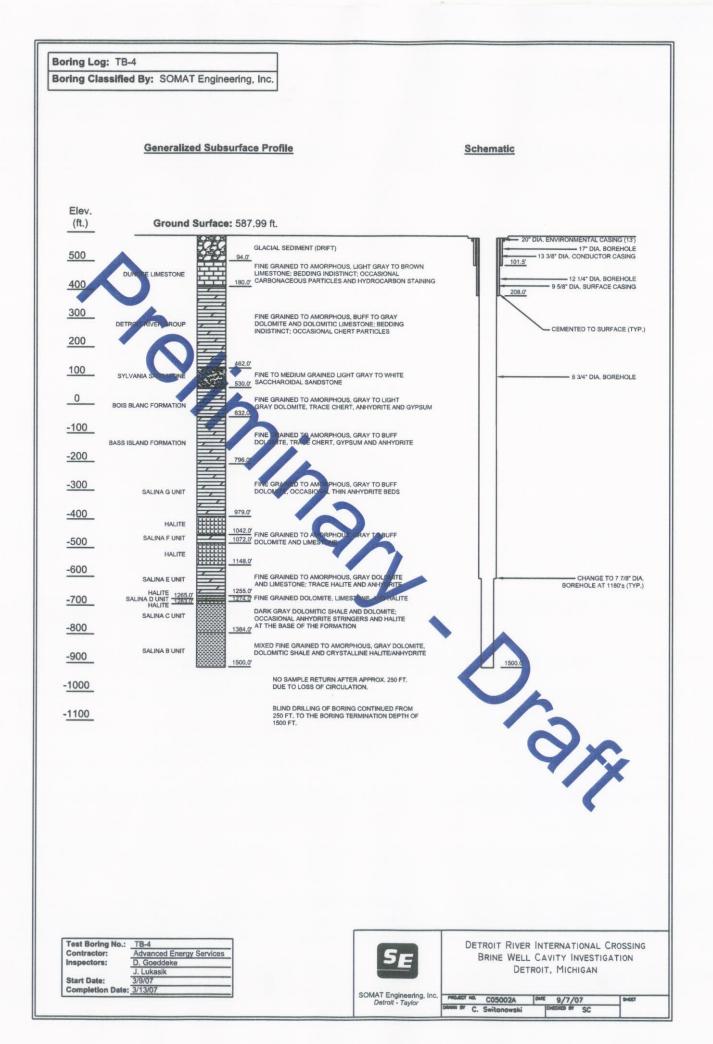
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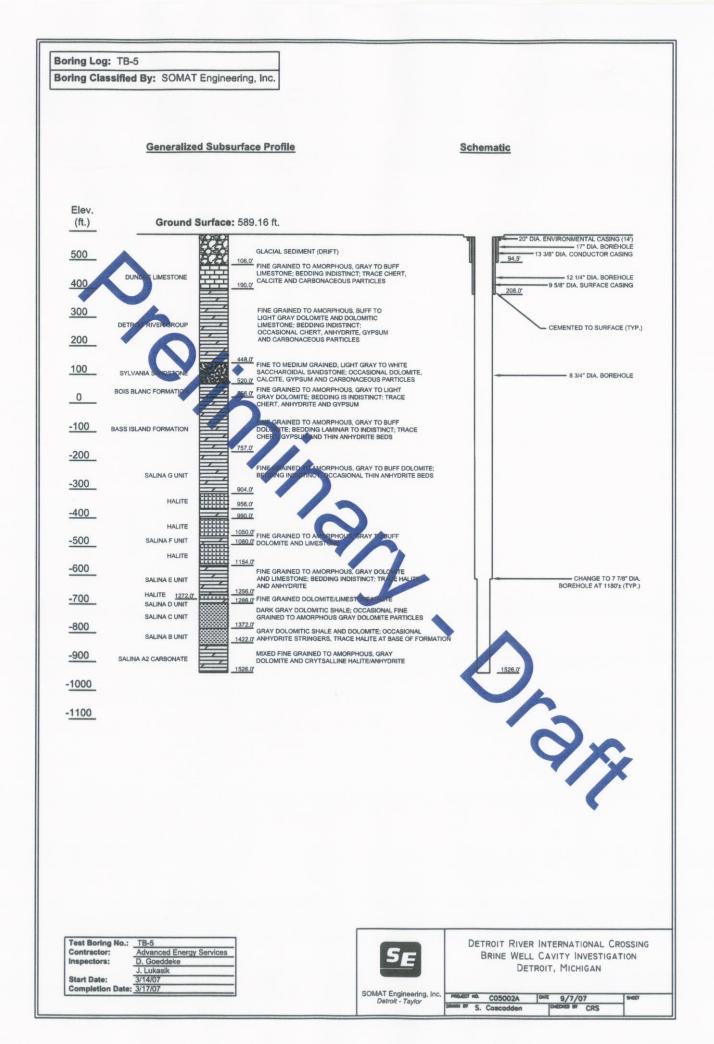
# Appendix A

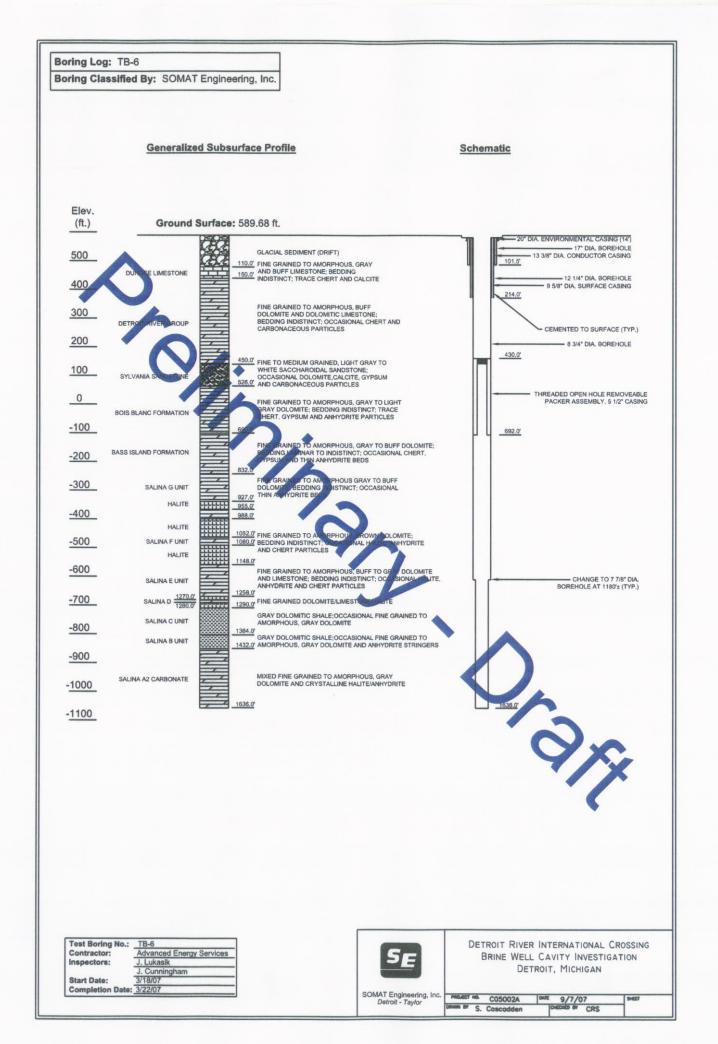
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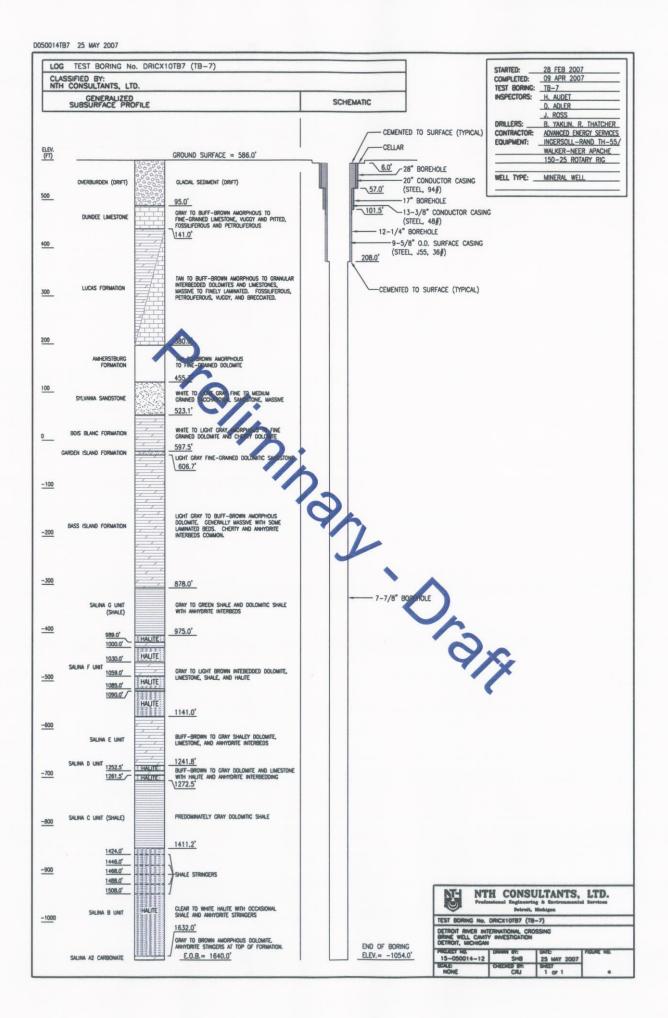


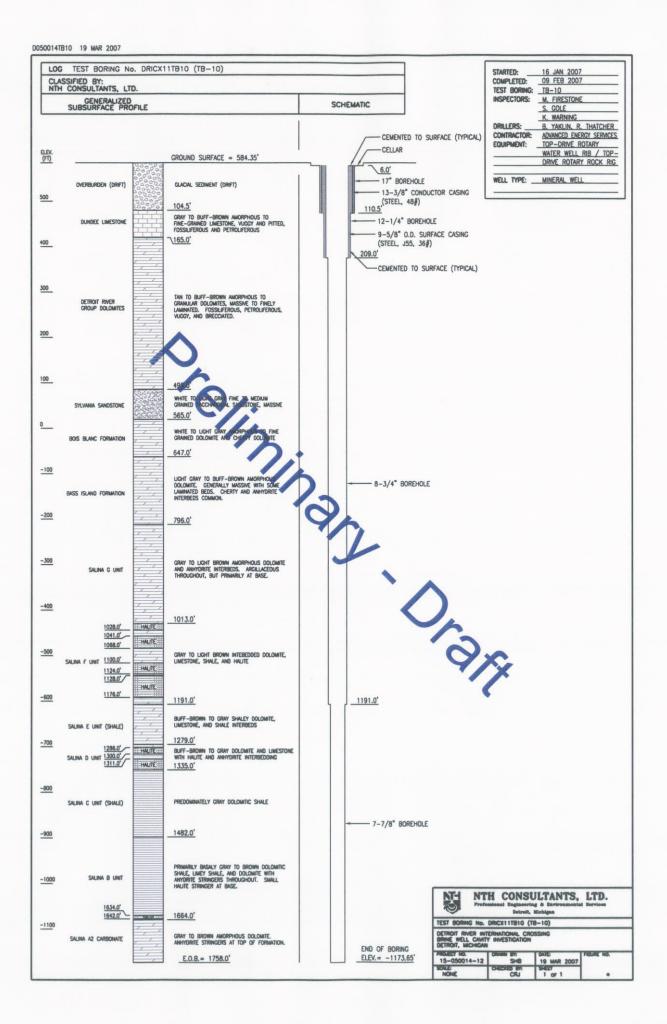


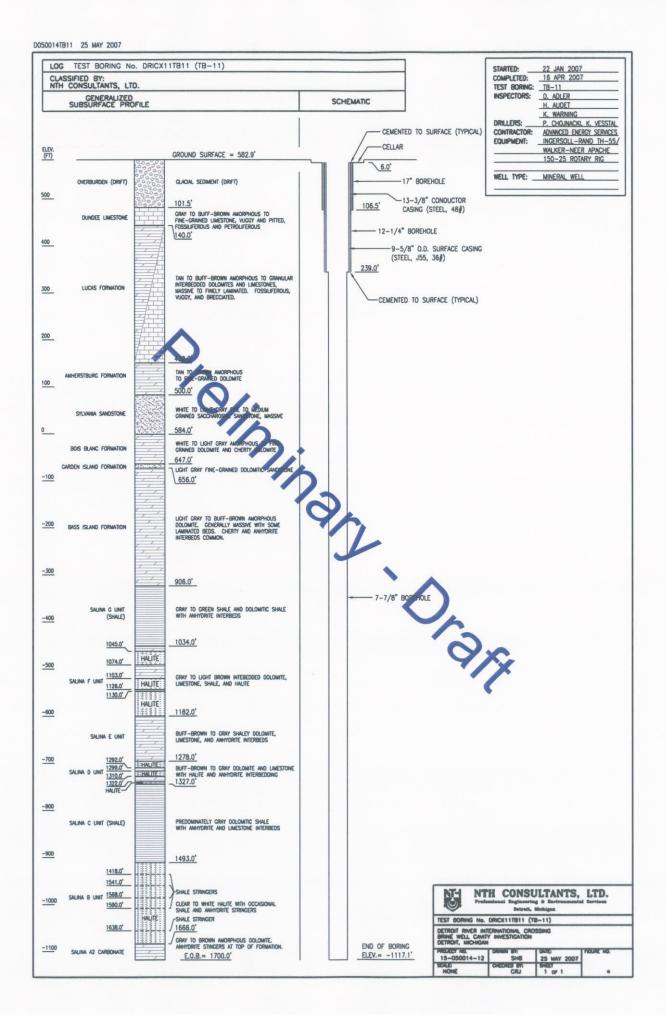


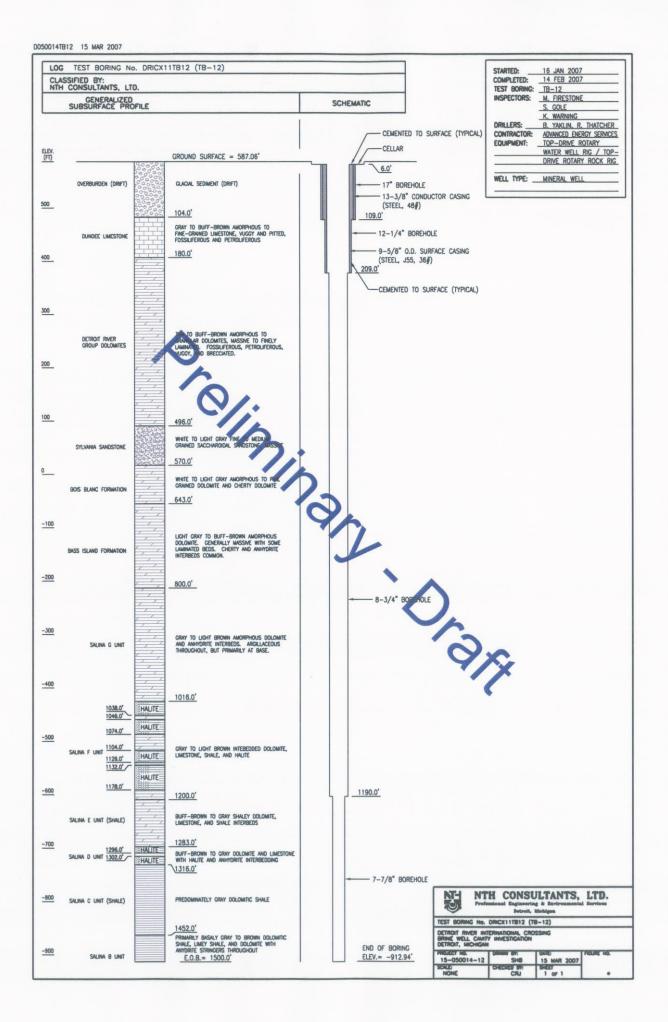


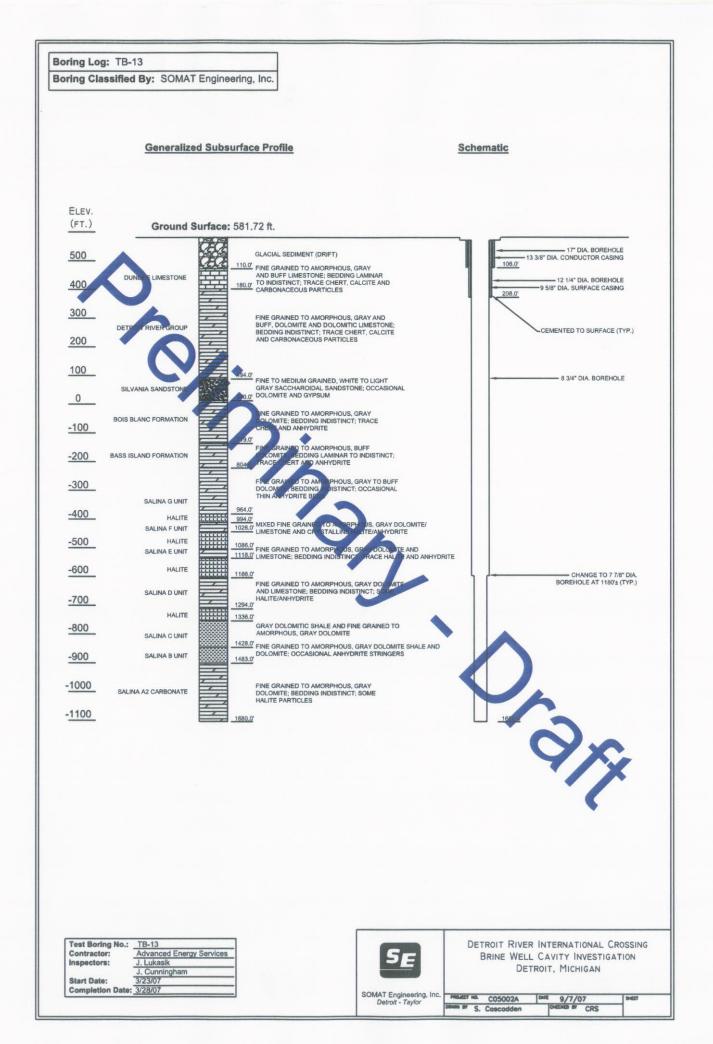


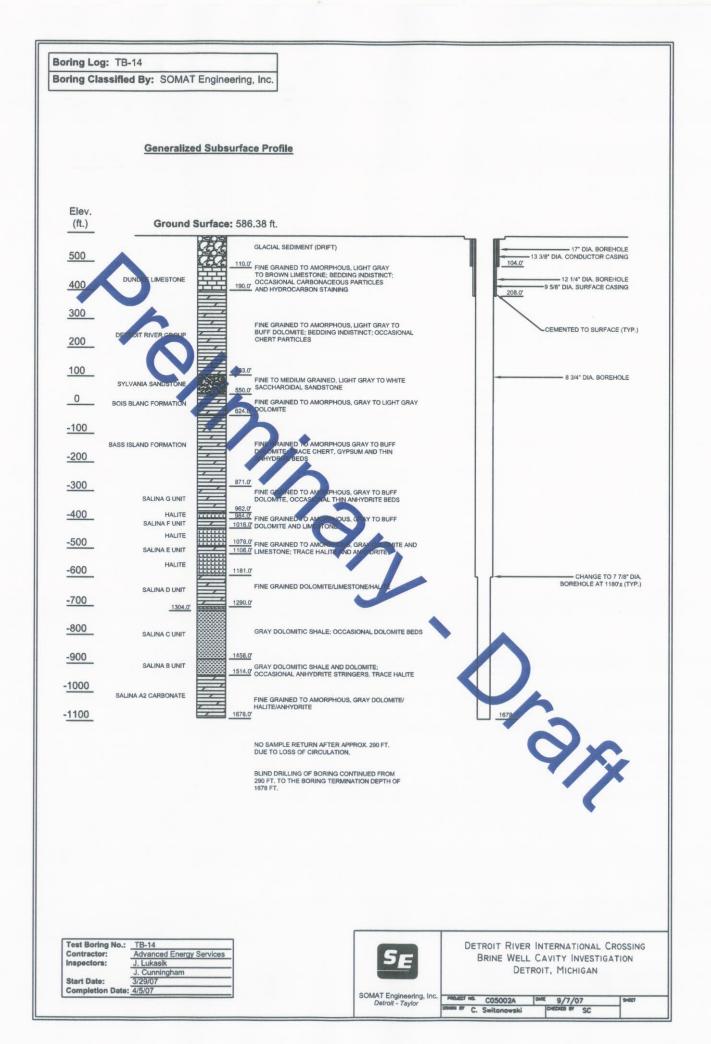


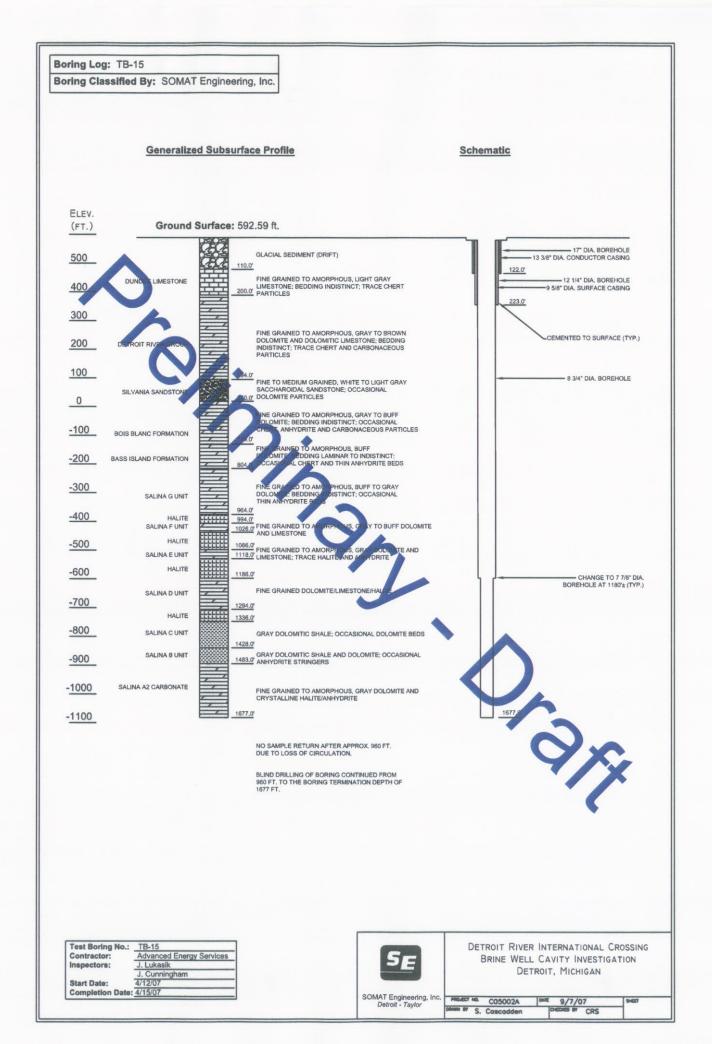




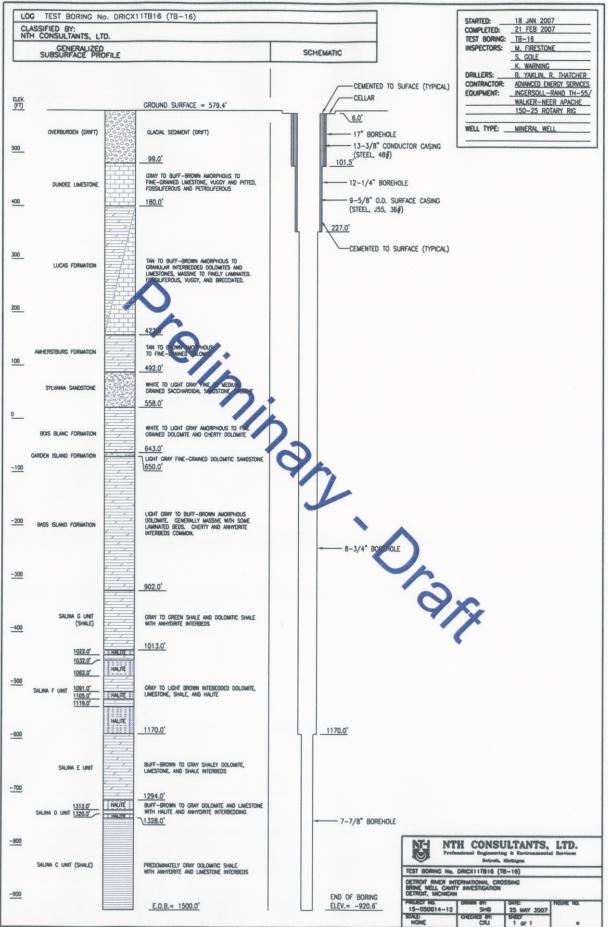






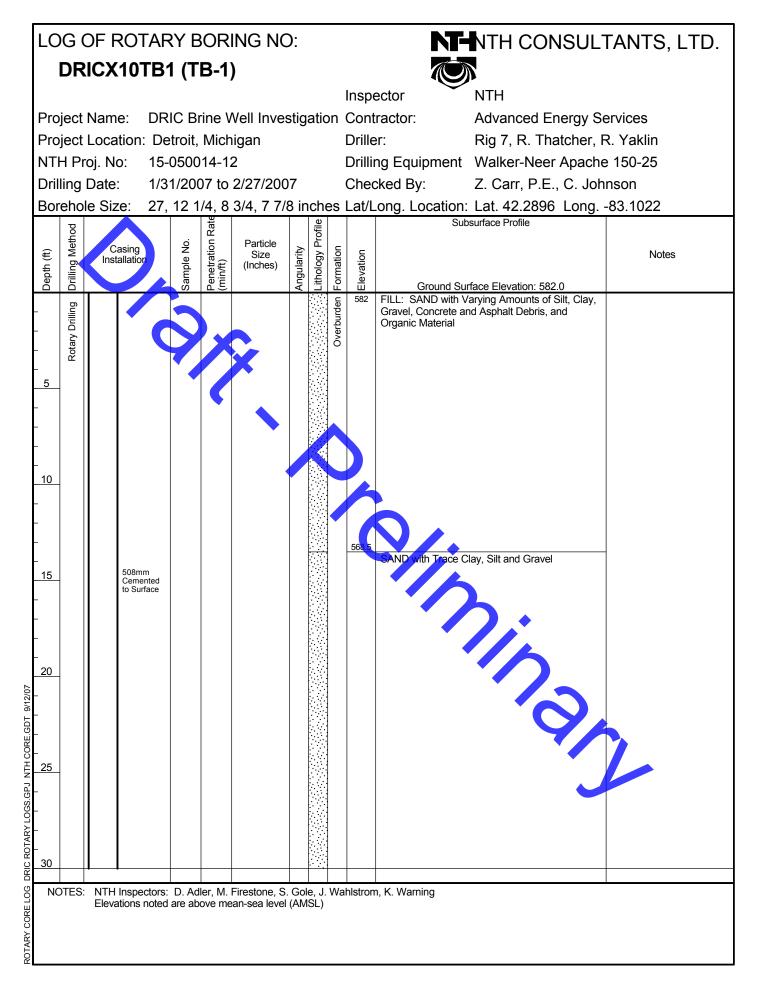


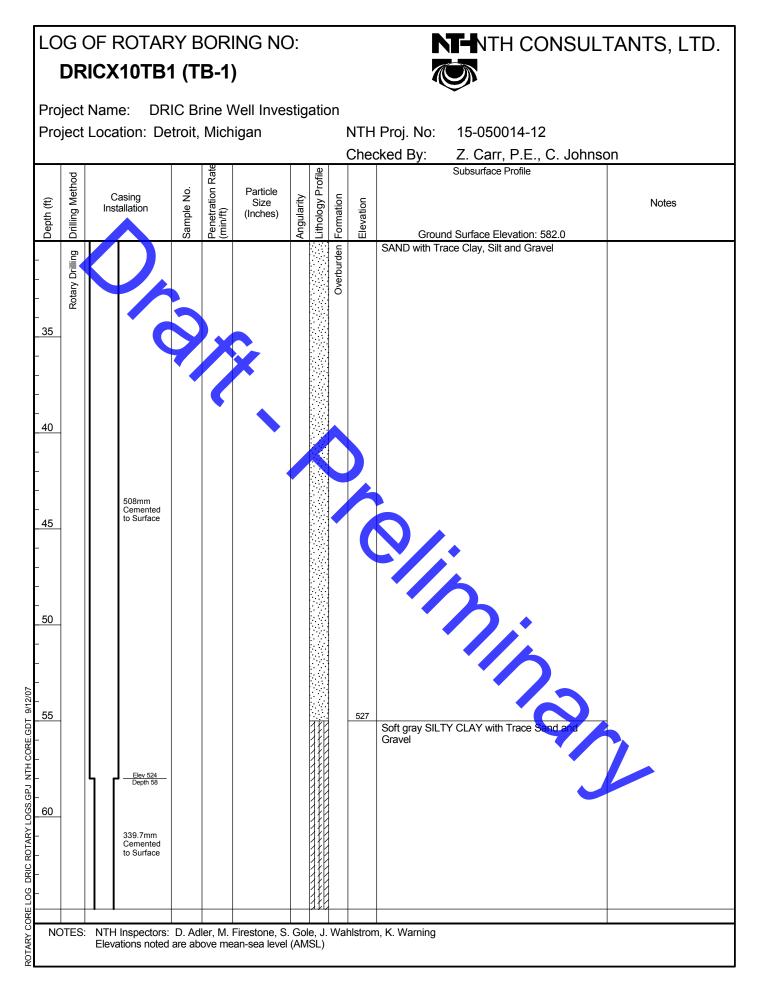
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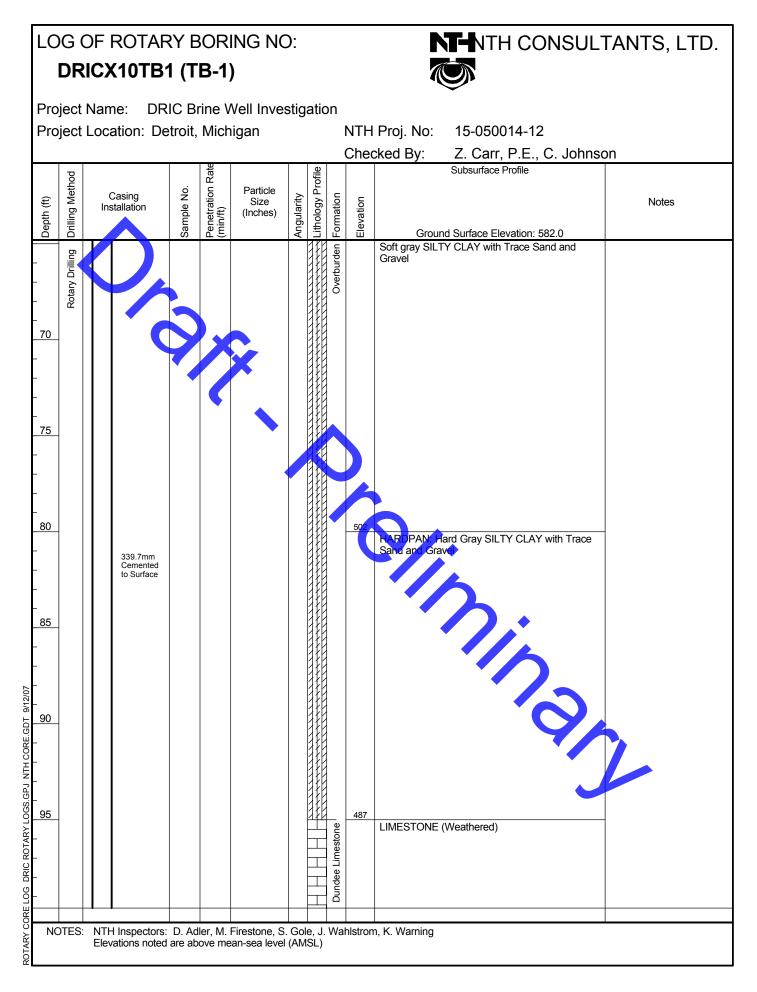


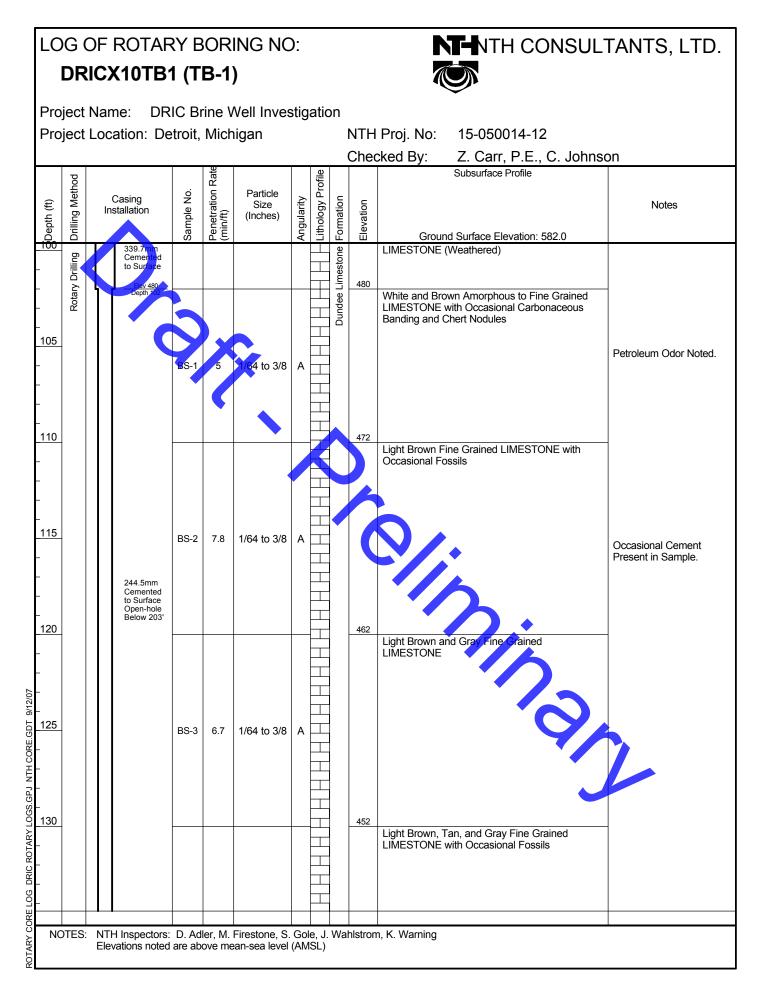
## **Appendix B**

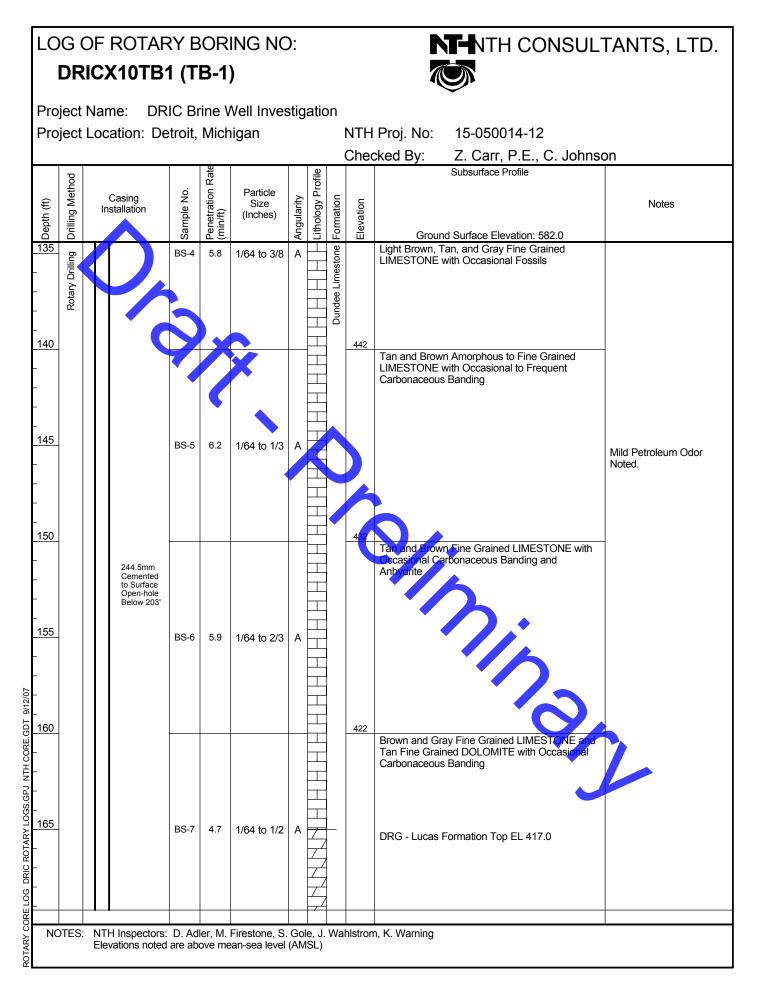
## Detroit River International Crossing Study Logs of Rotary Boring

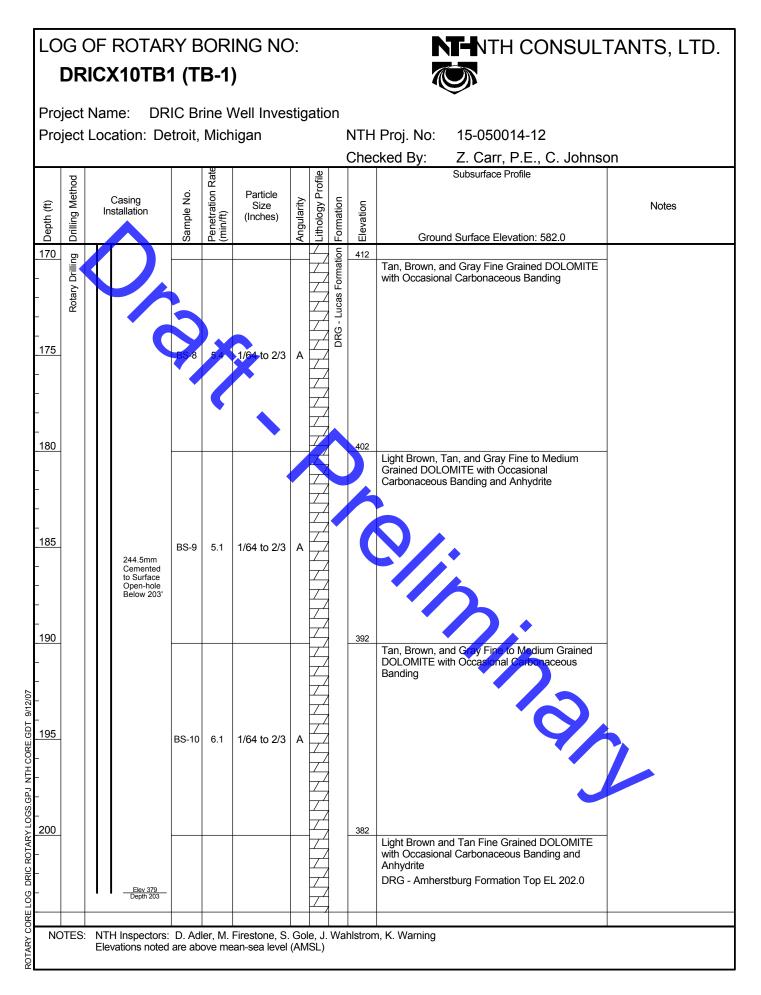


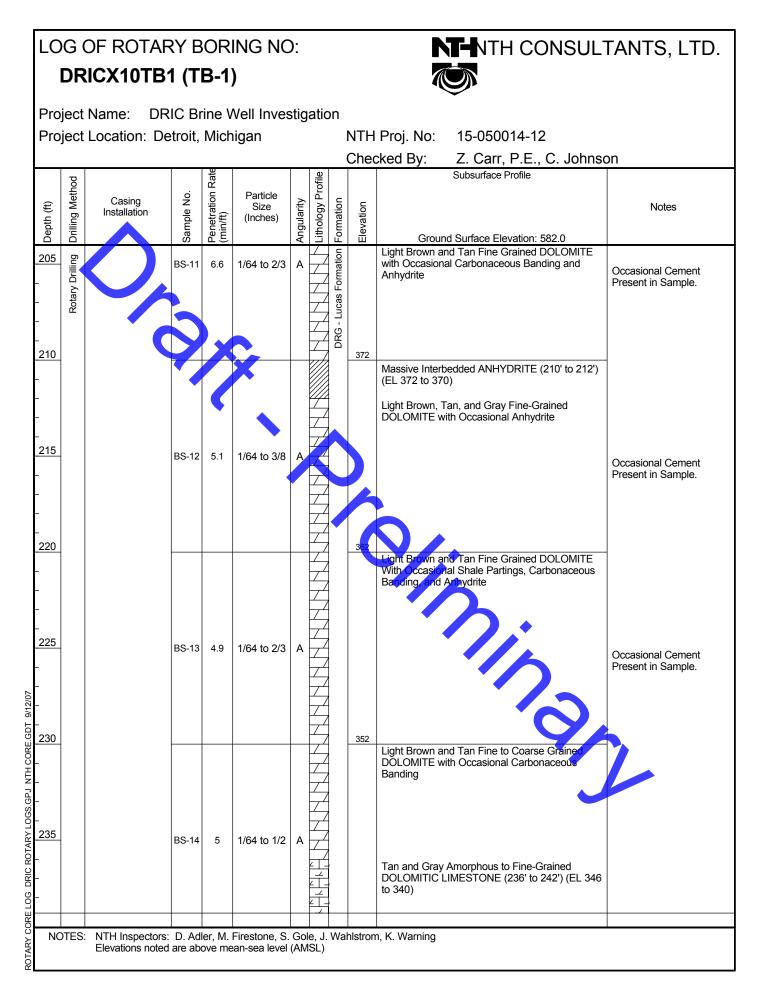


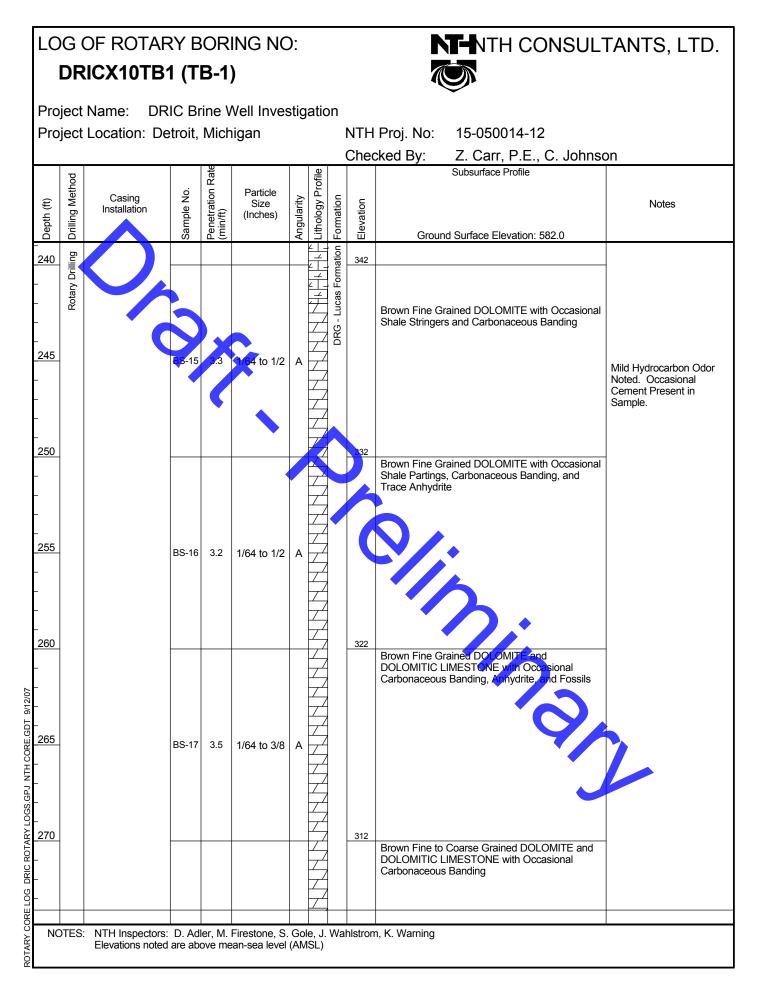


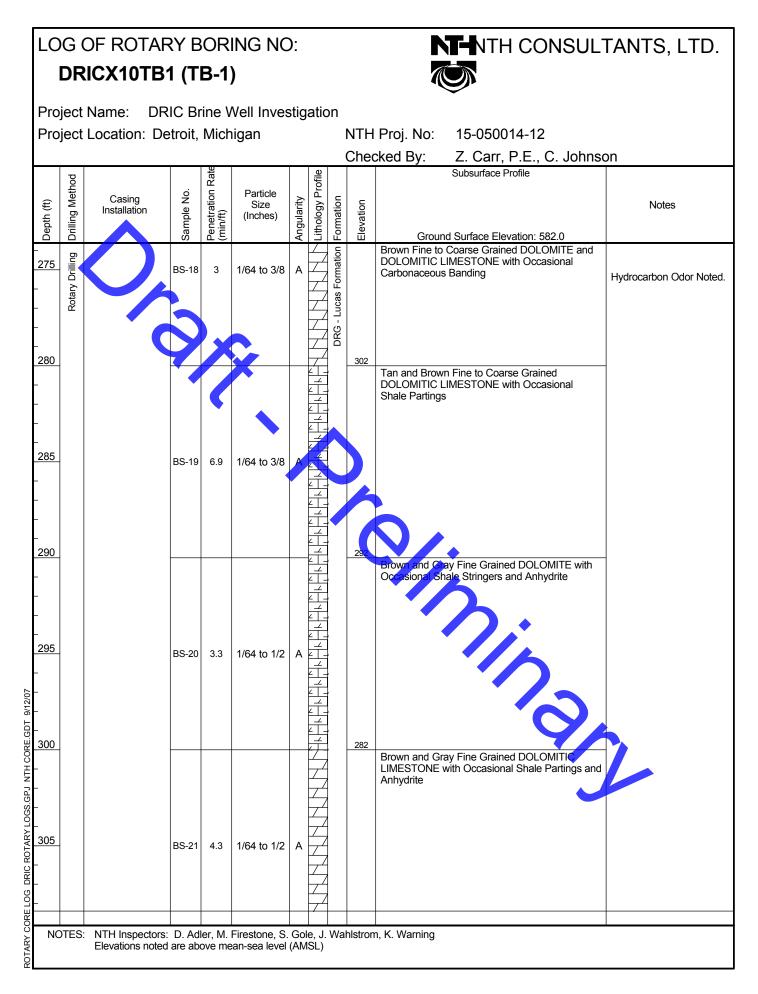


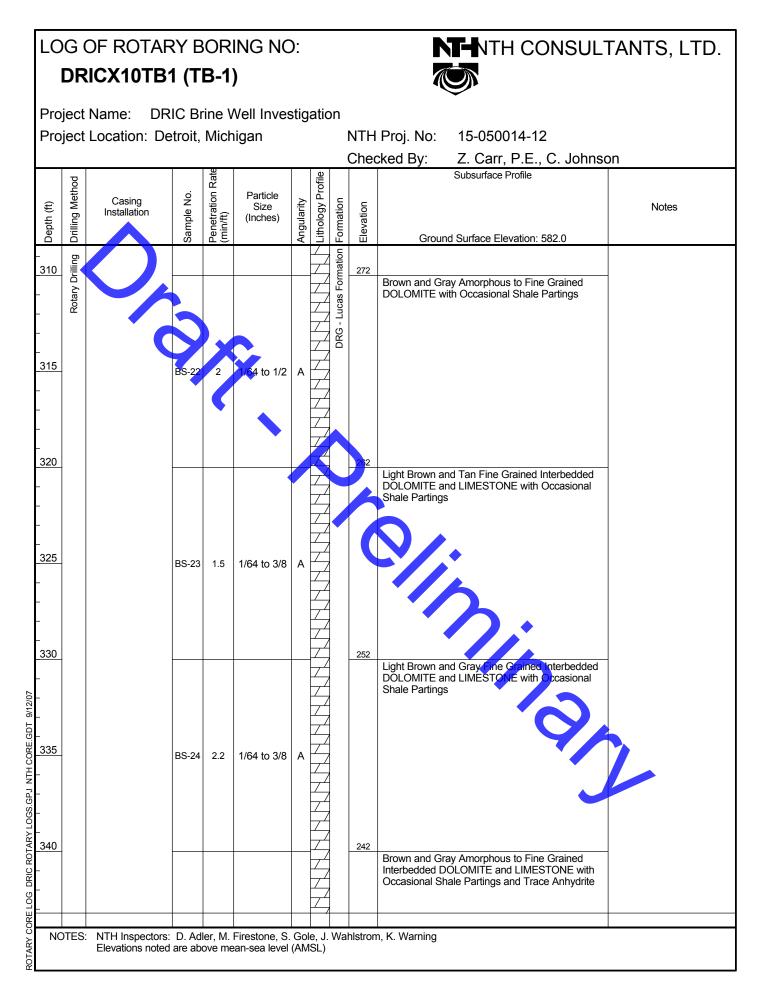


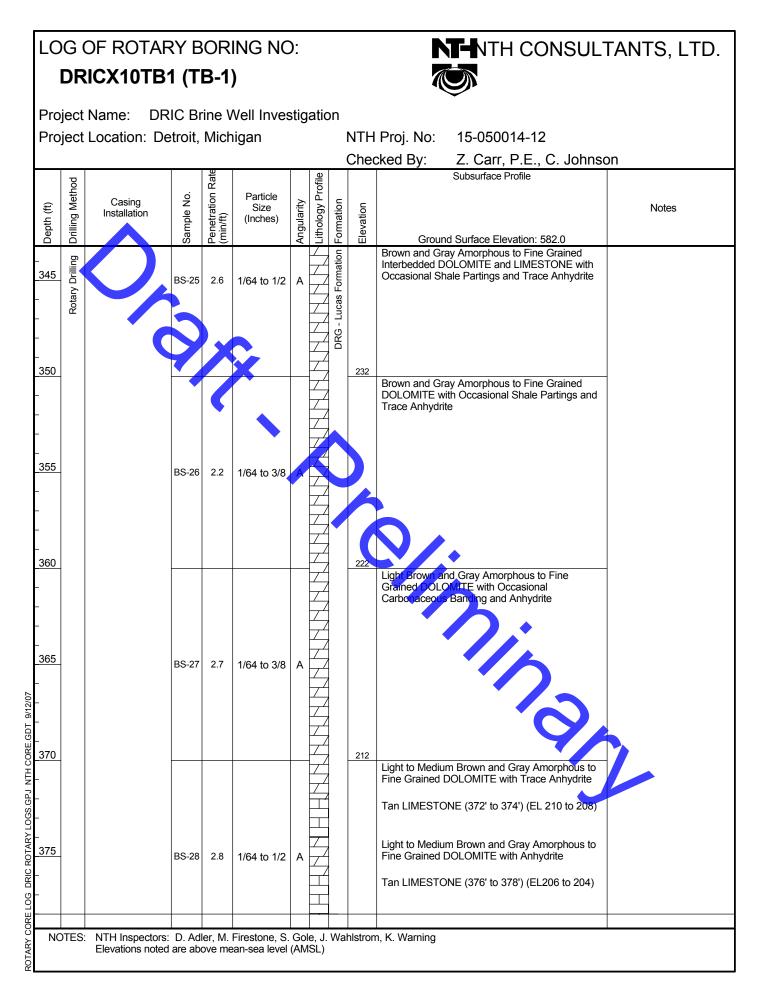


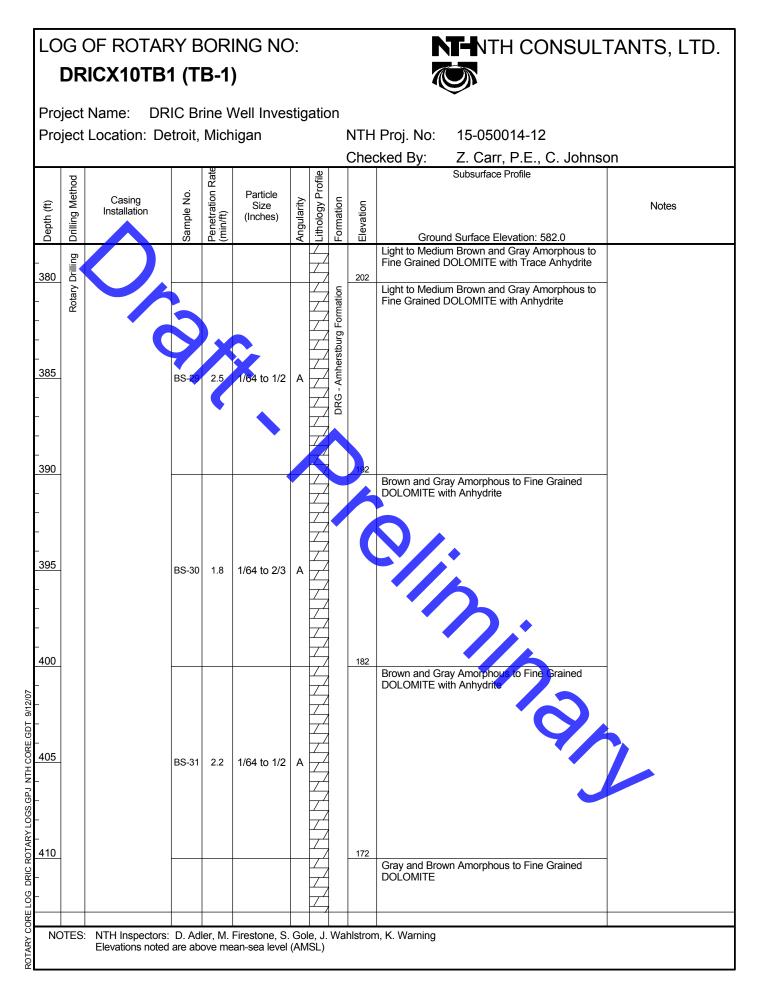


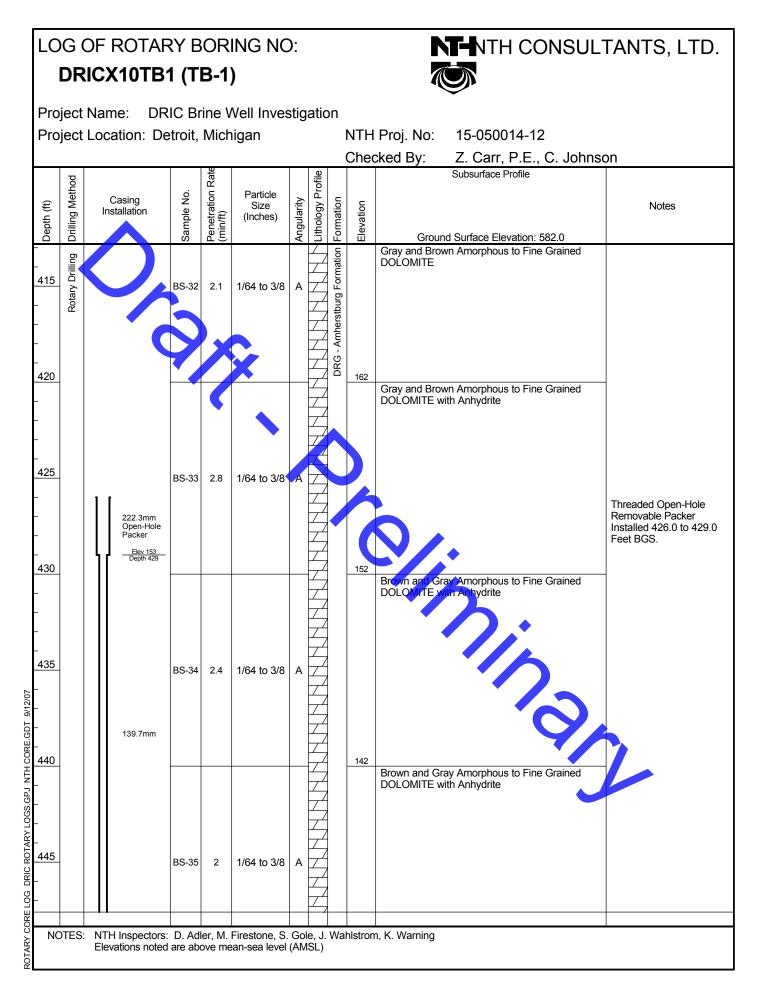


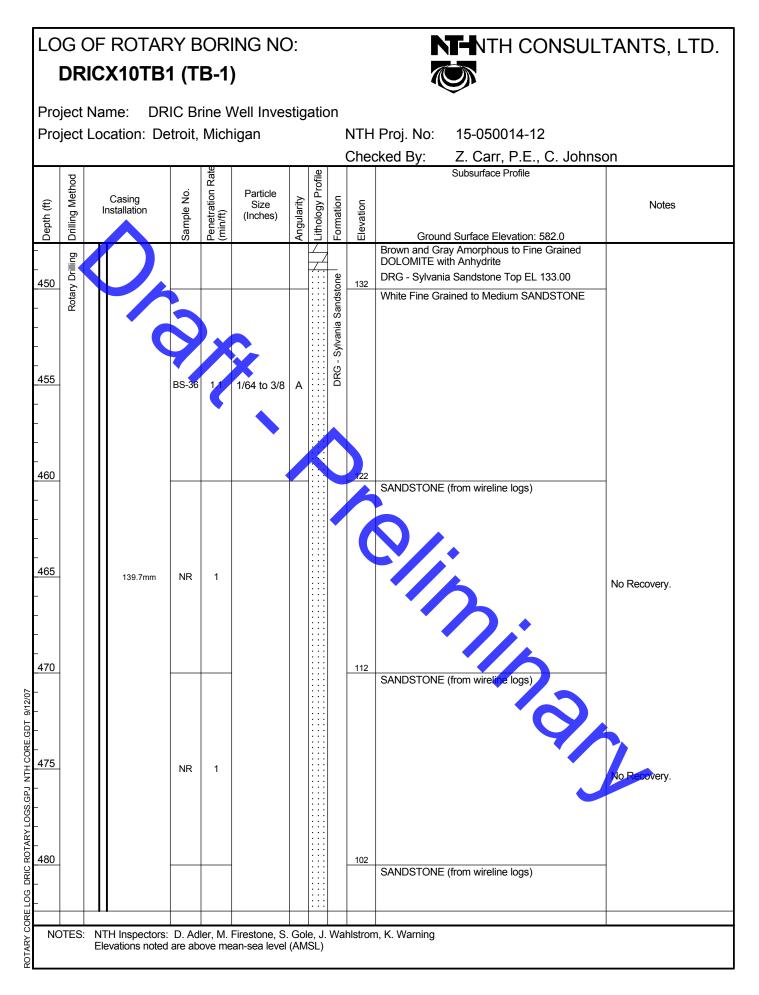


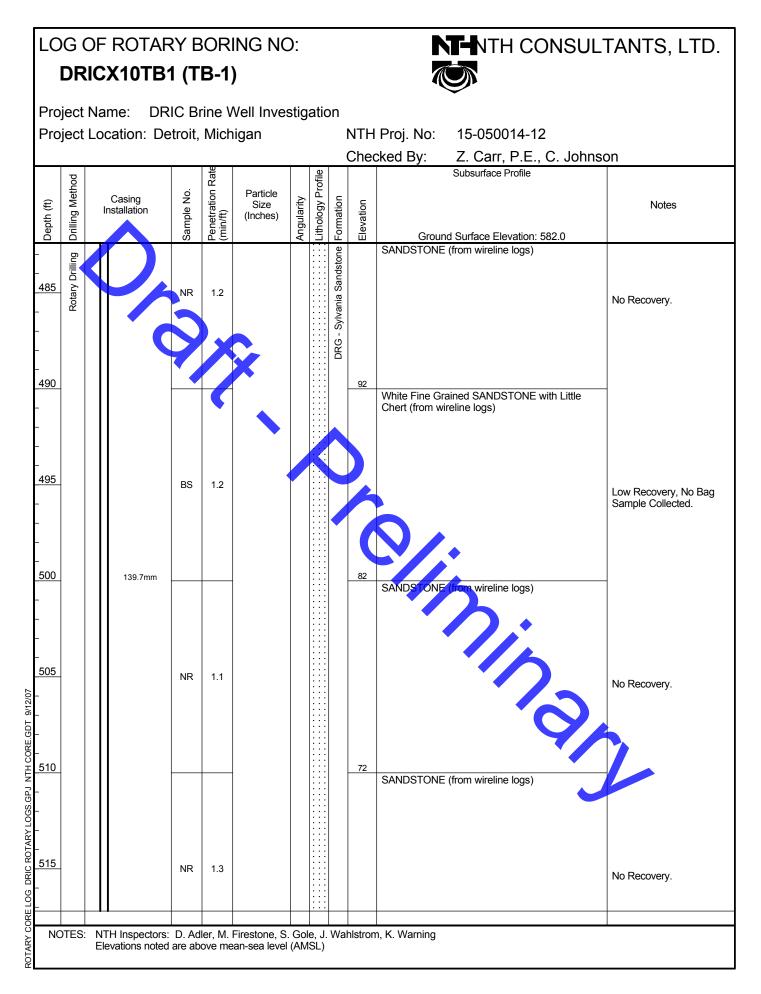


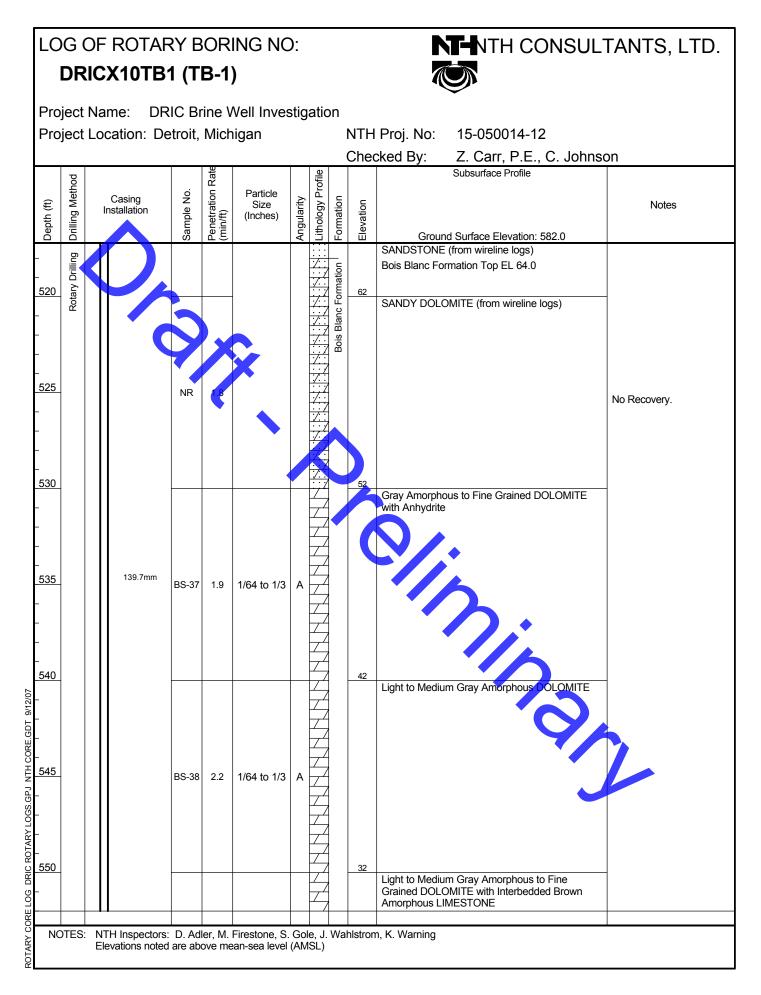


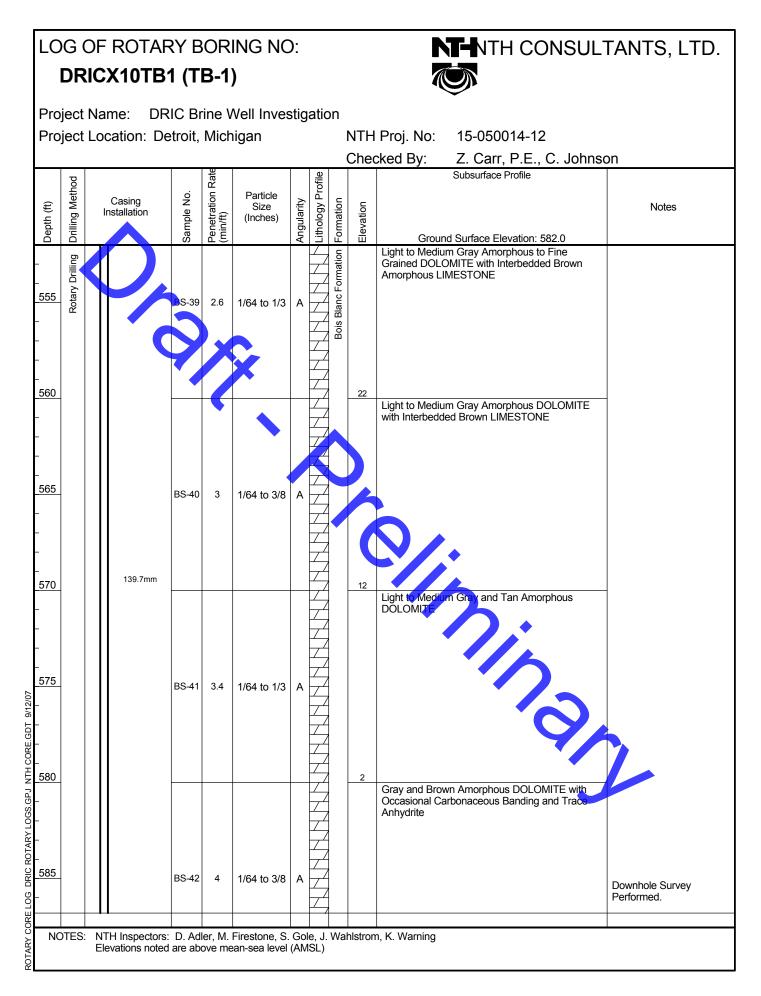


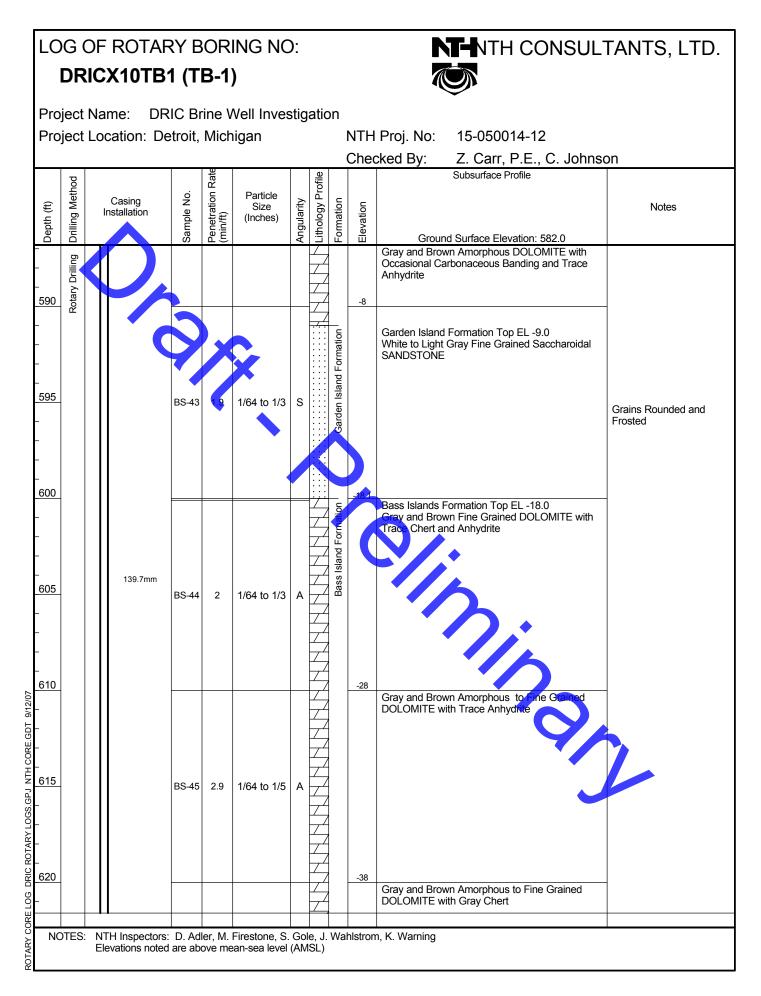


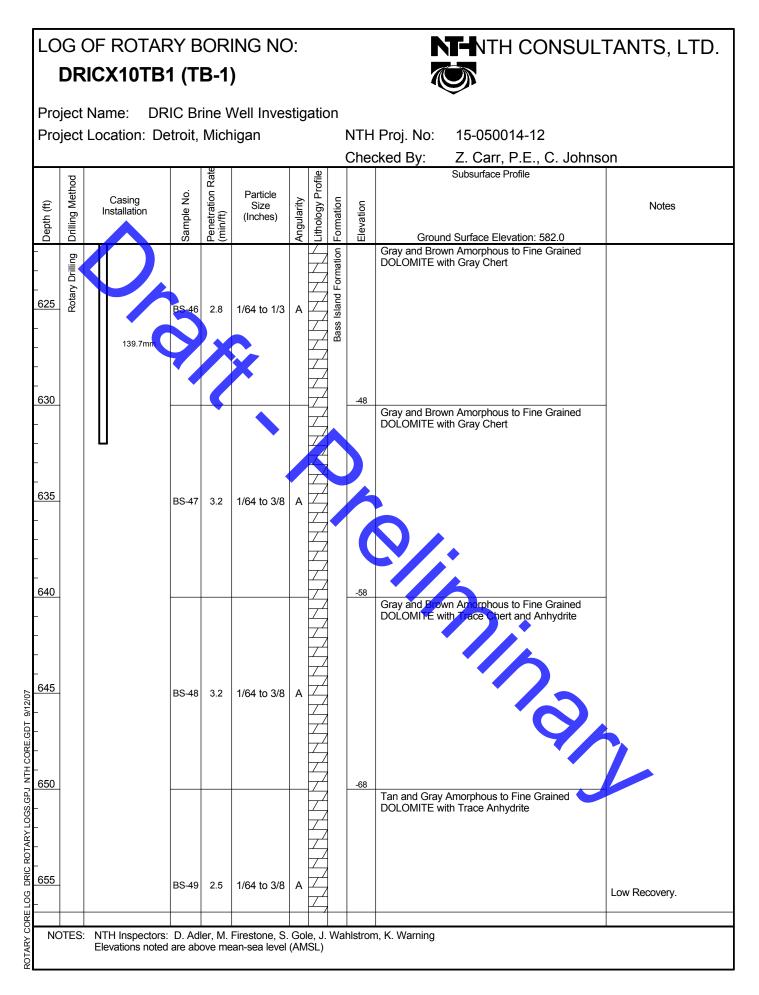


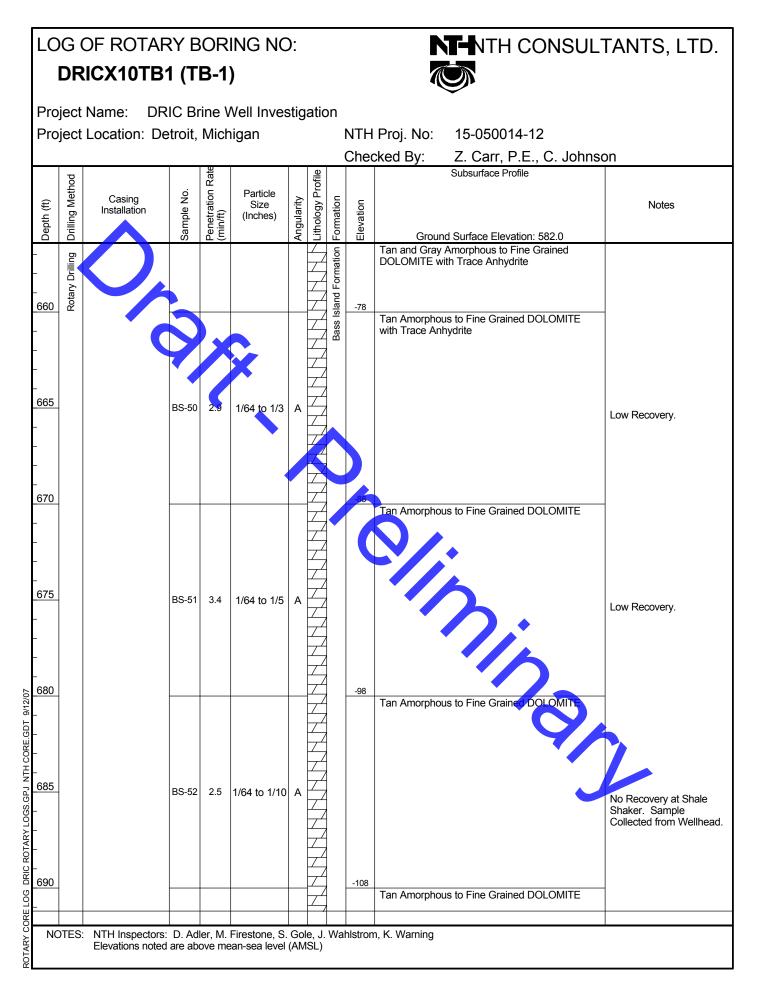


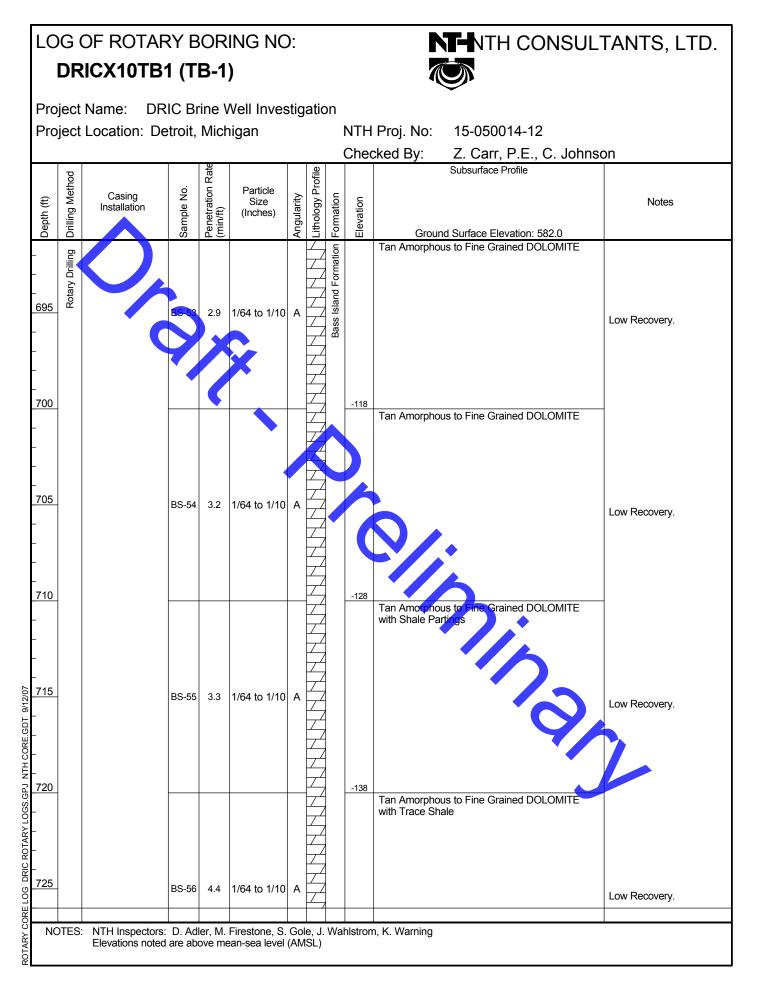


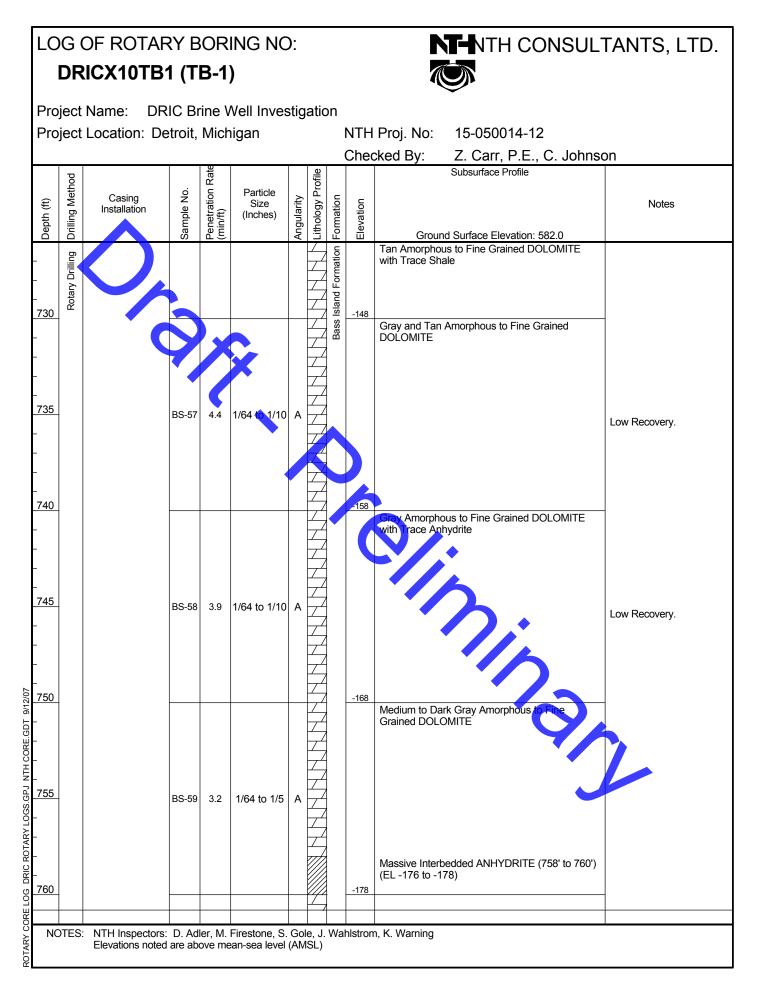


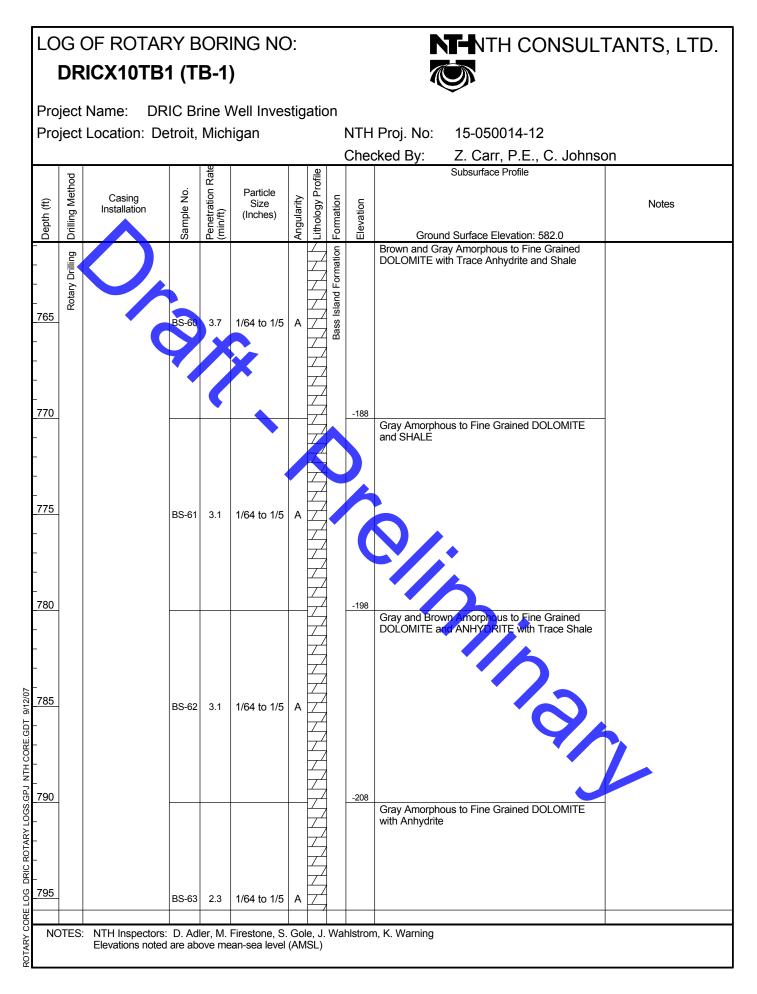


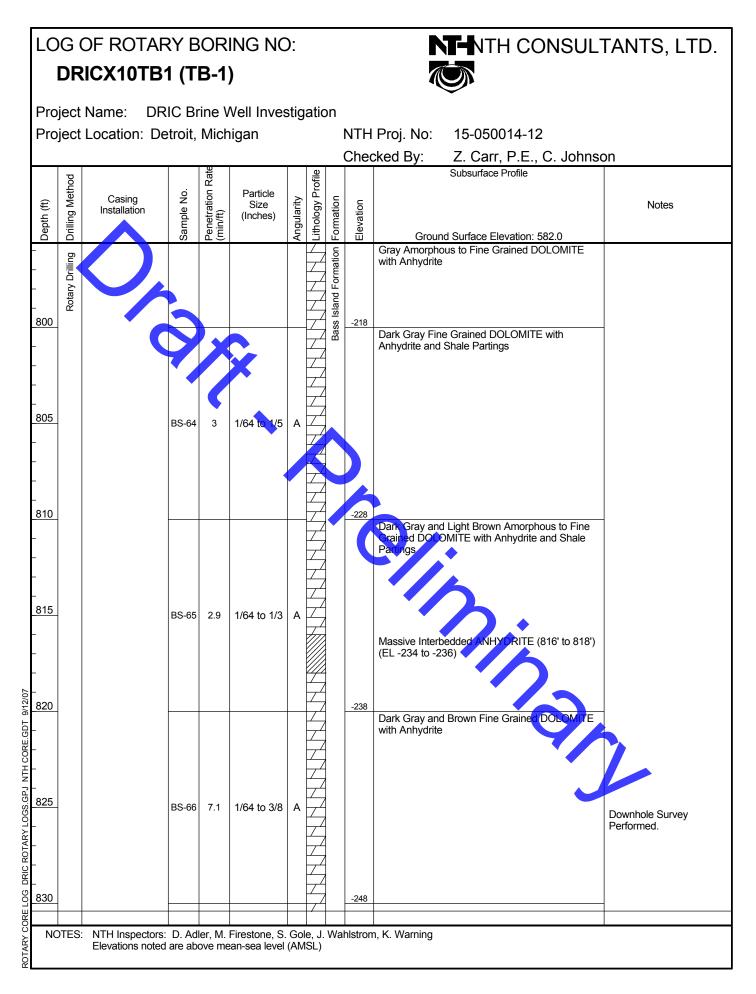


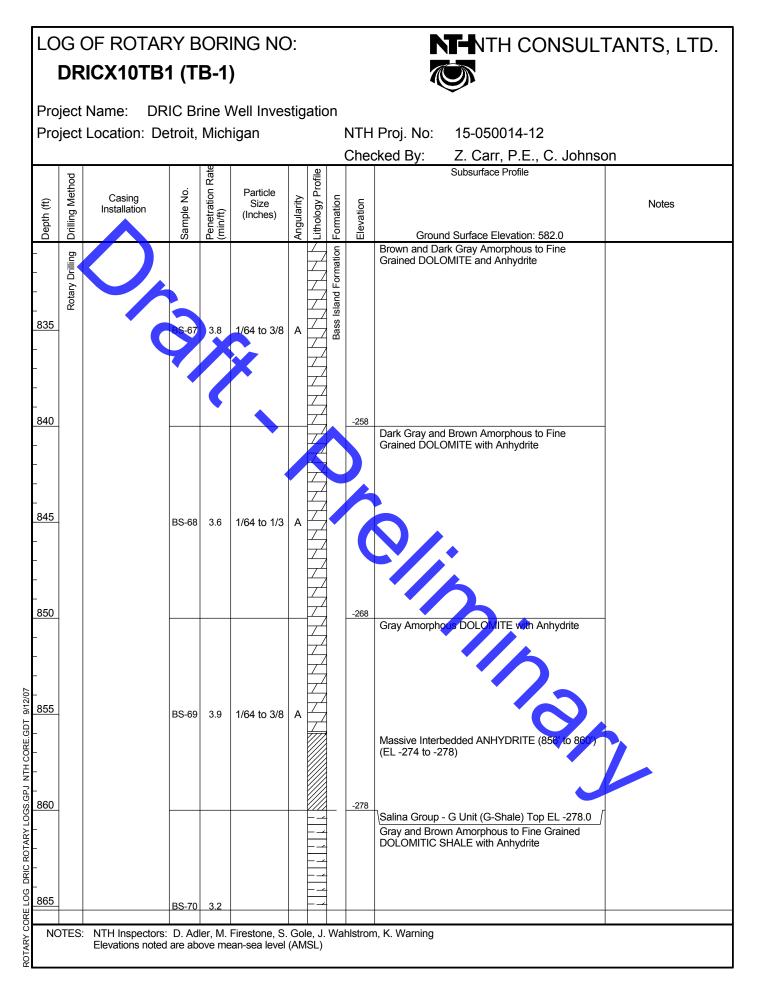


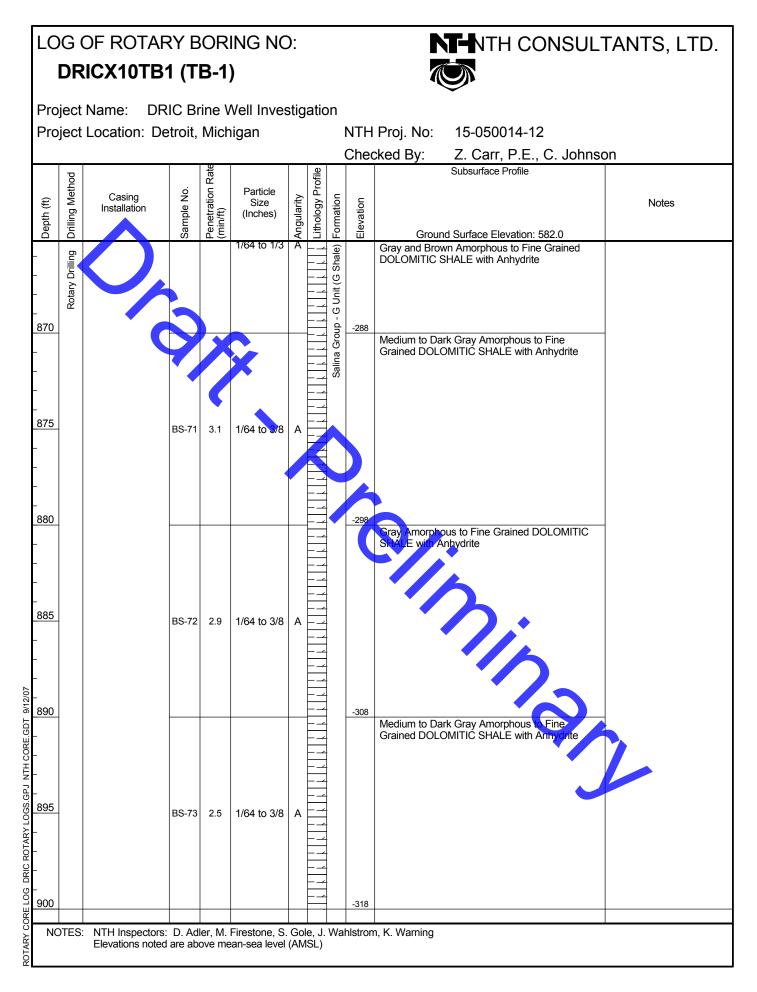


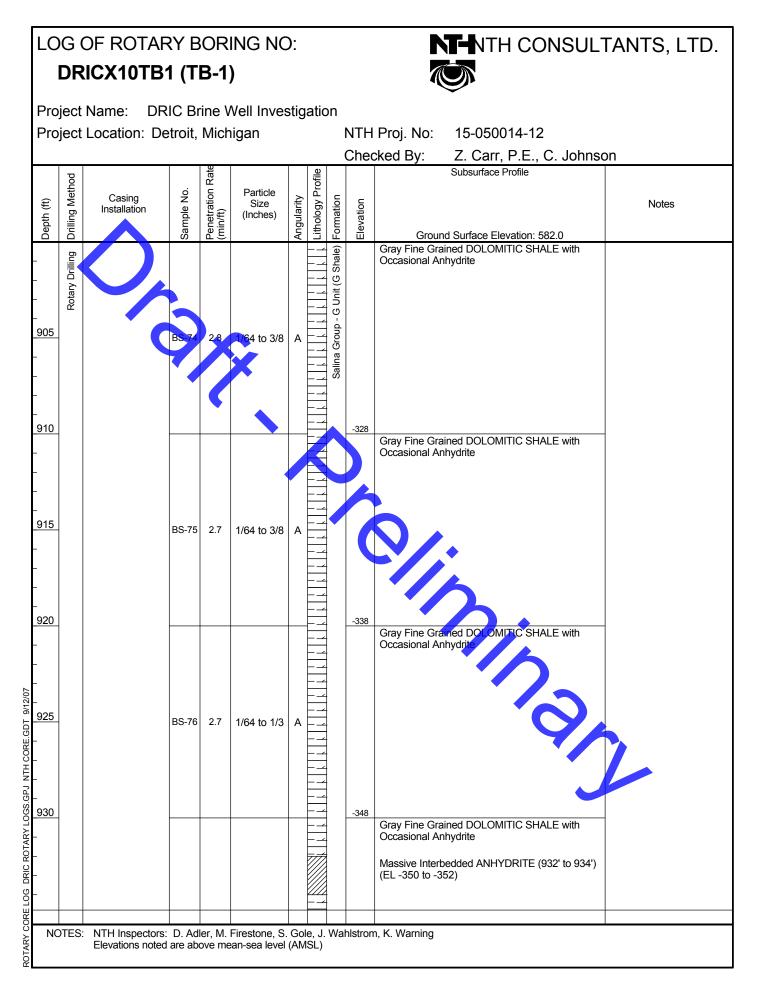


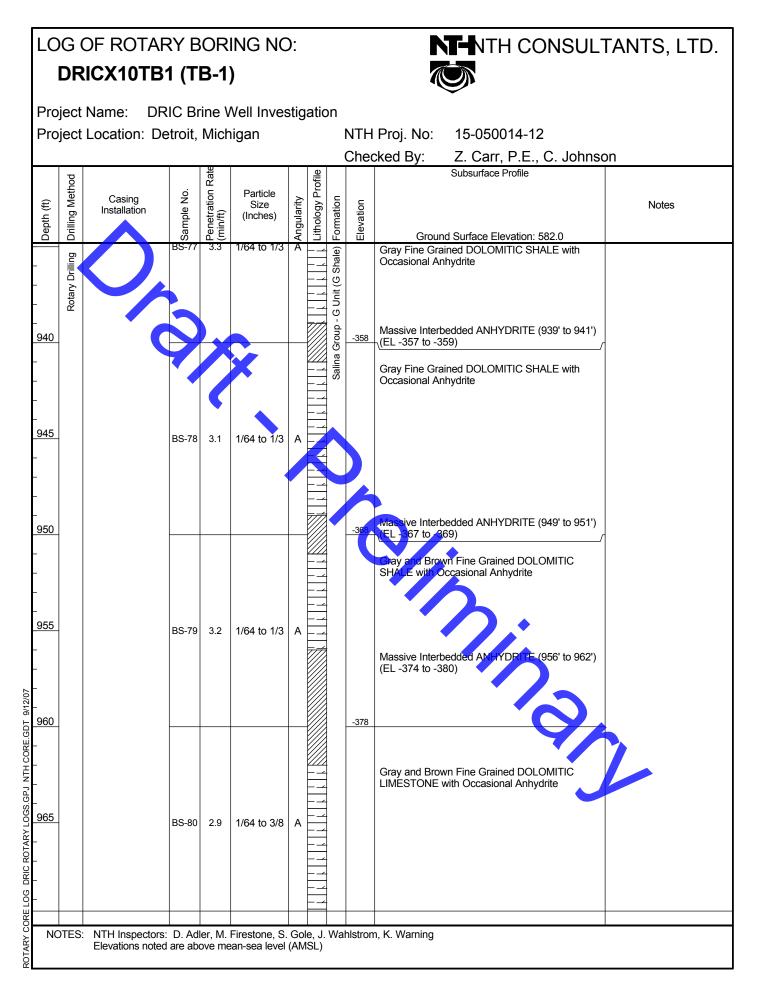


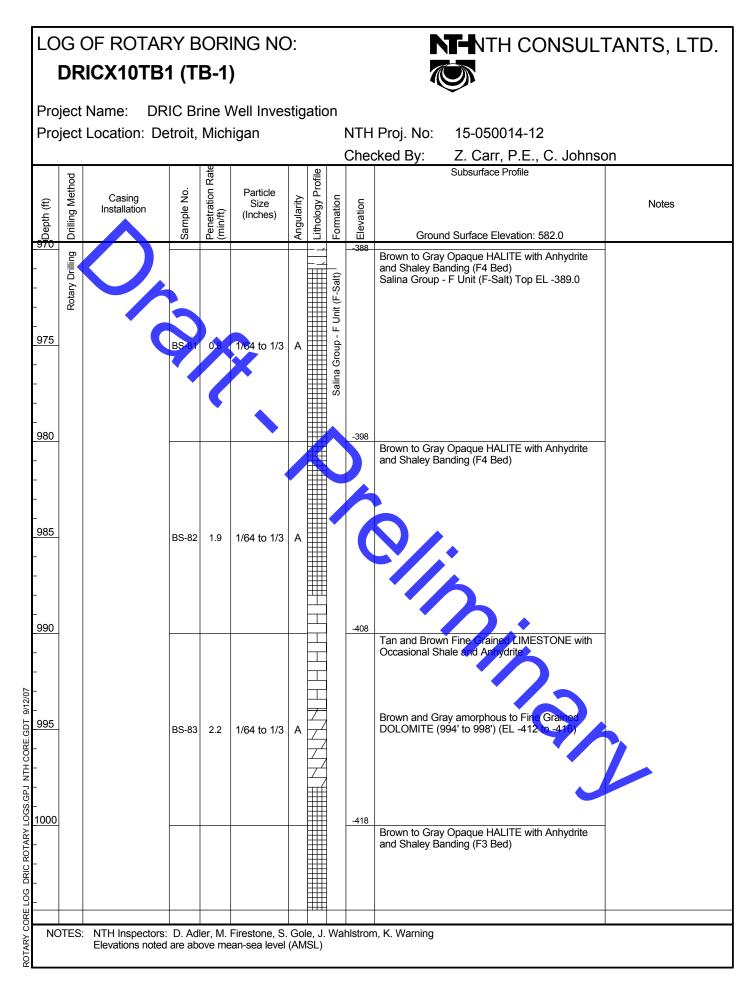


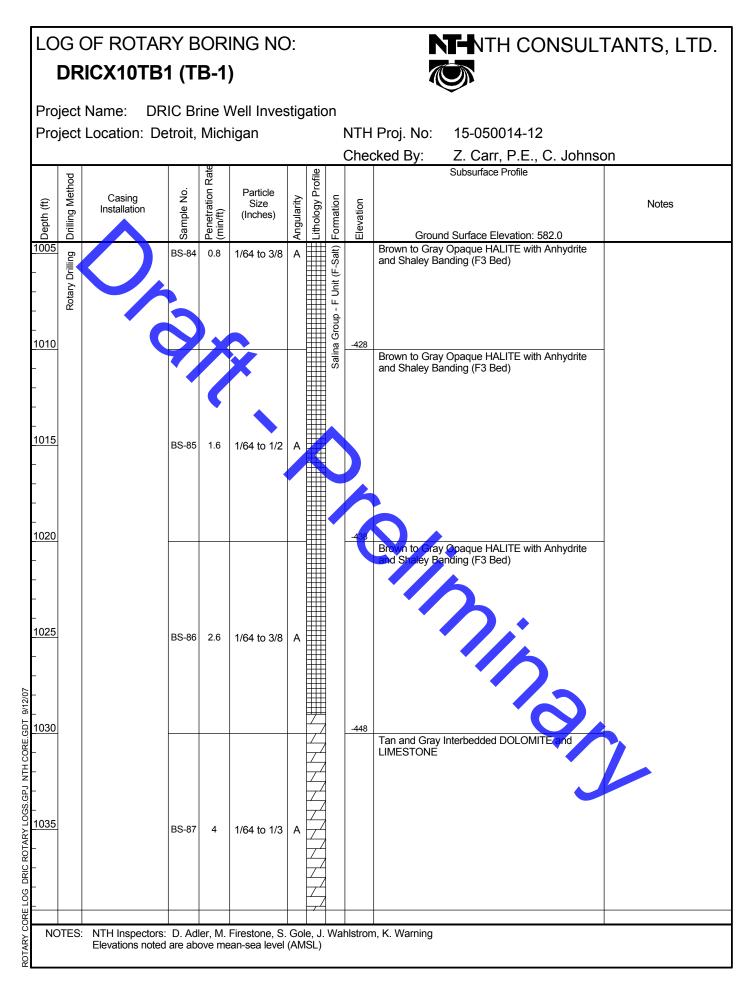


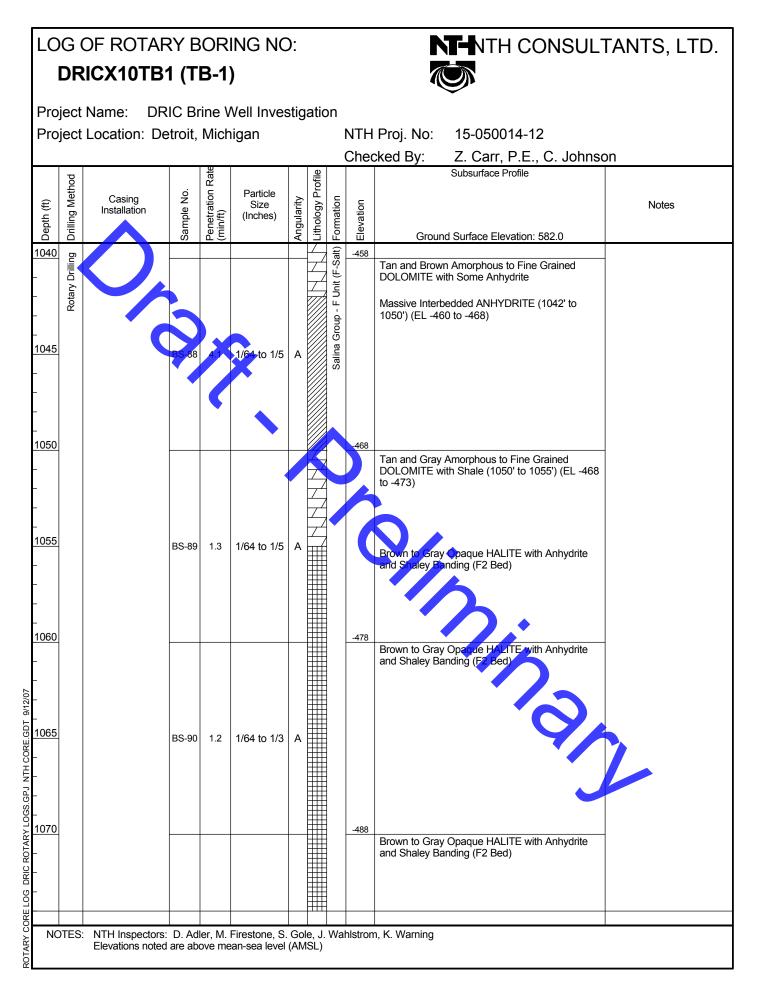


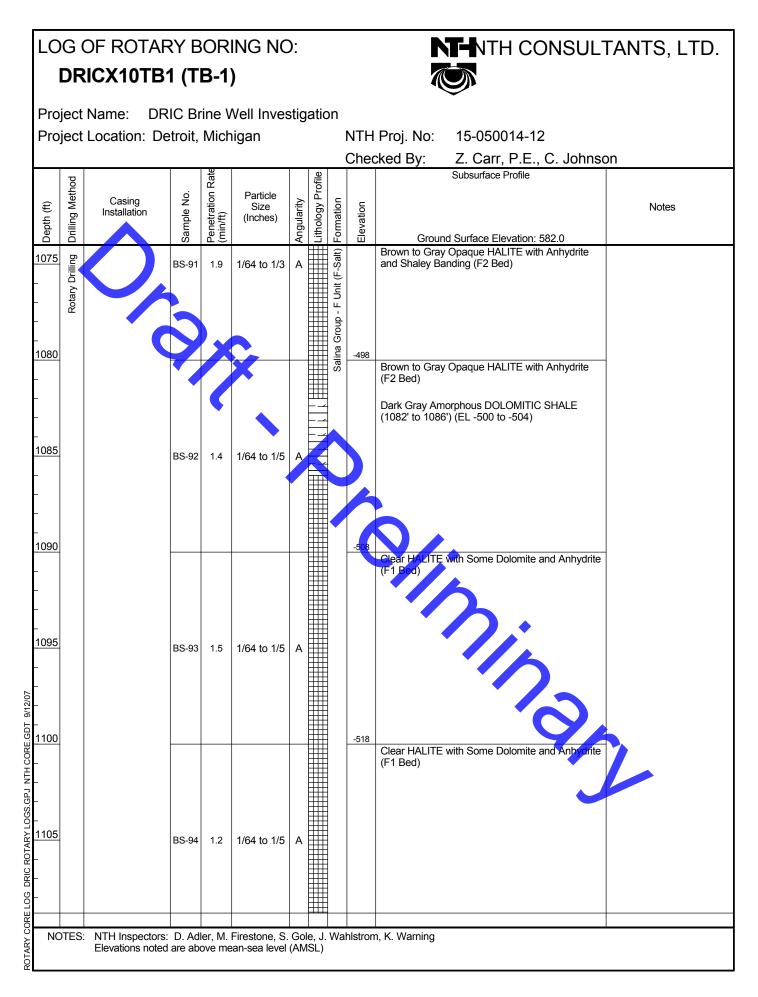


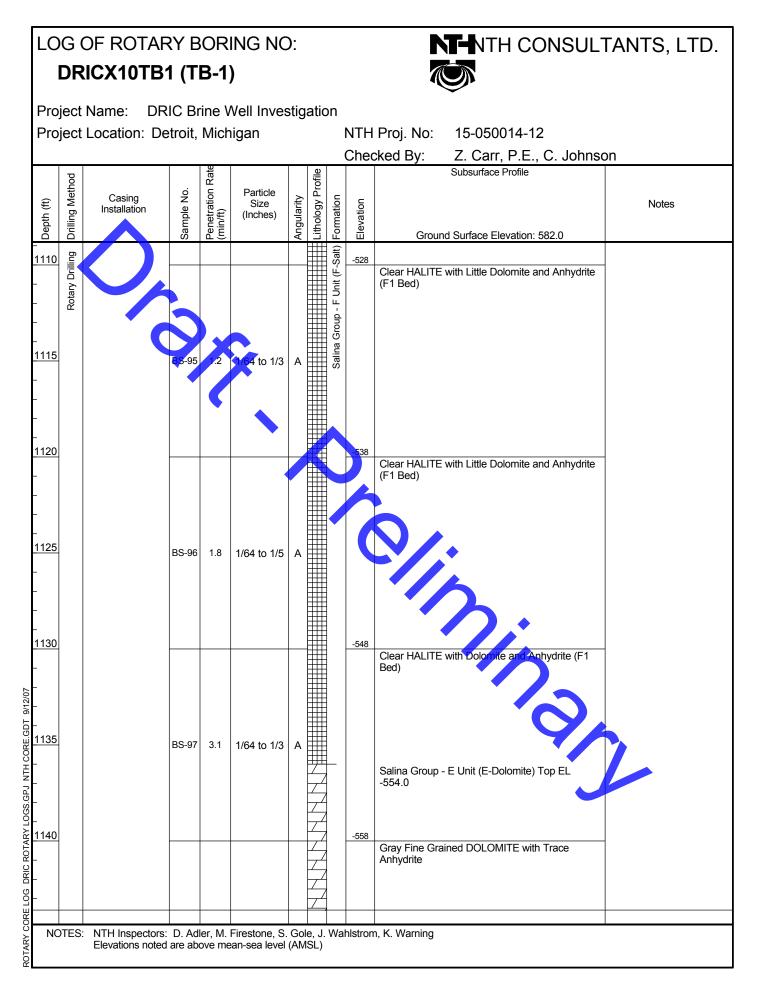


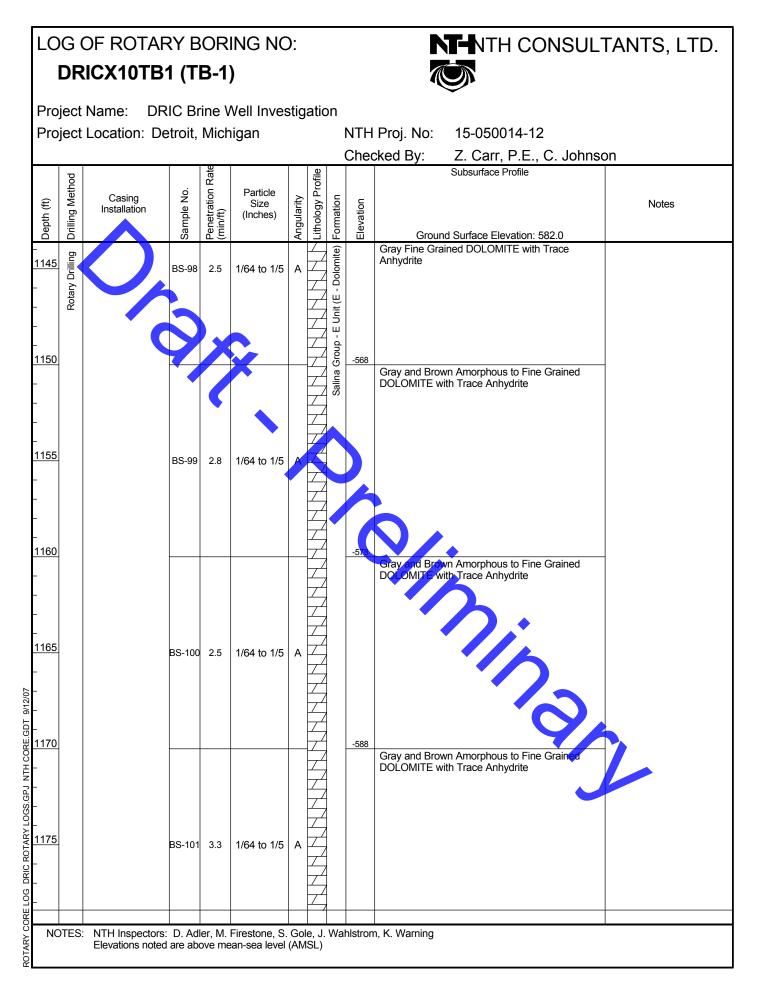


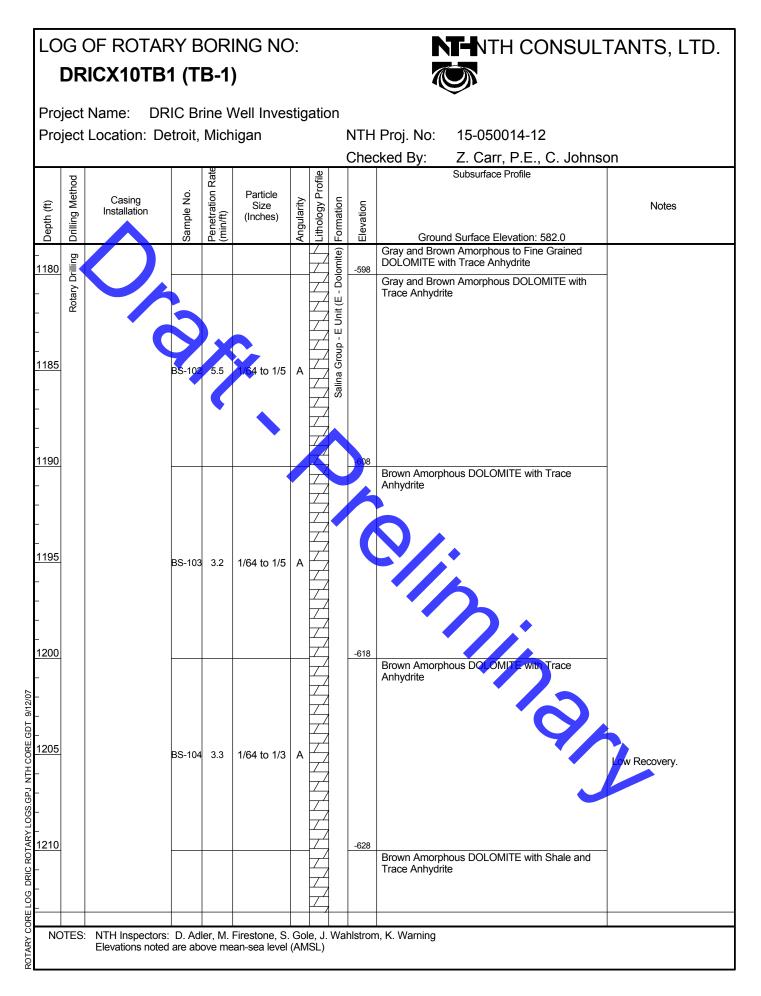


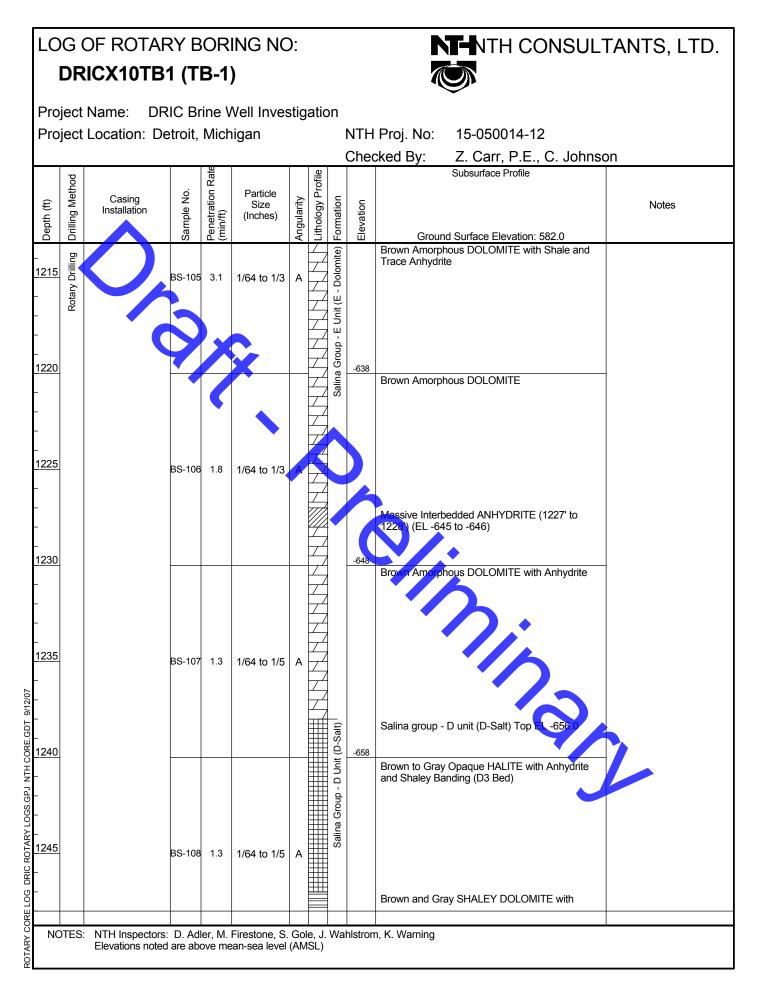


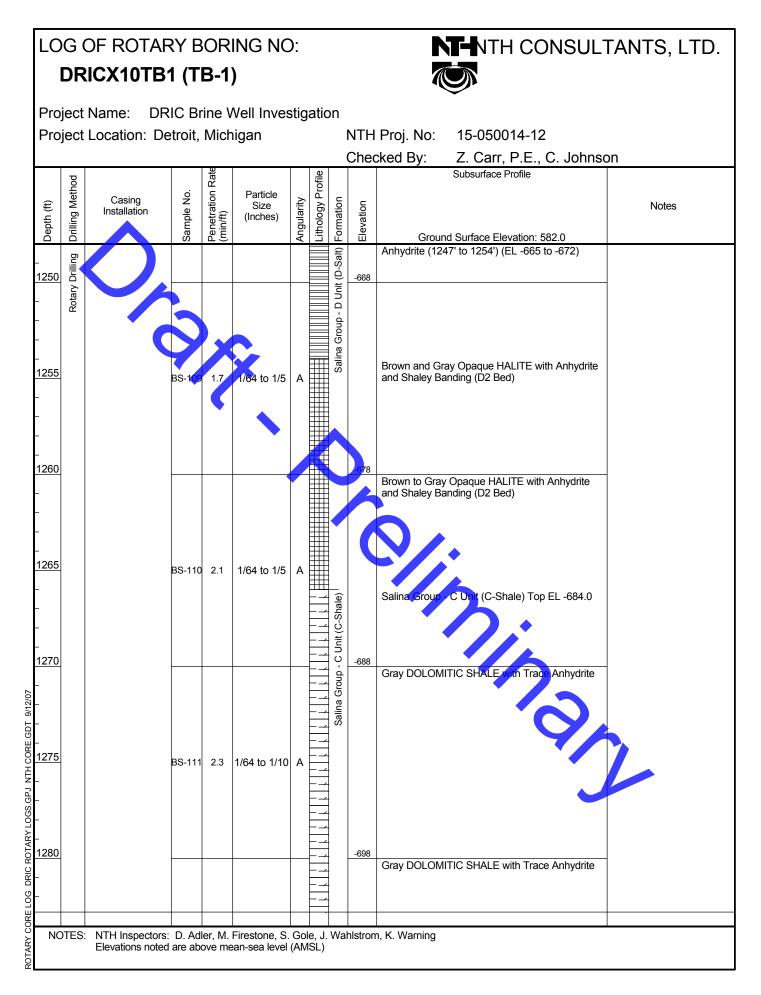


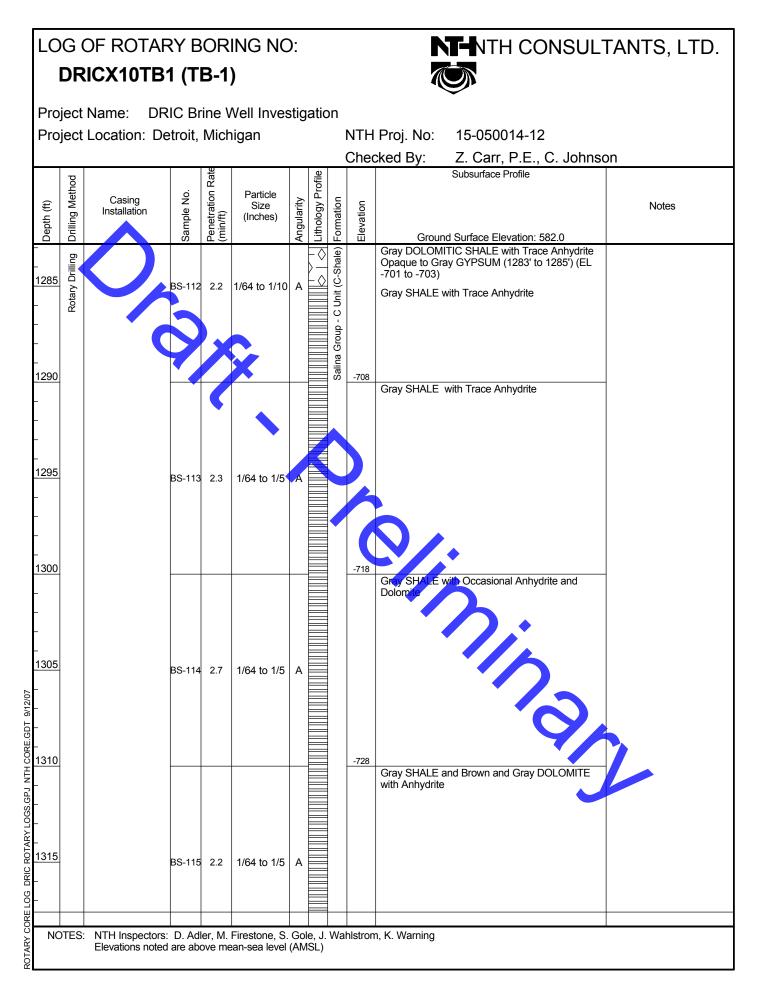


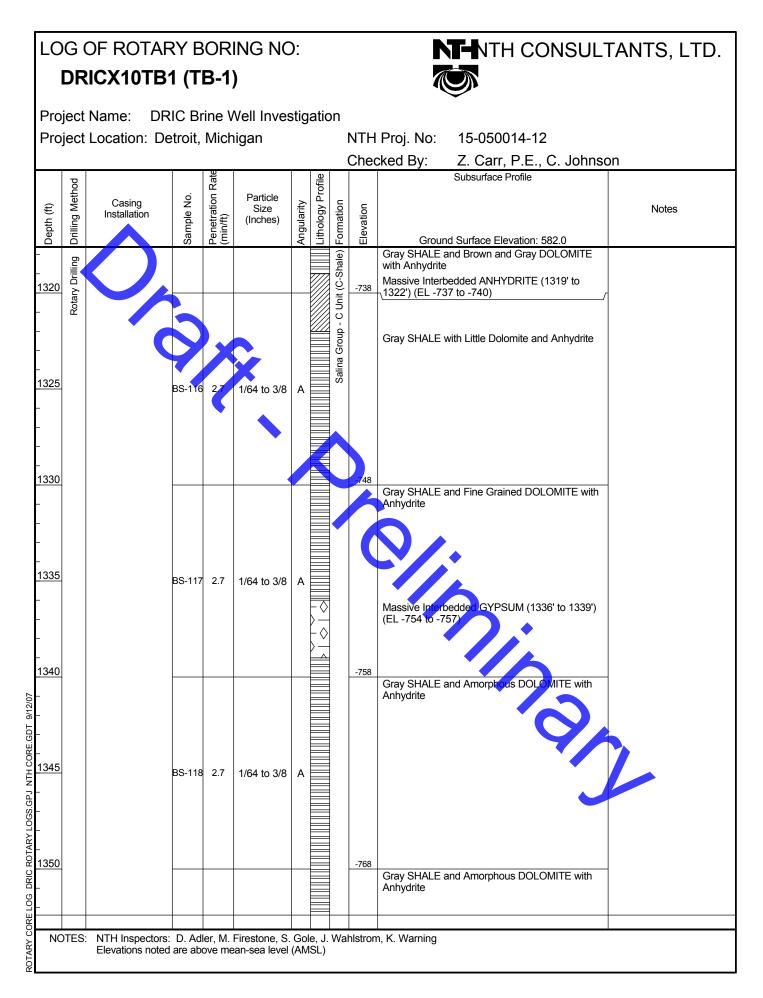


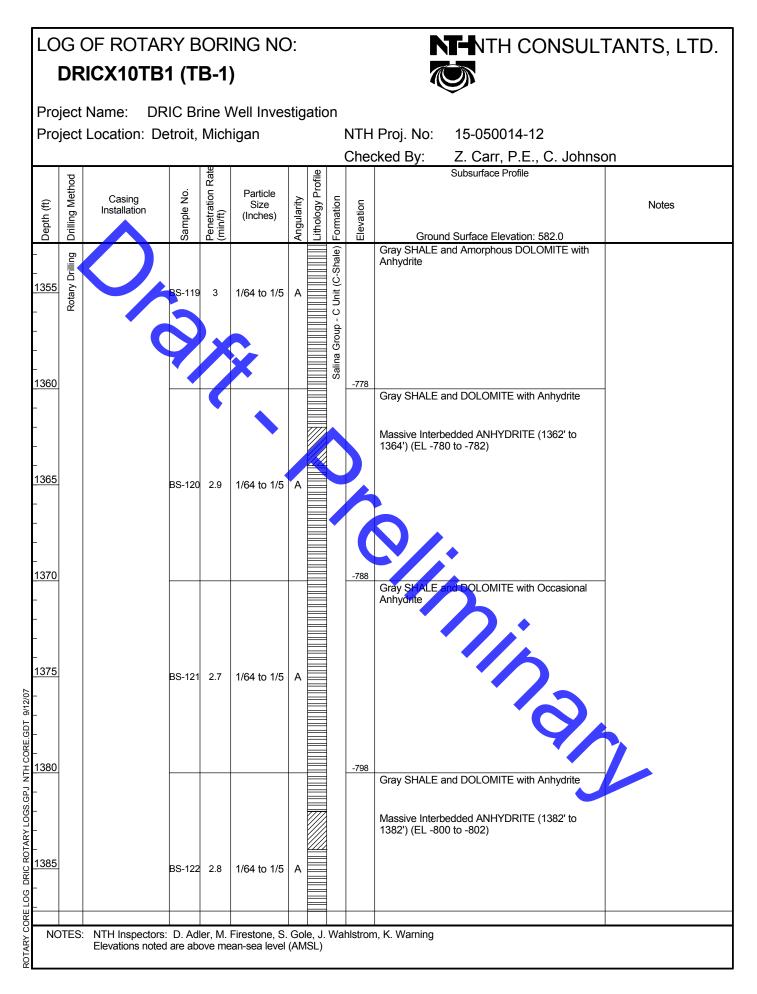


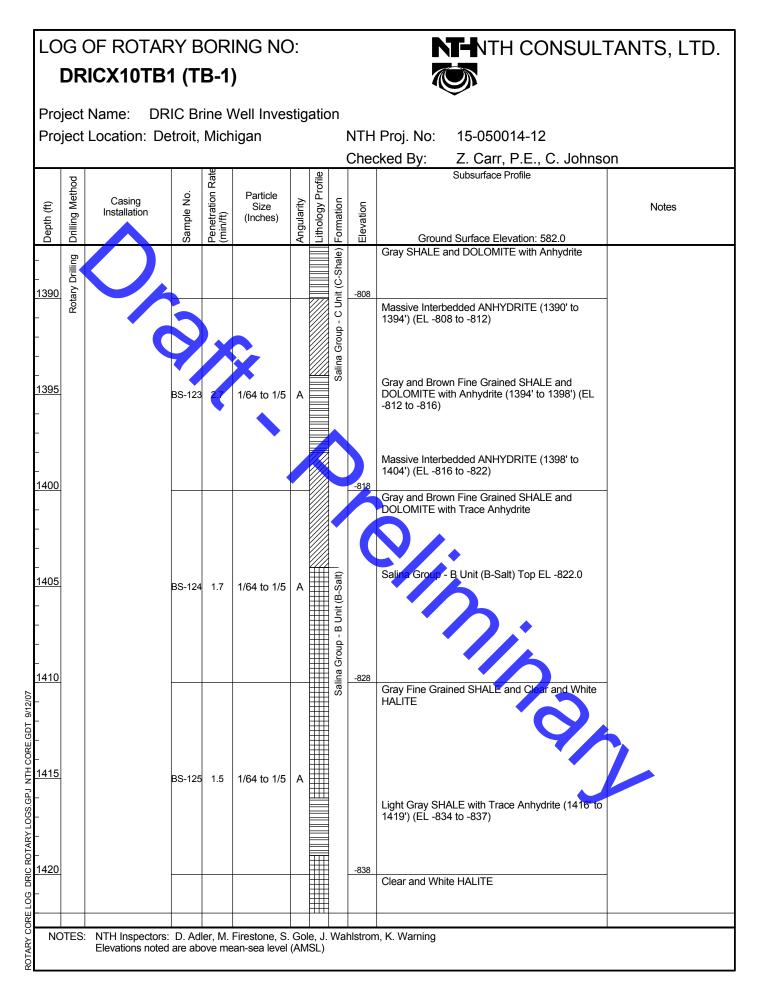


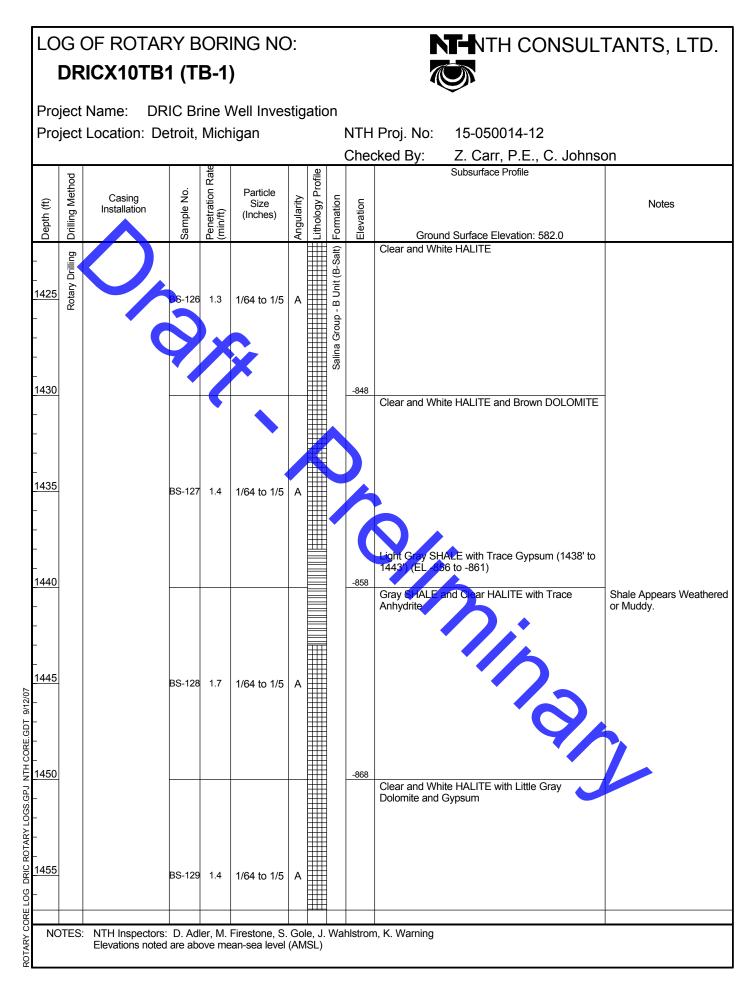


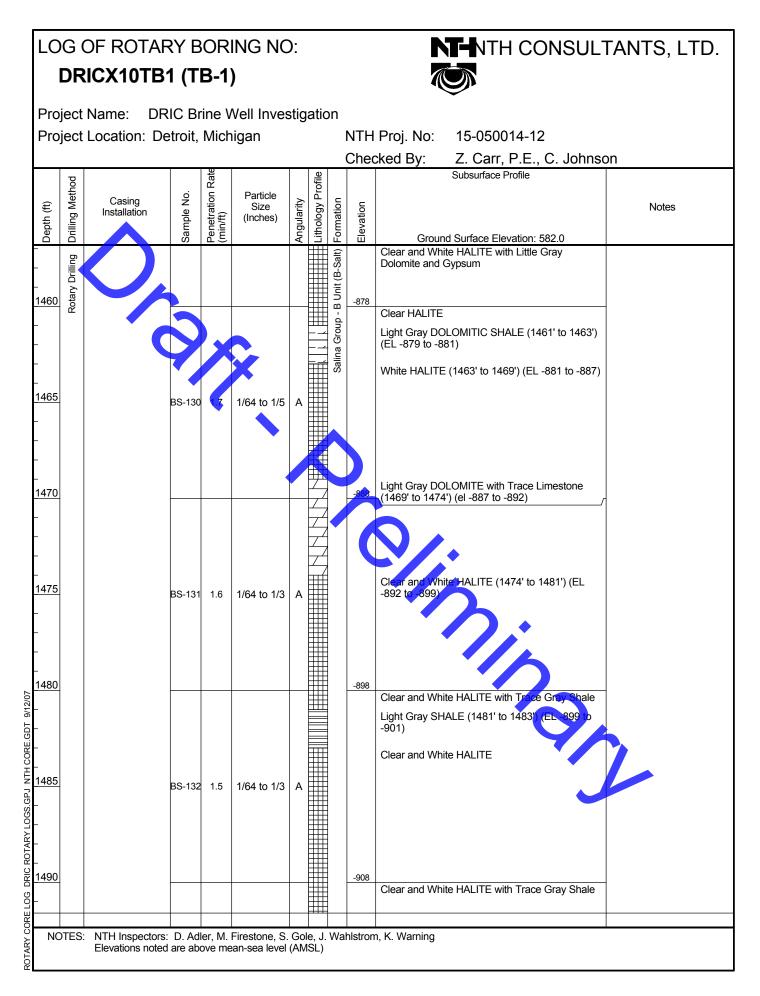


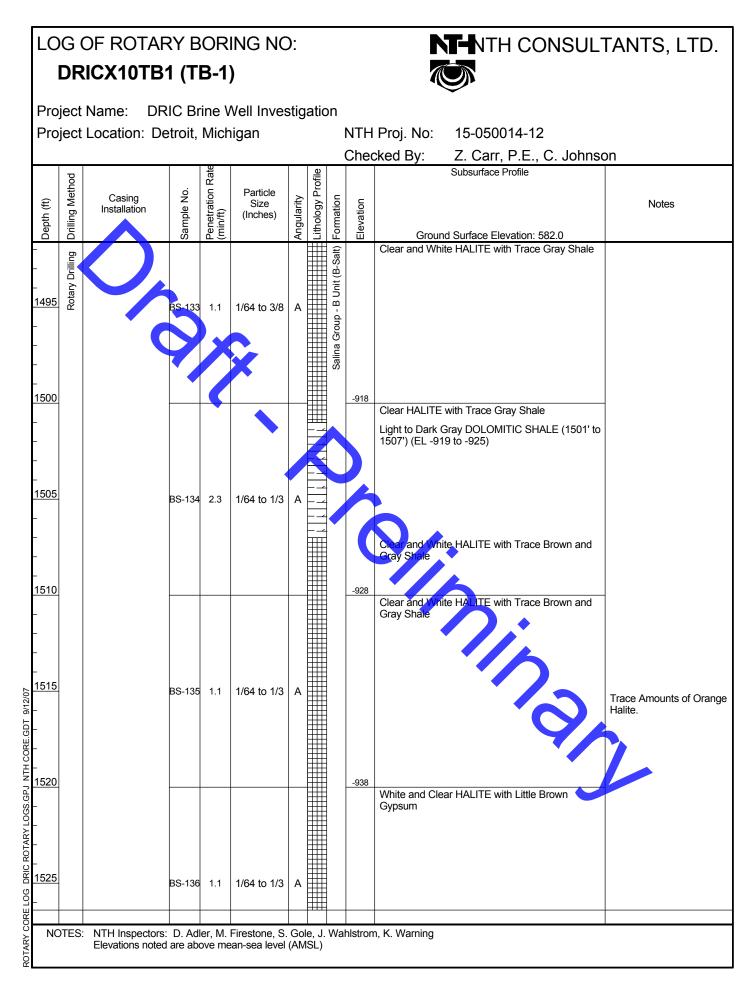


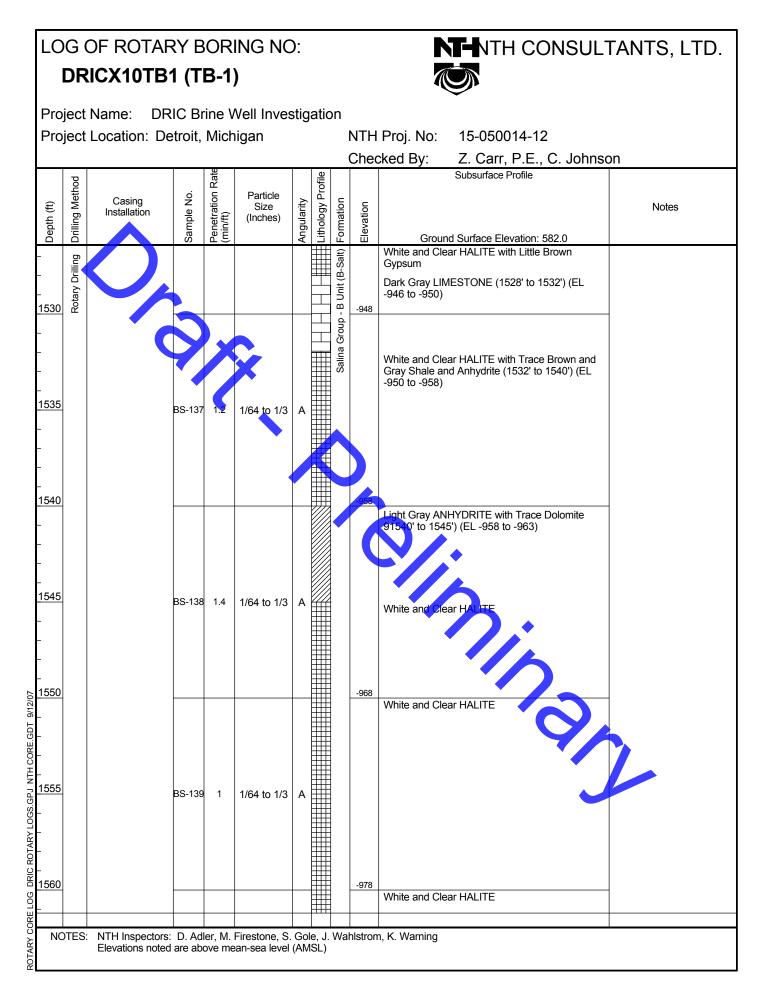


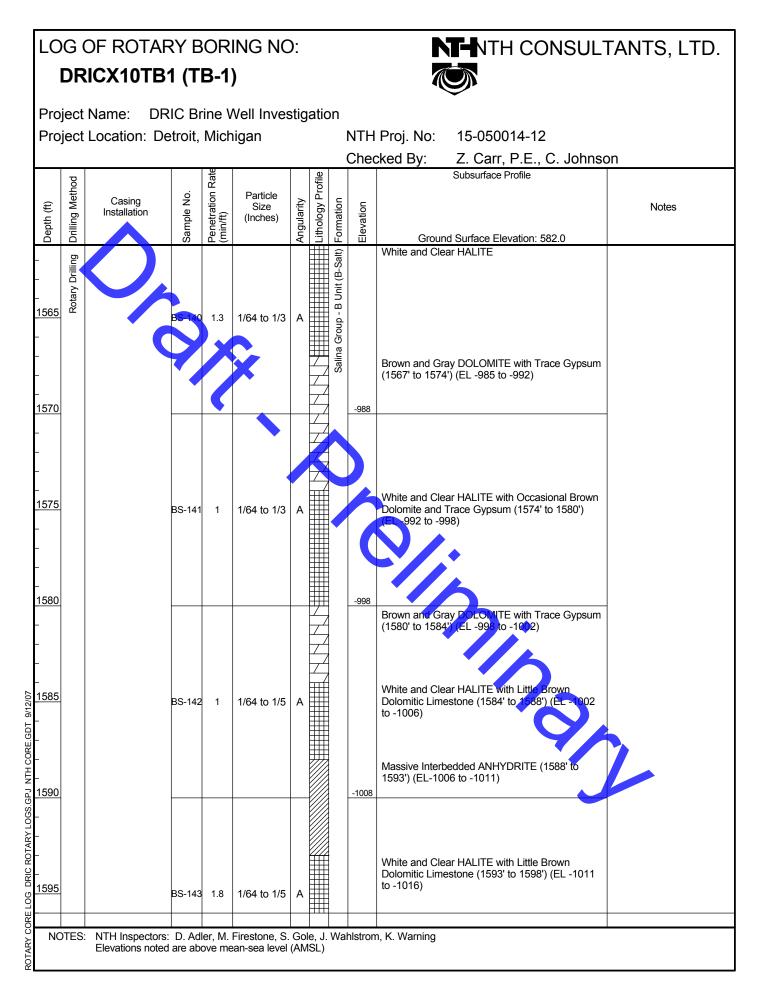


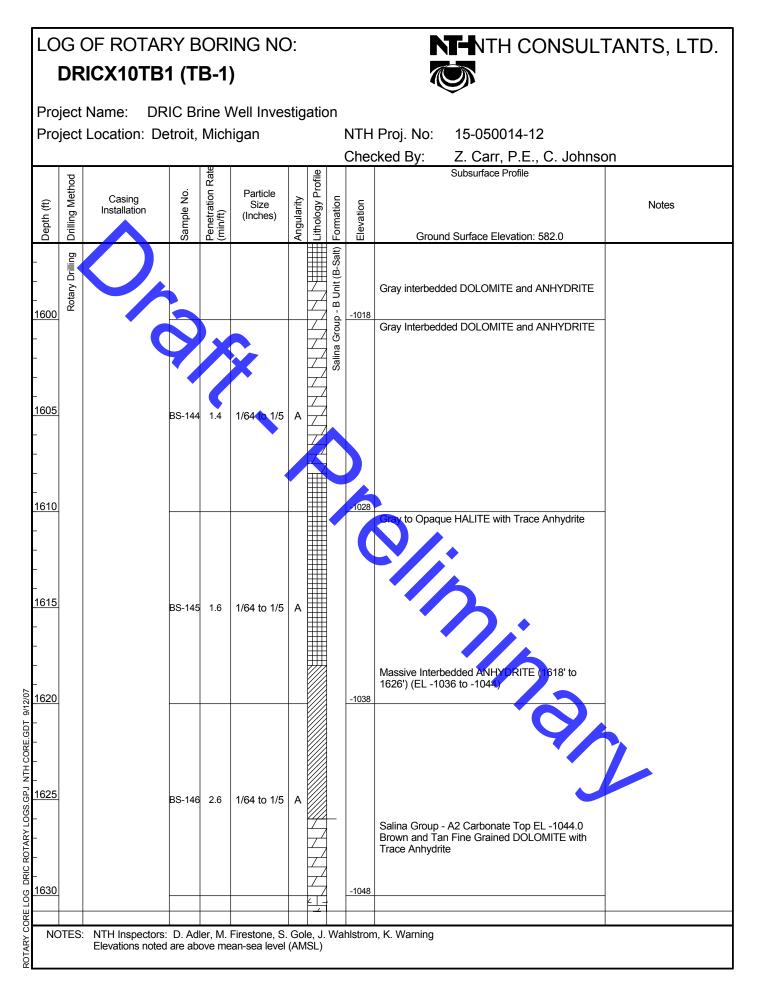


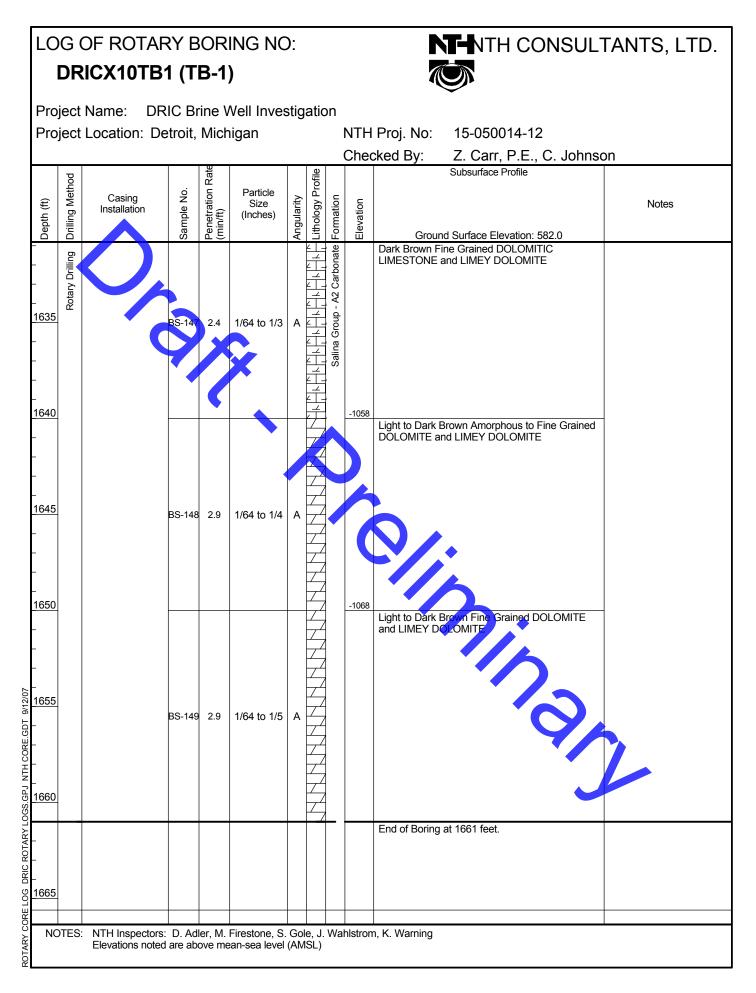


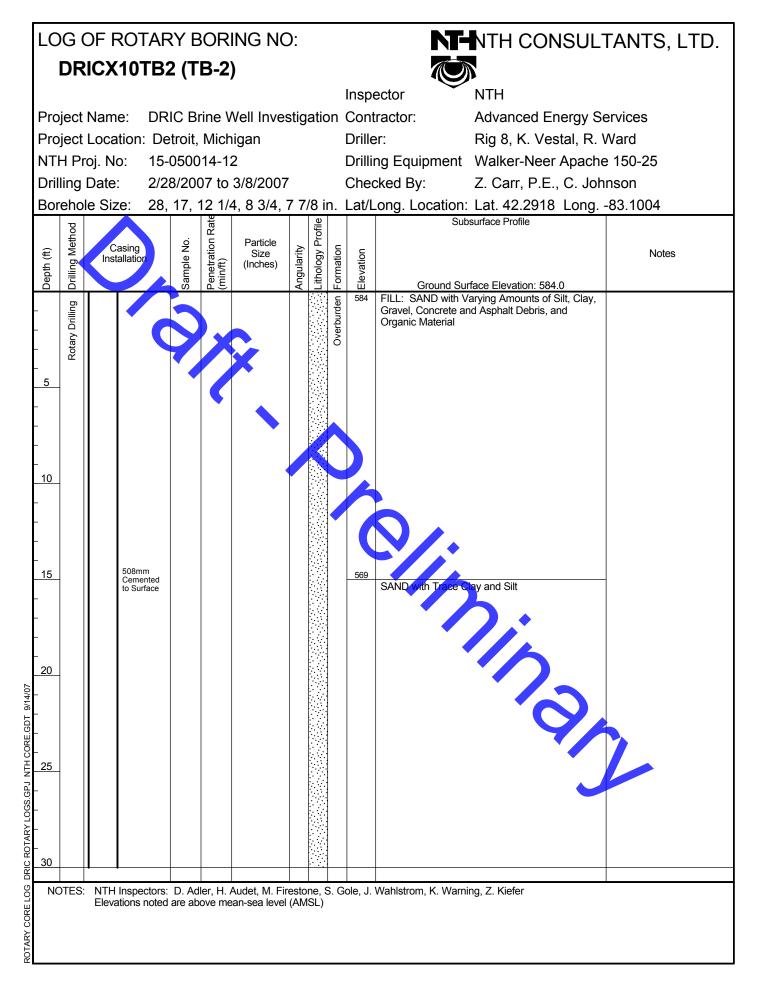


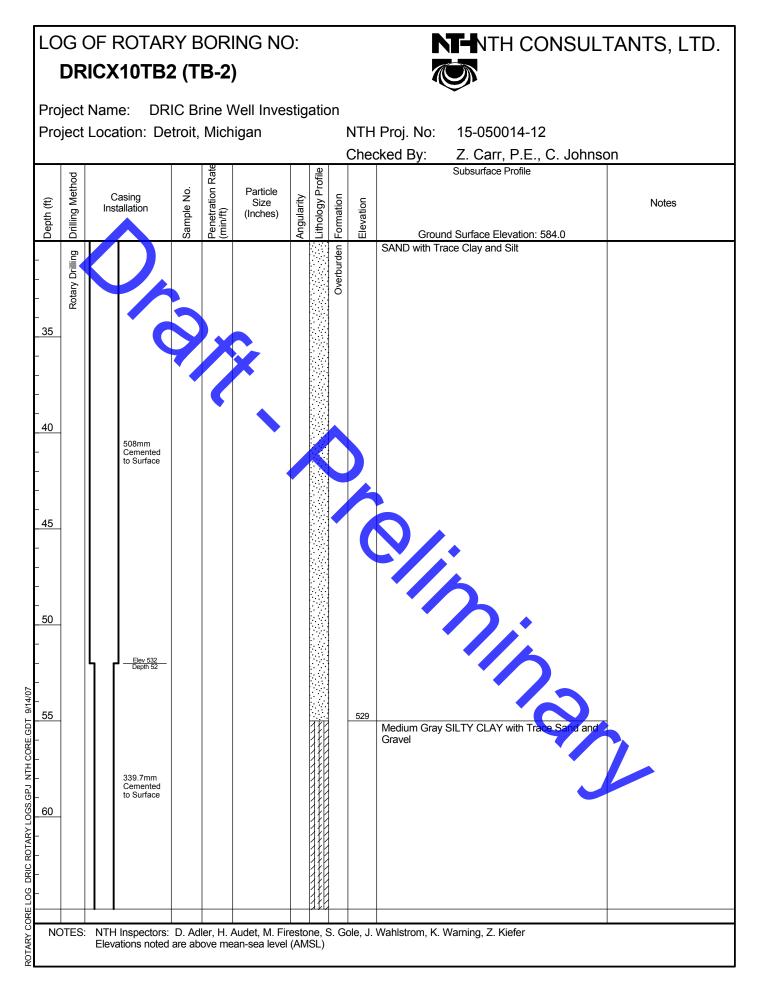


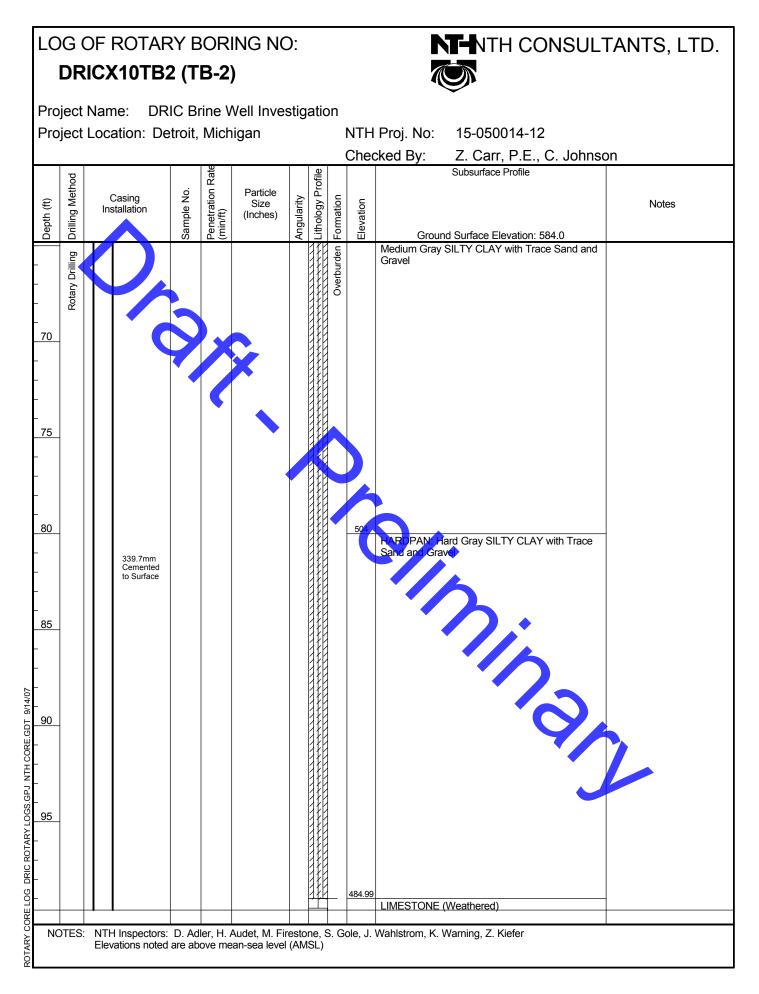


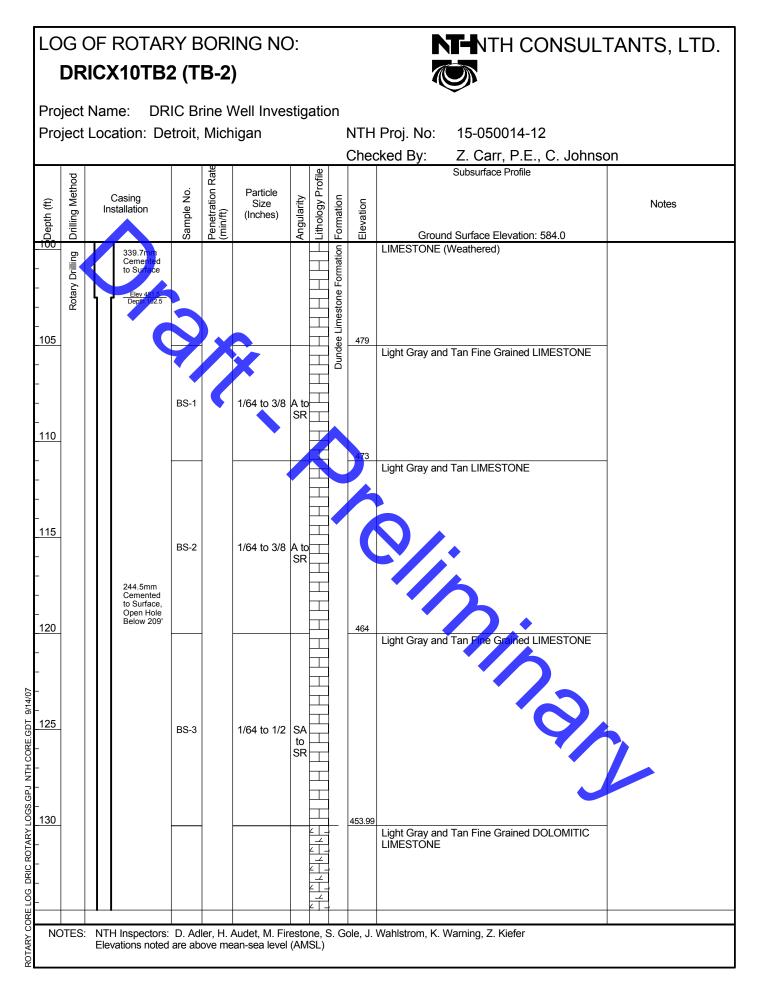


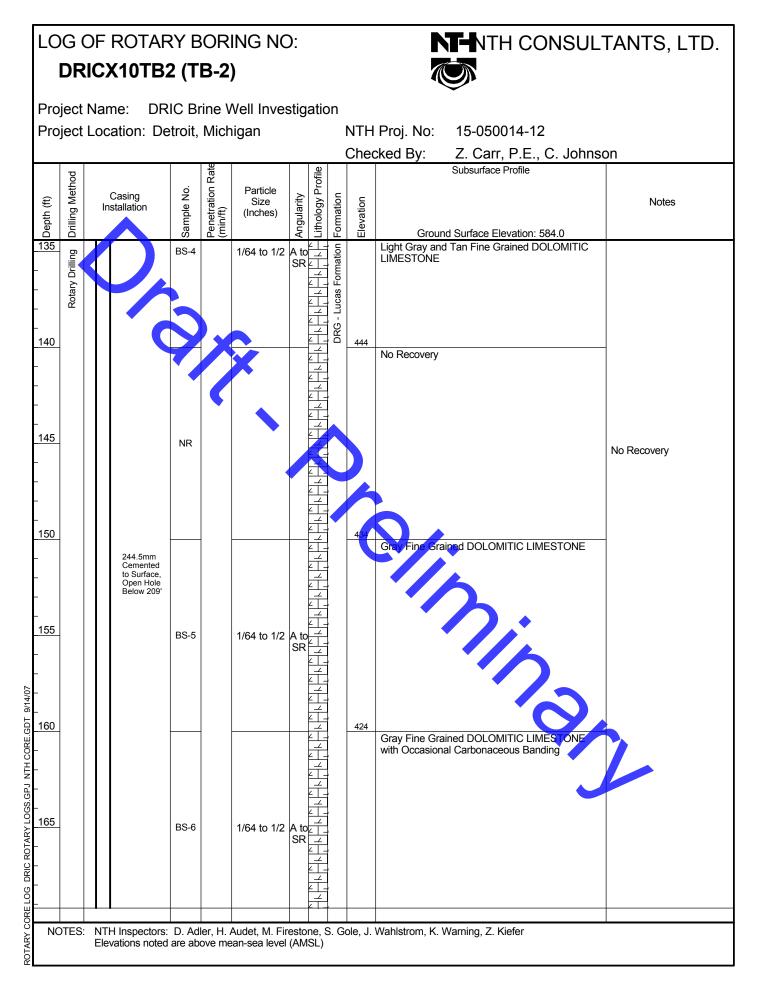


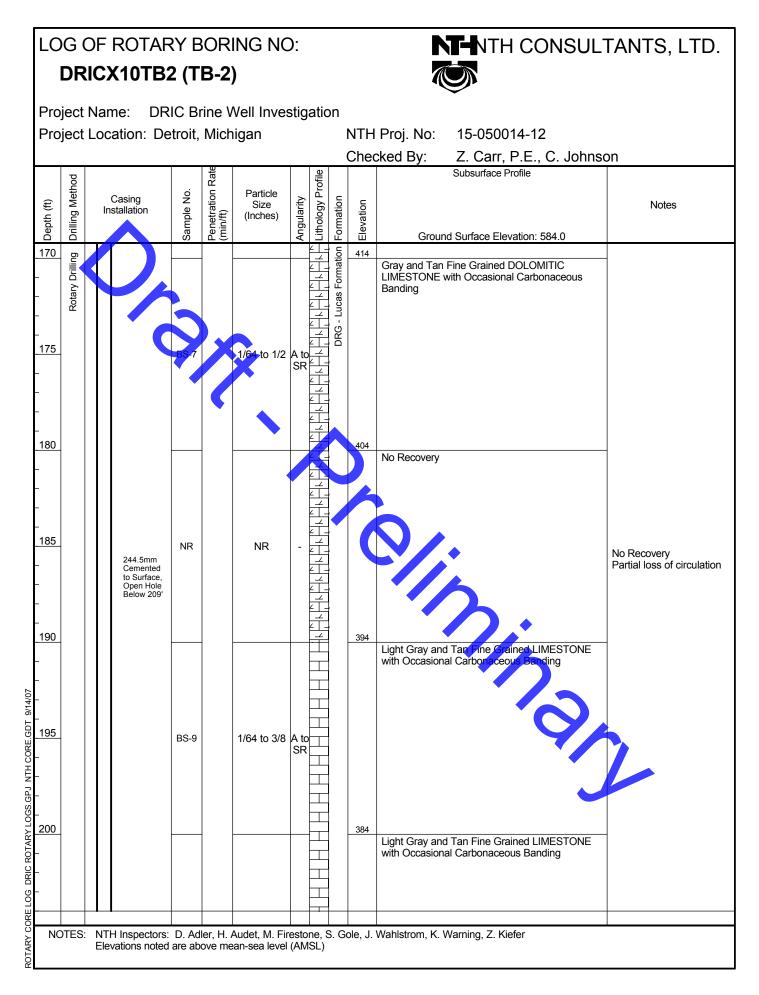


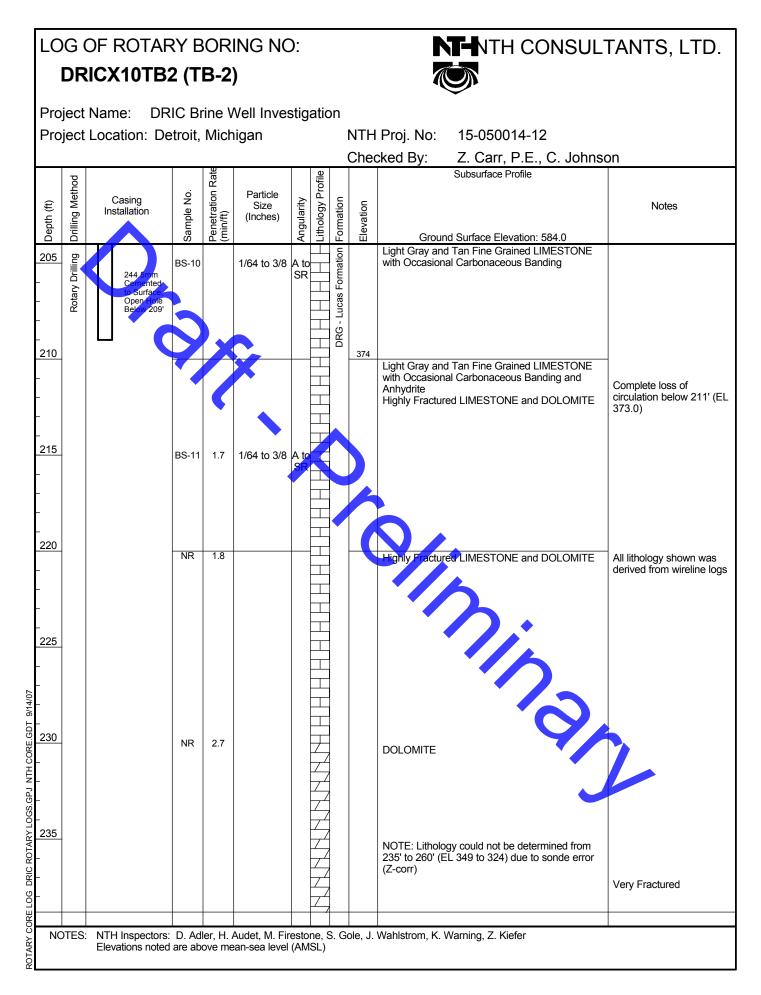


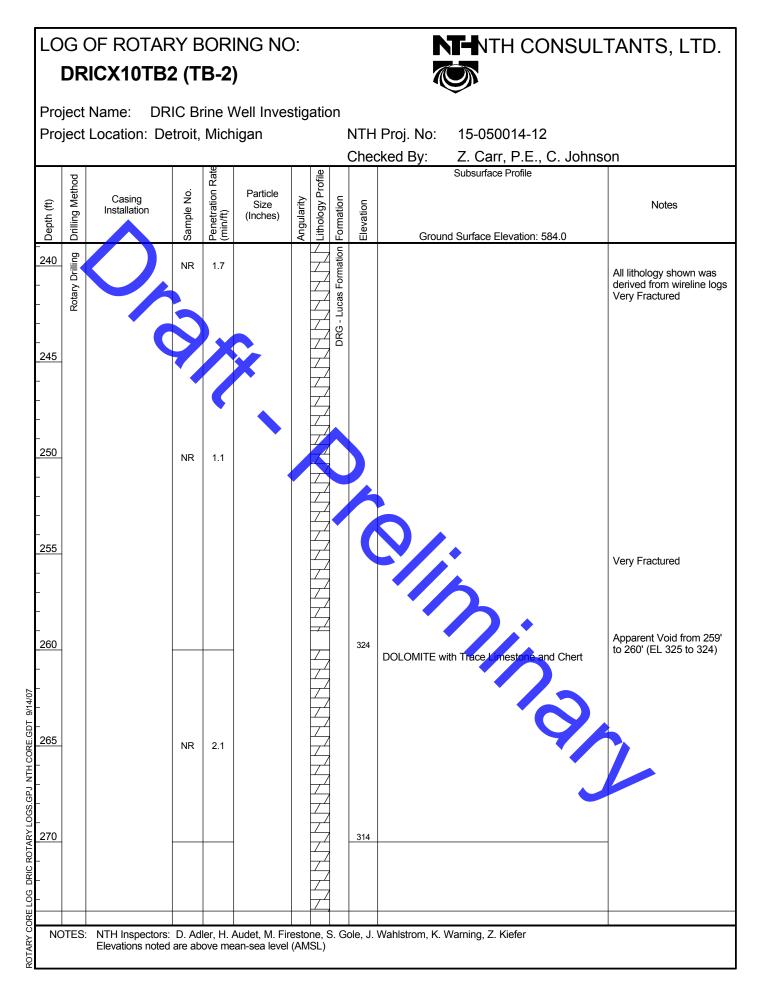


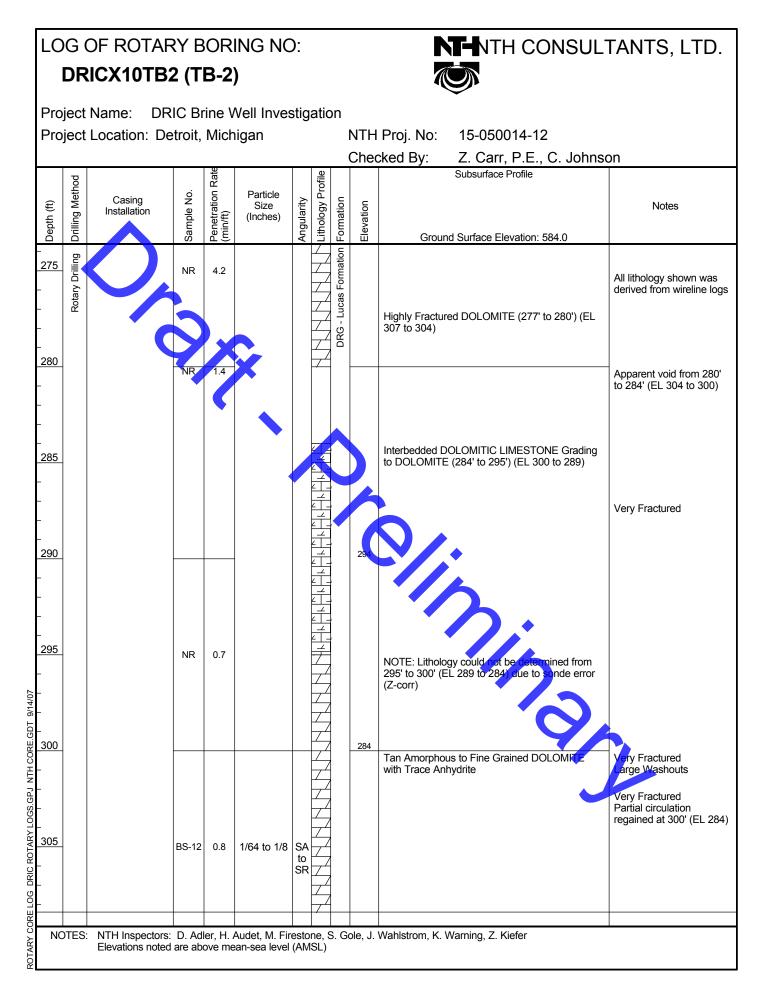


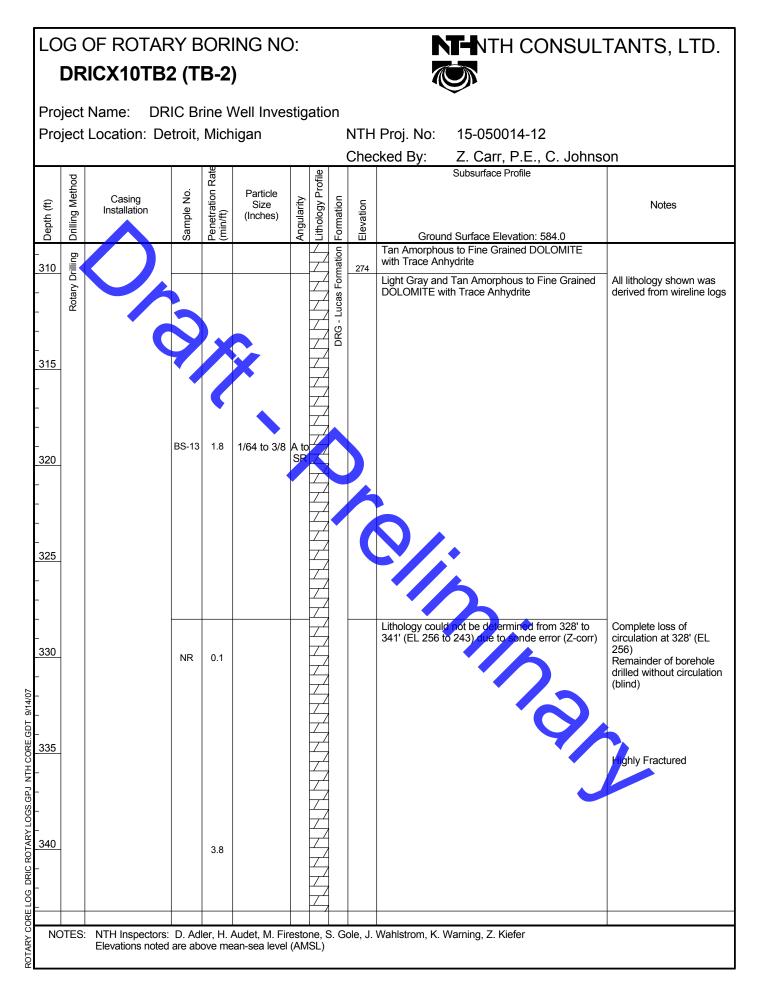


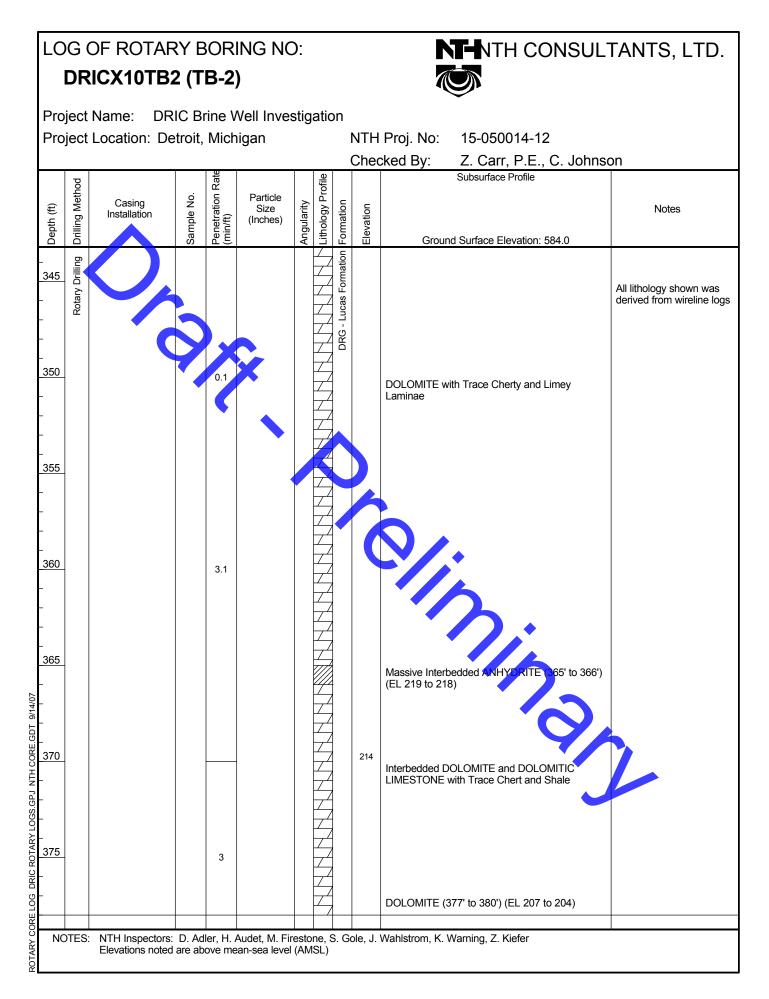


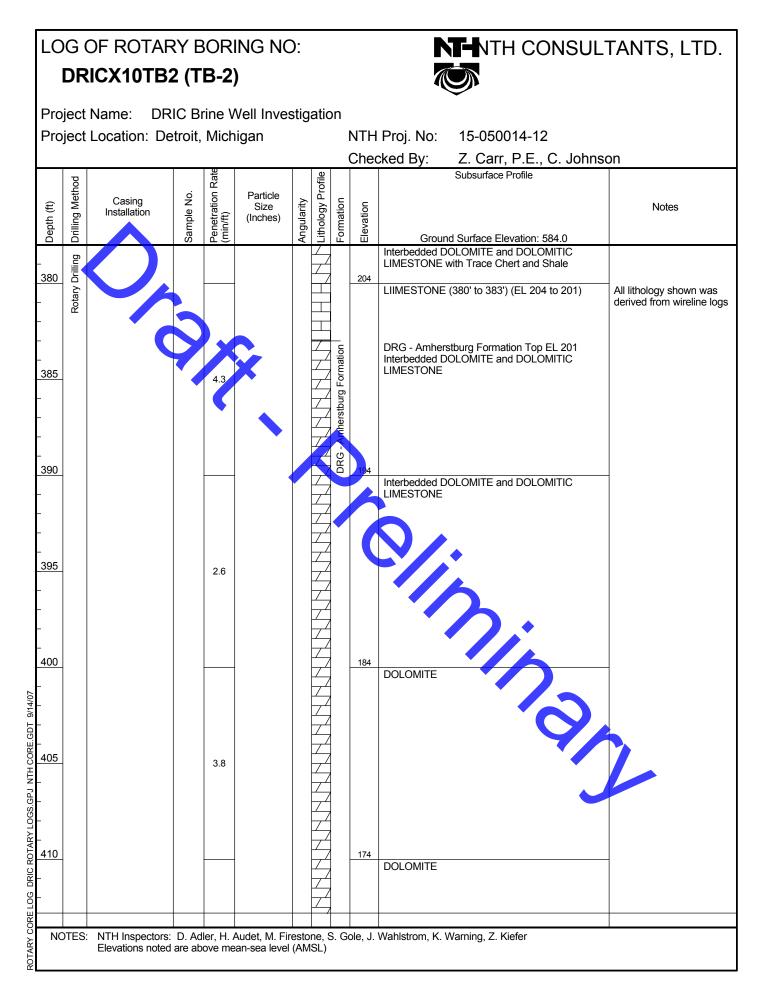


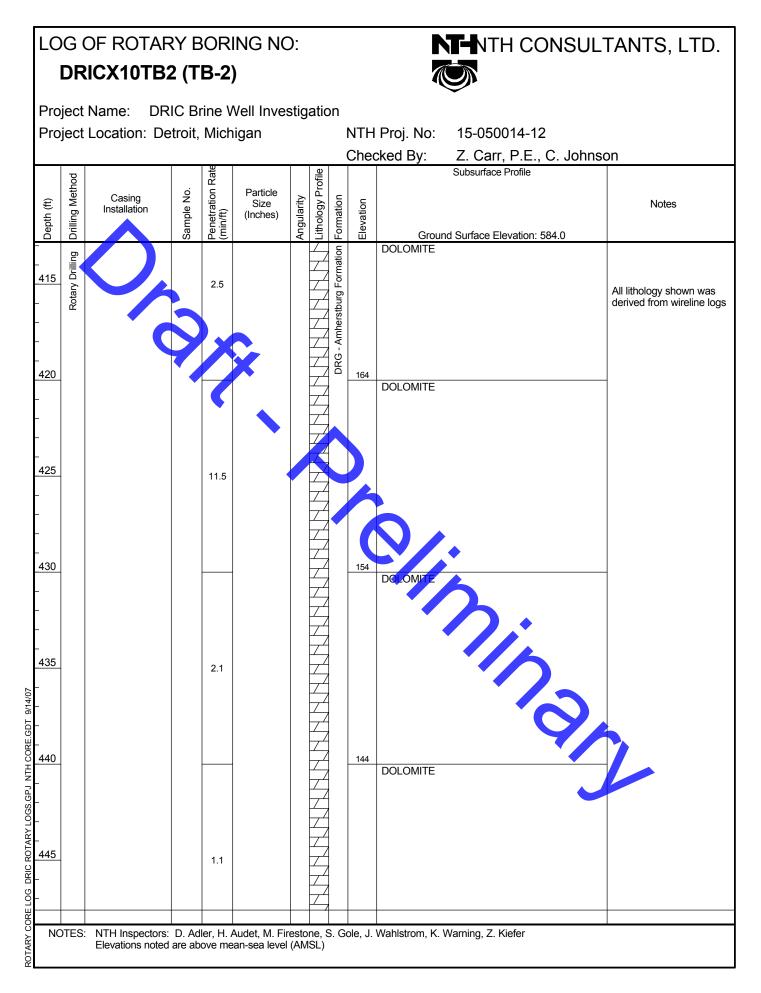


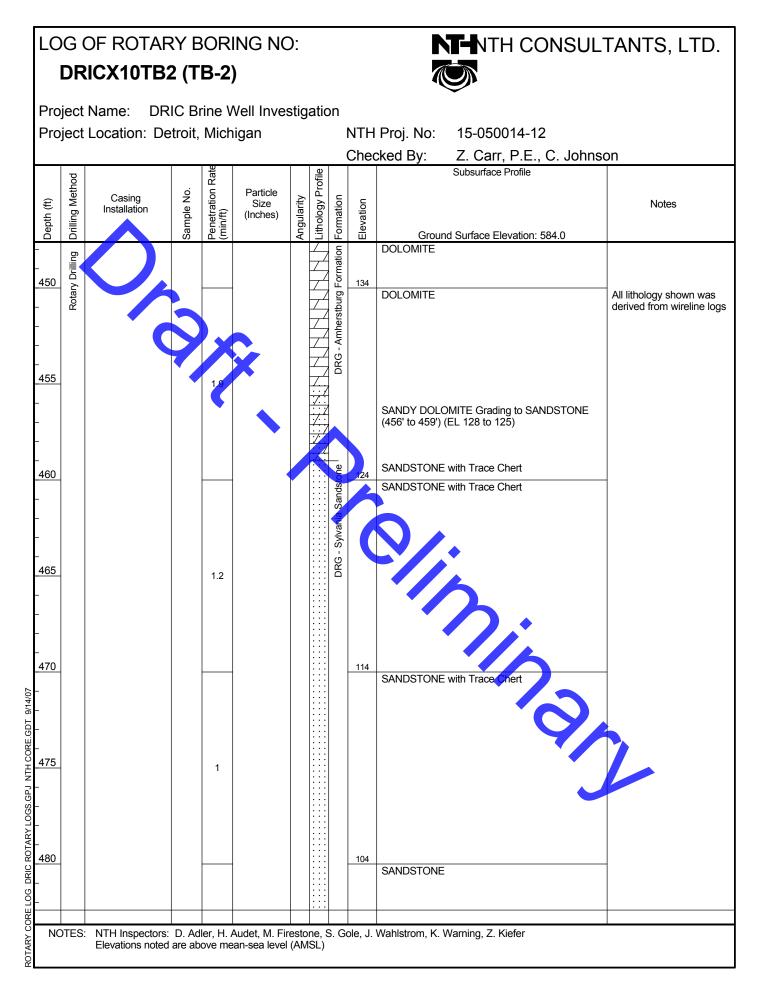


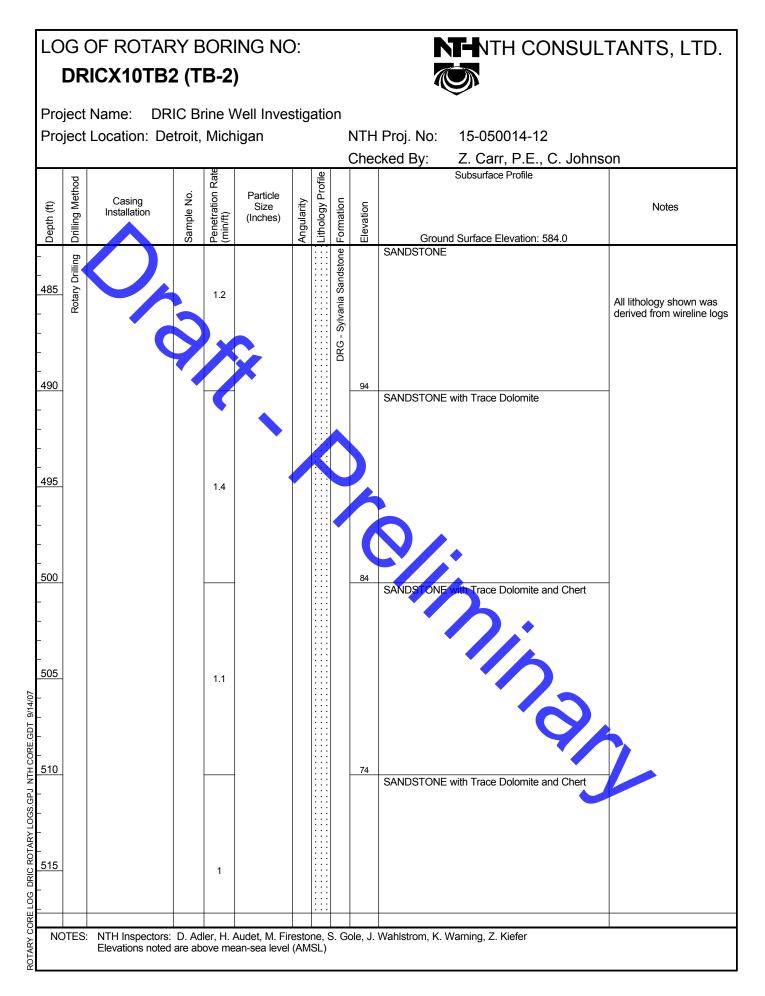


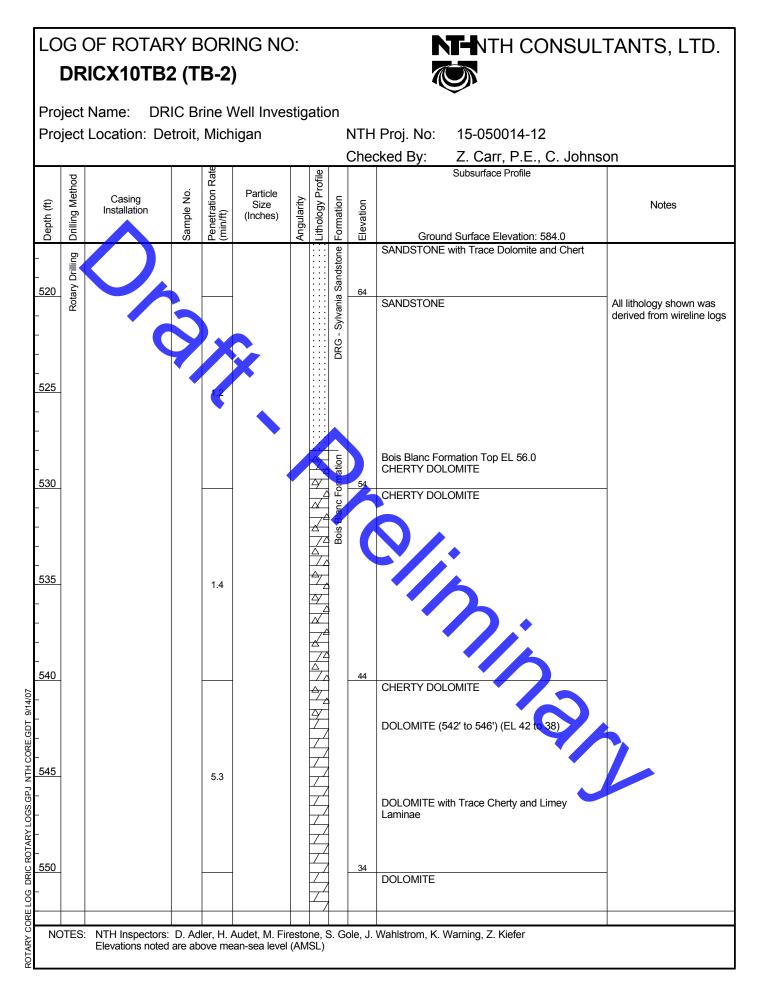


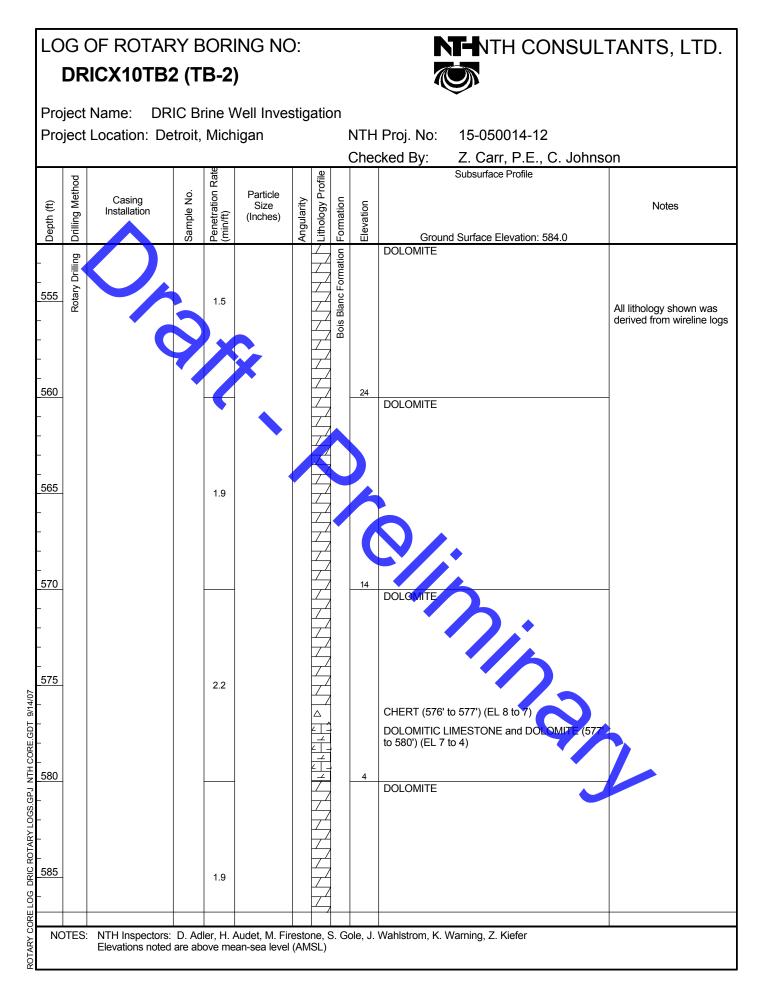


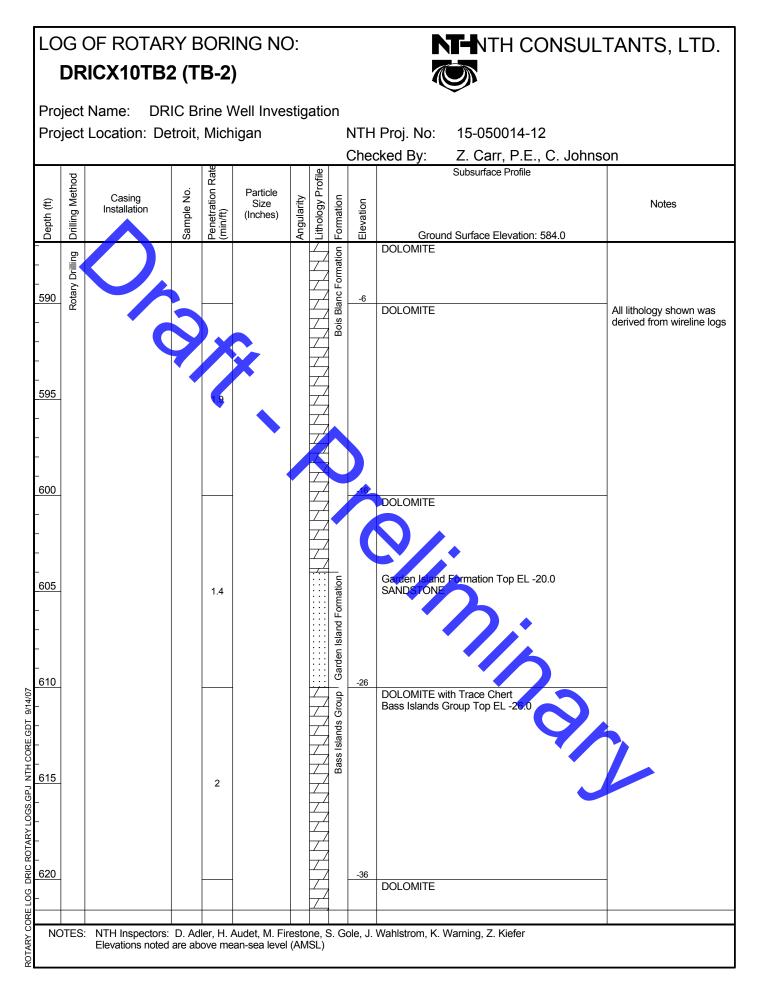


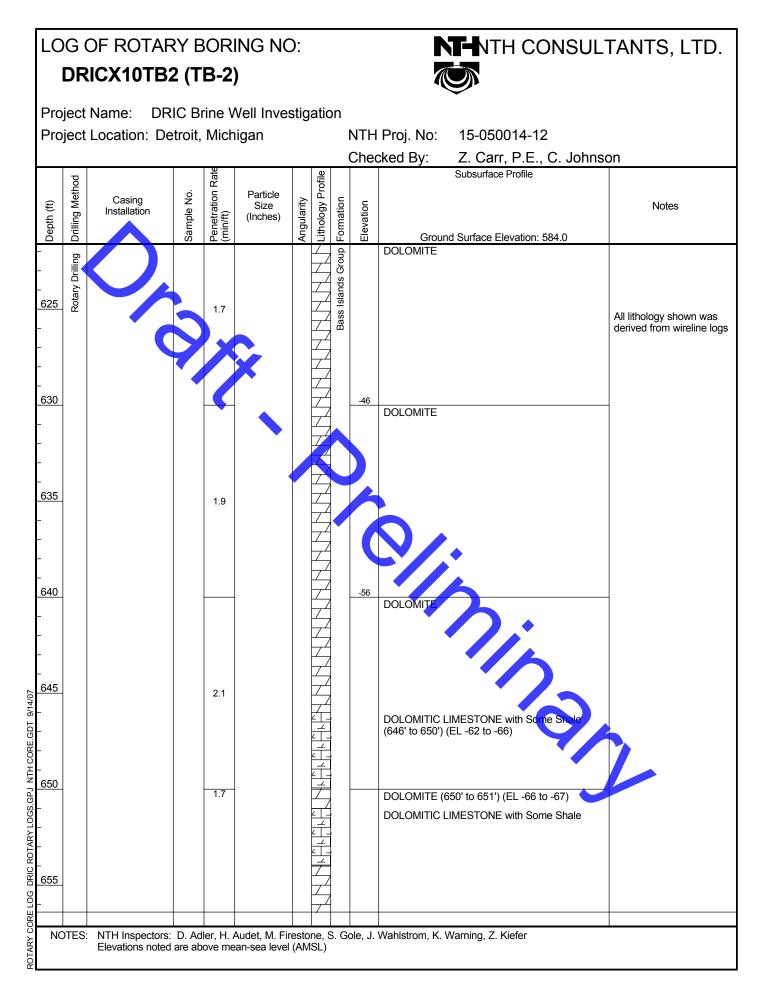


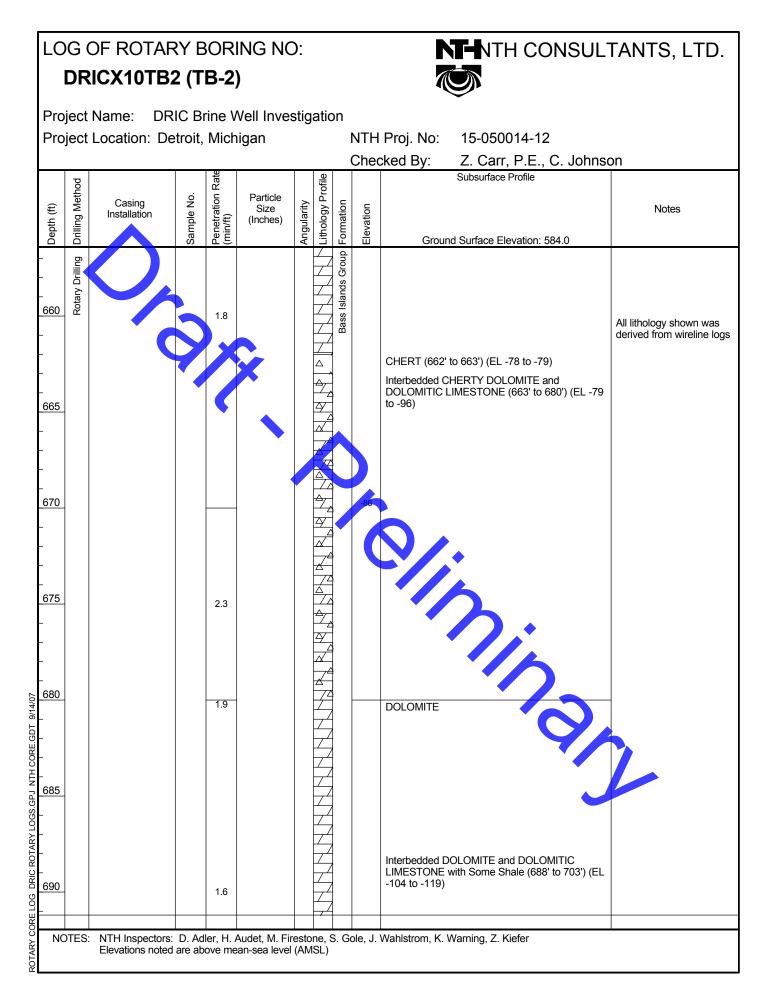


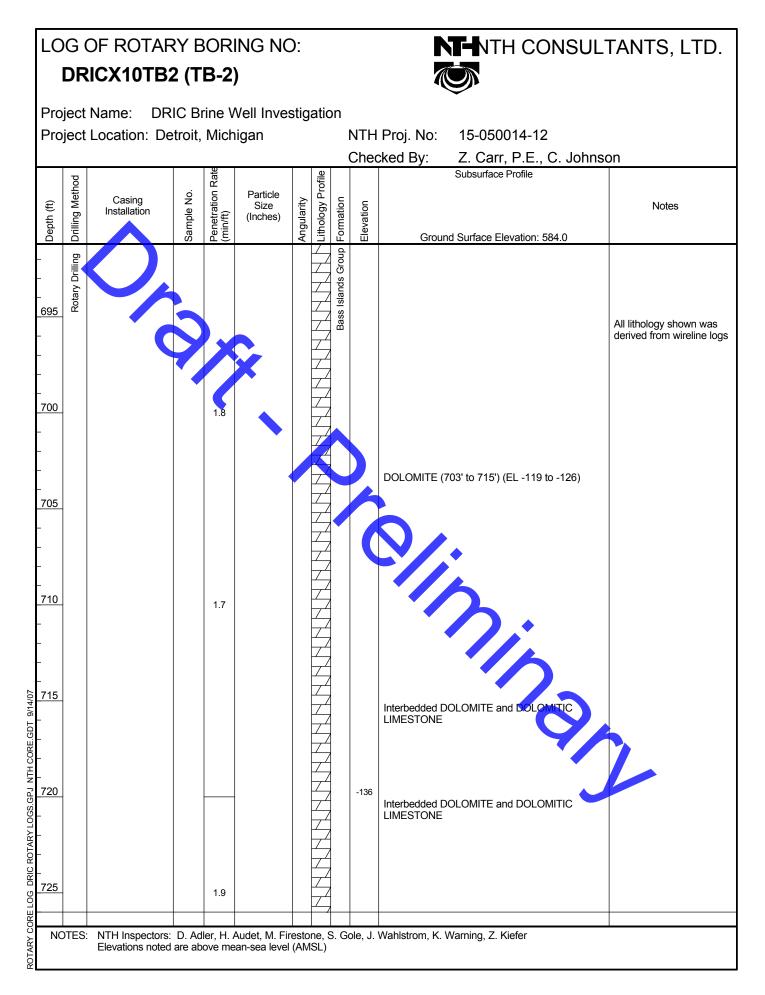


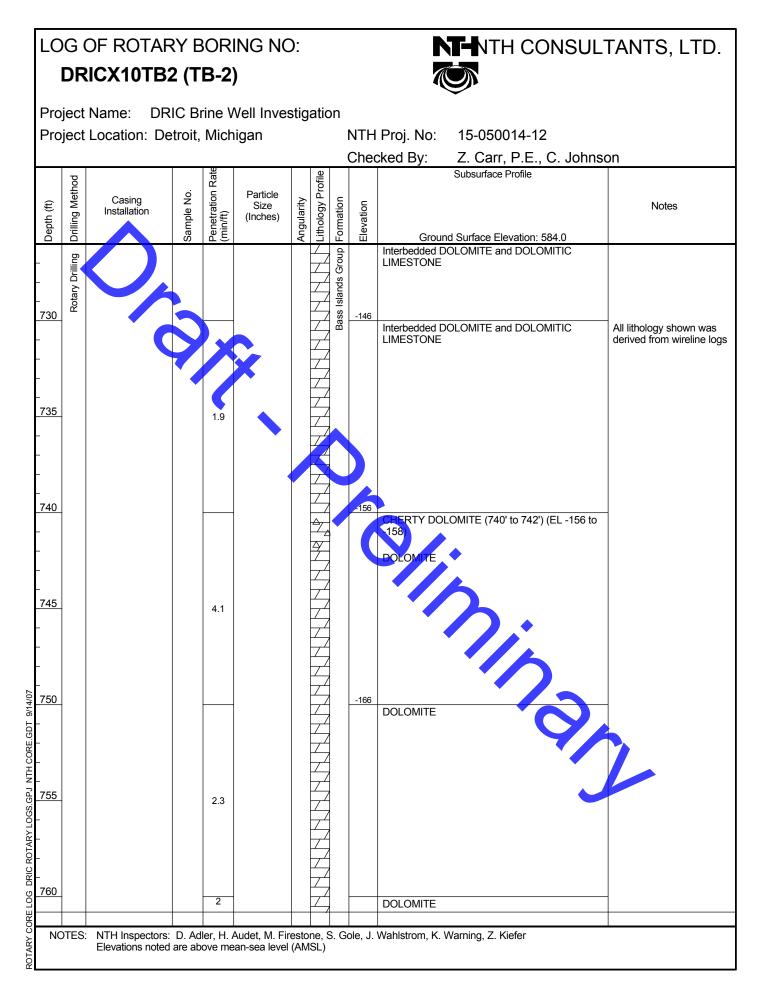


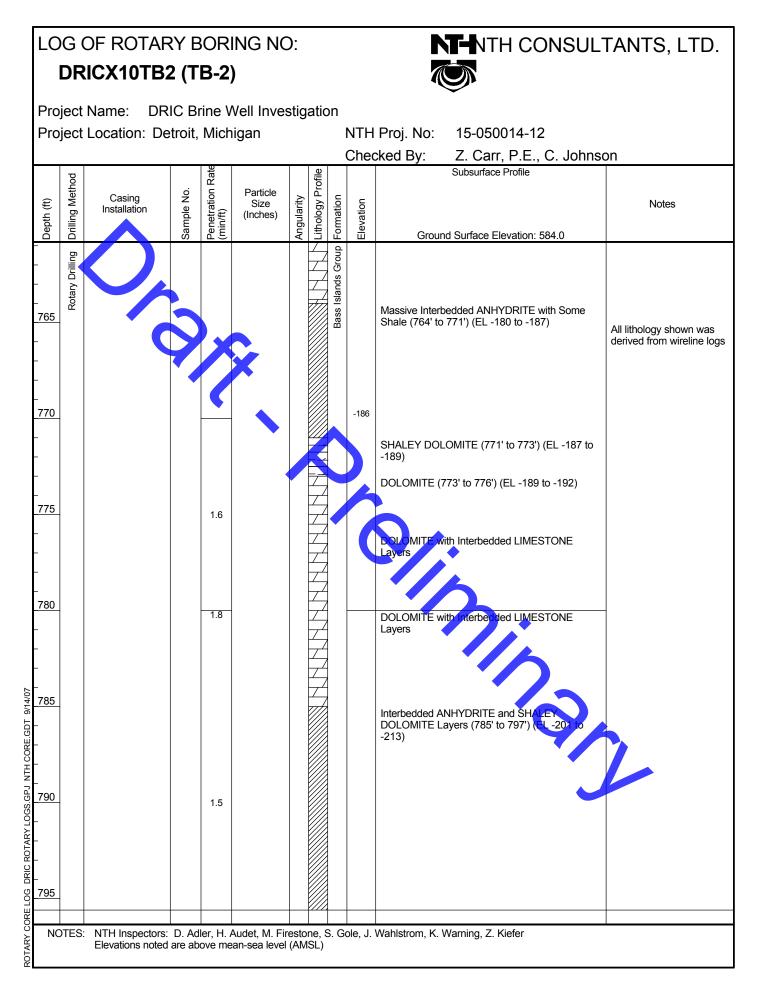


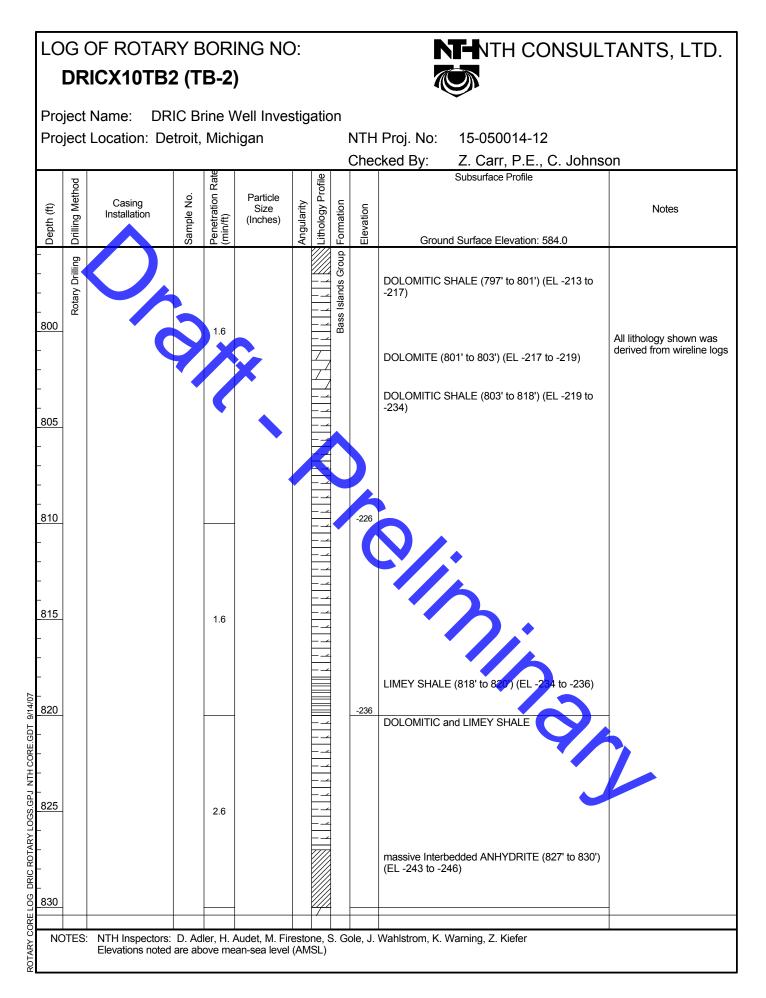


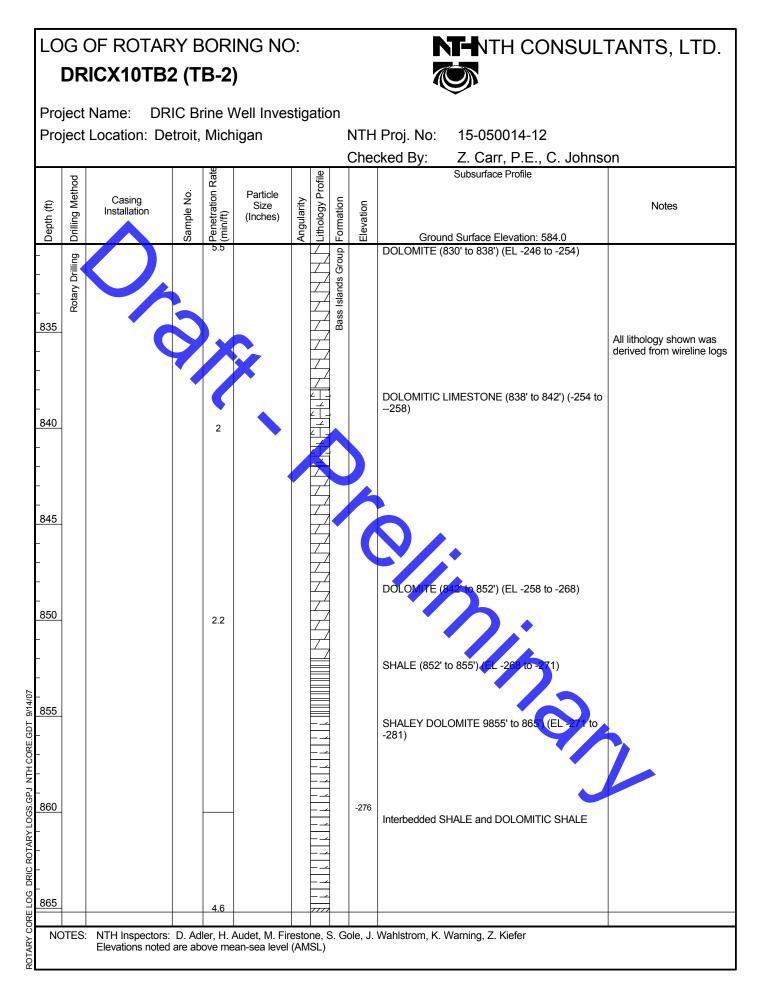


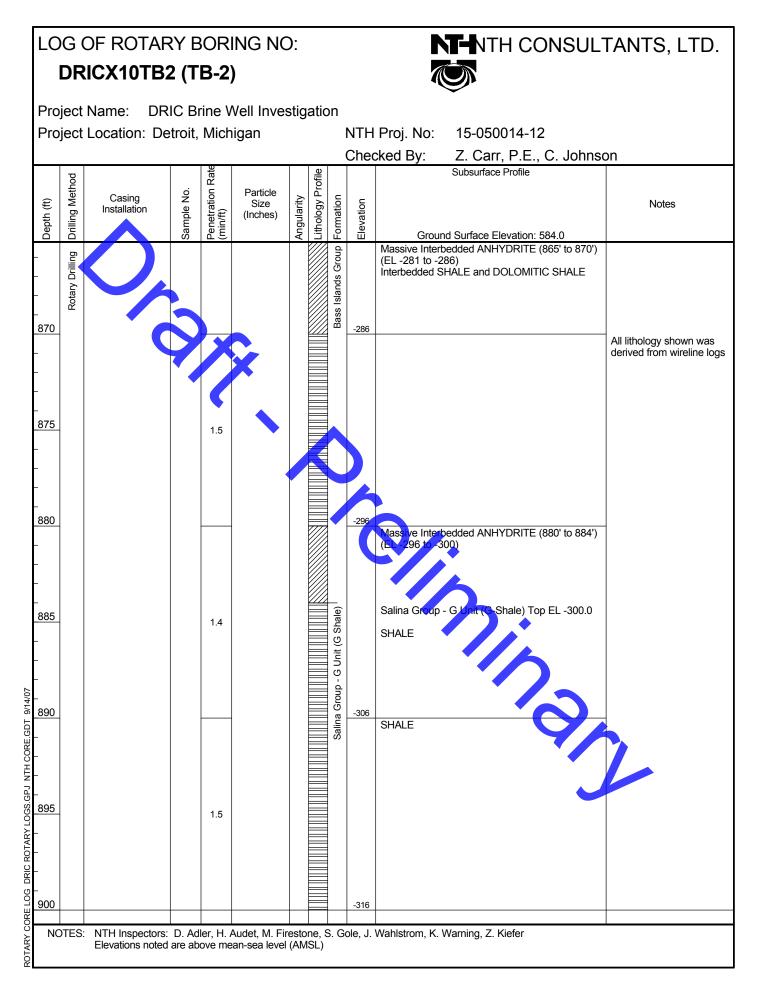


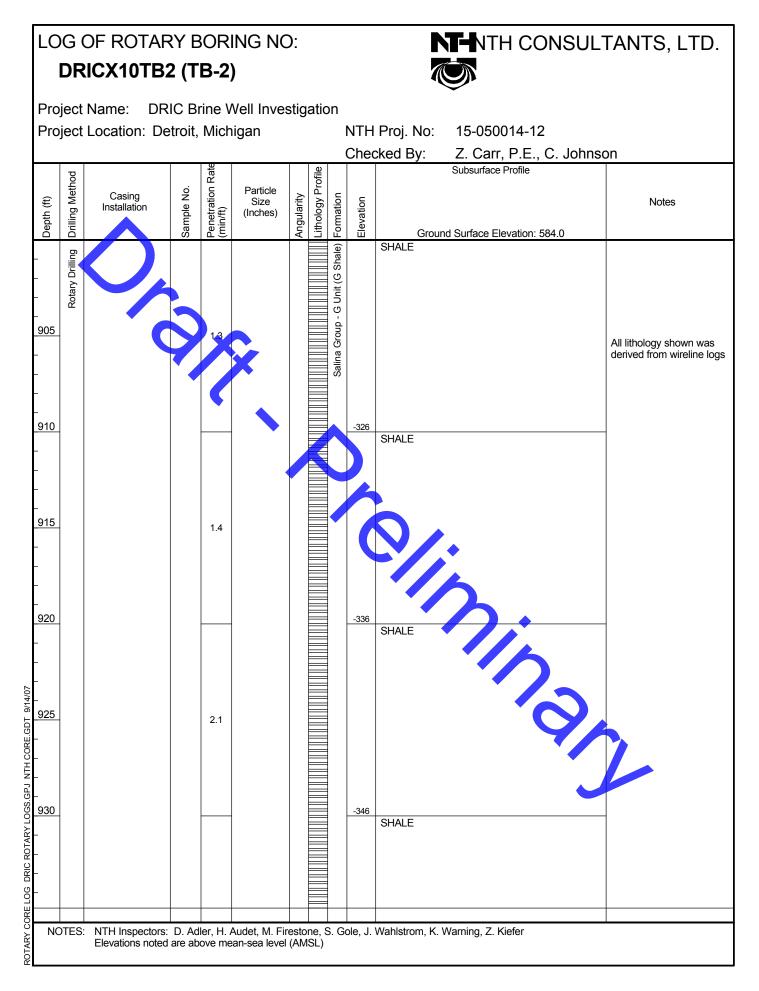


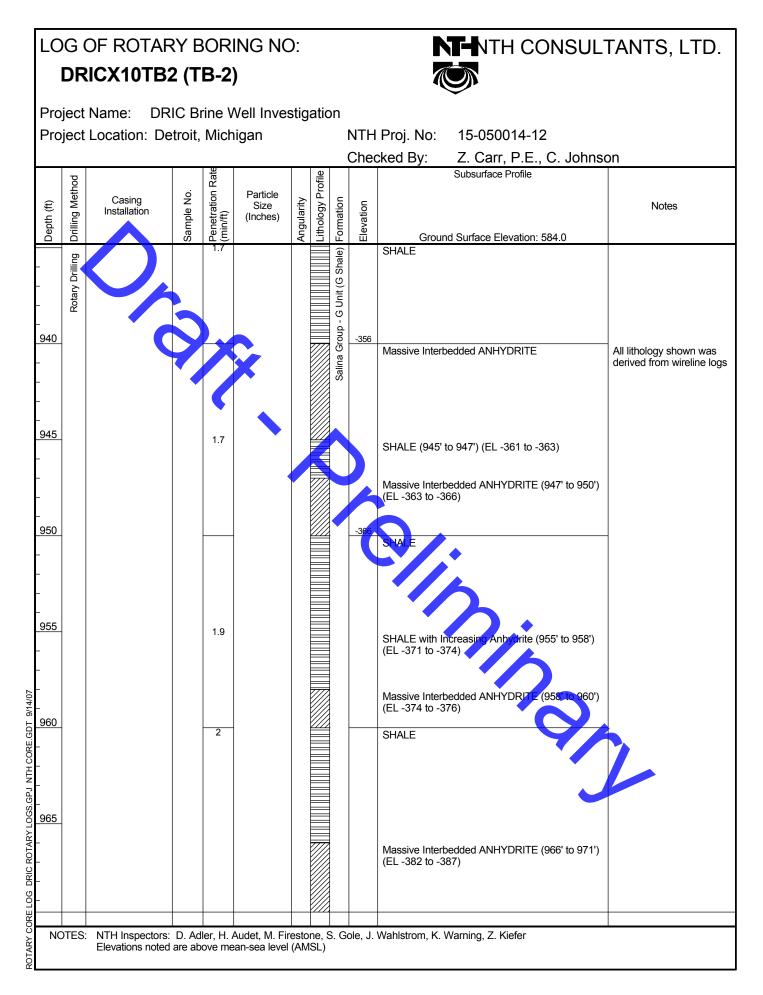


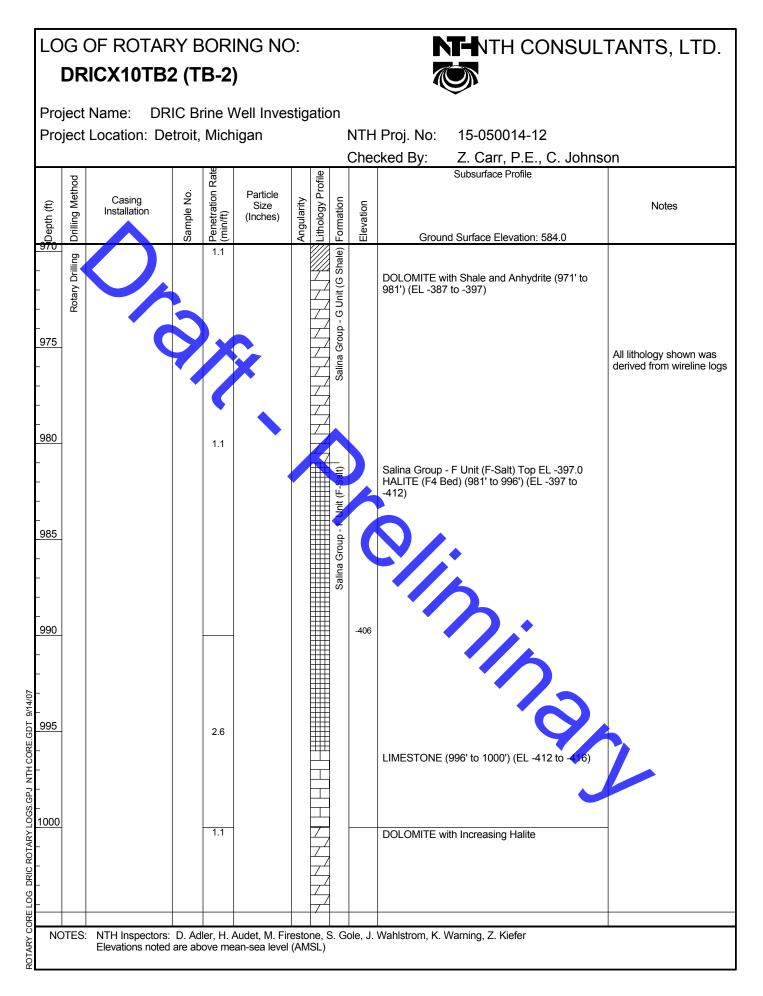


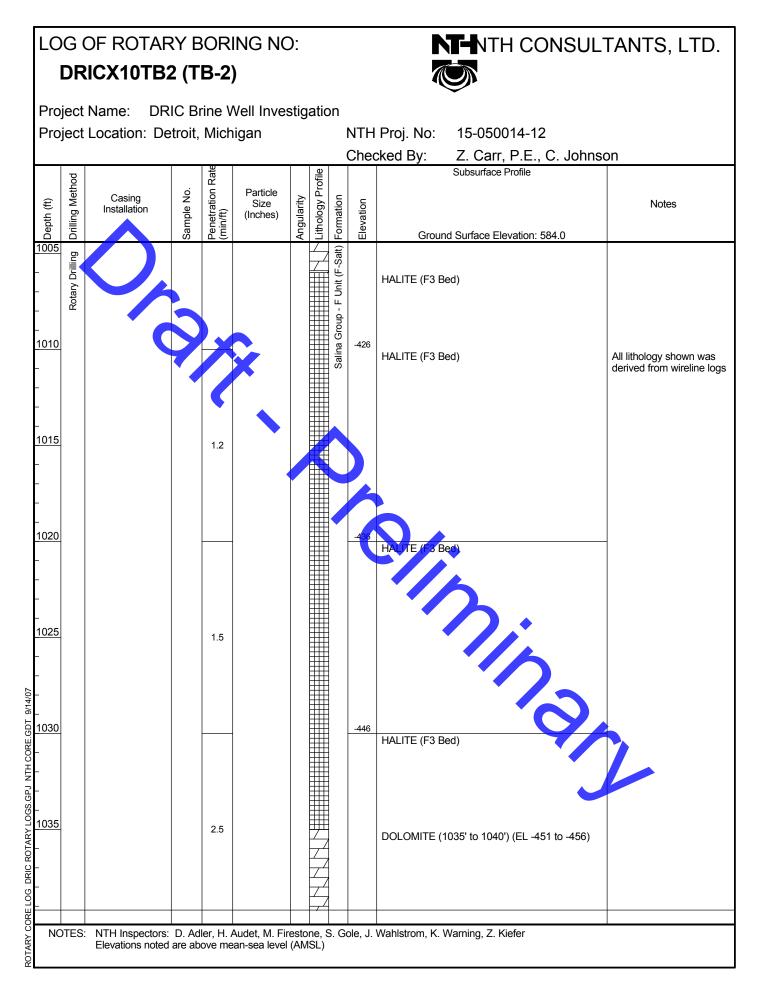


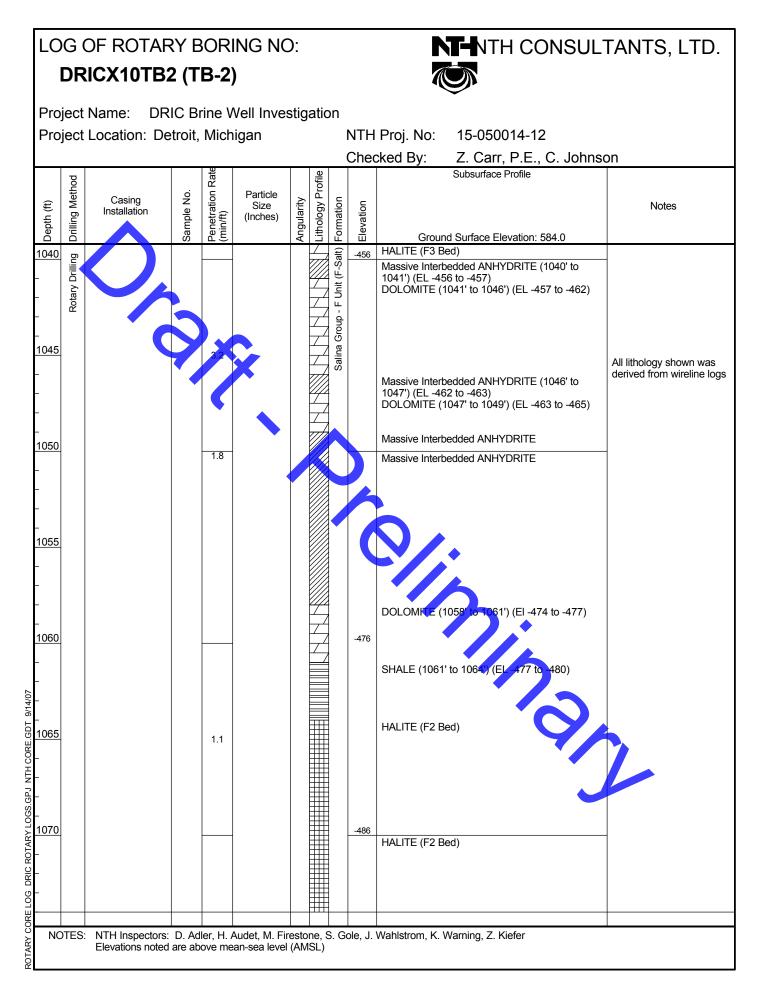


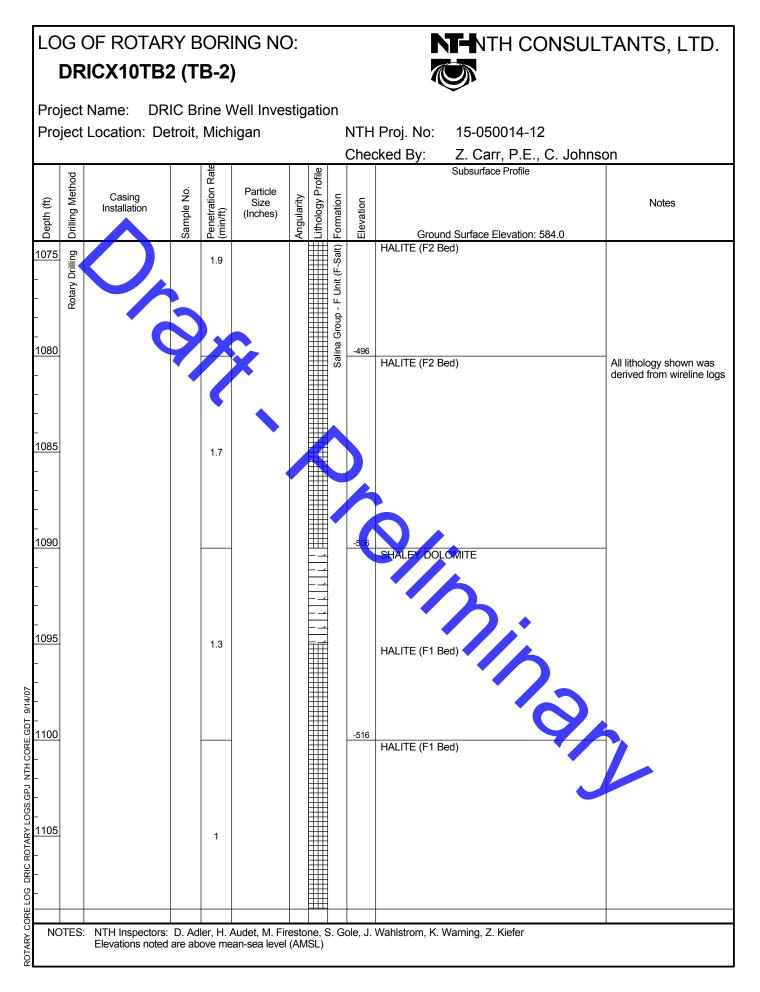


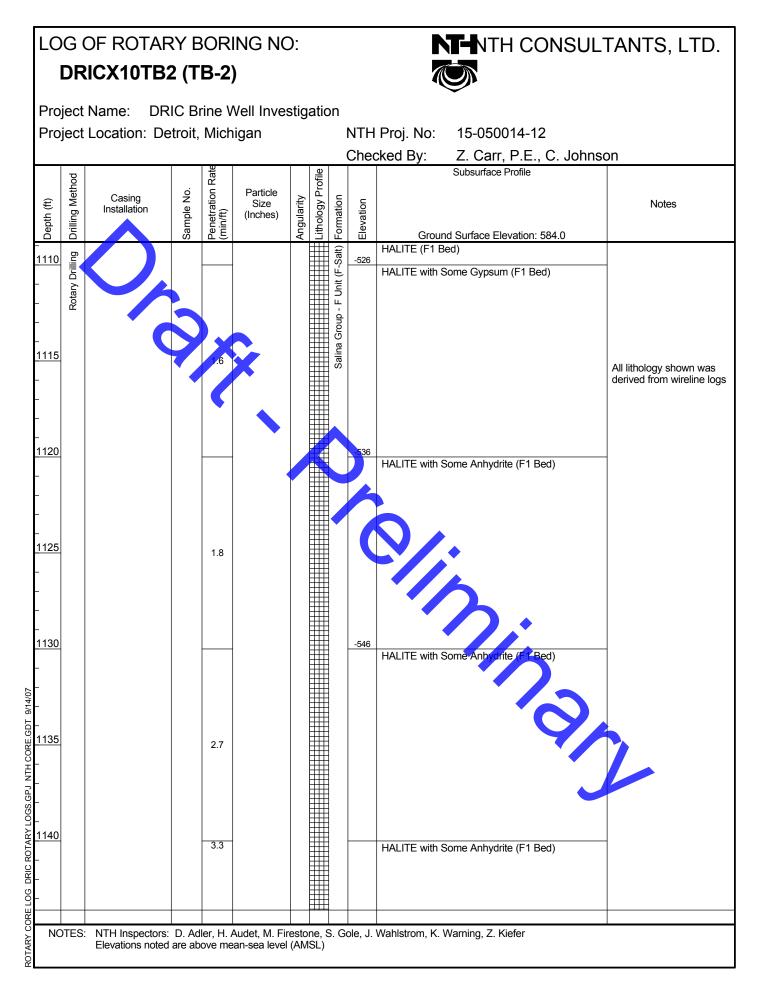


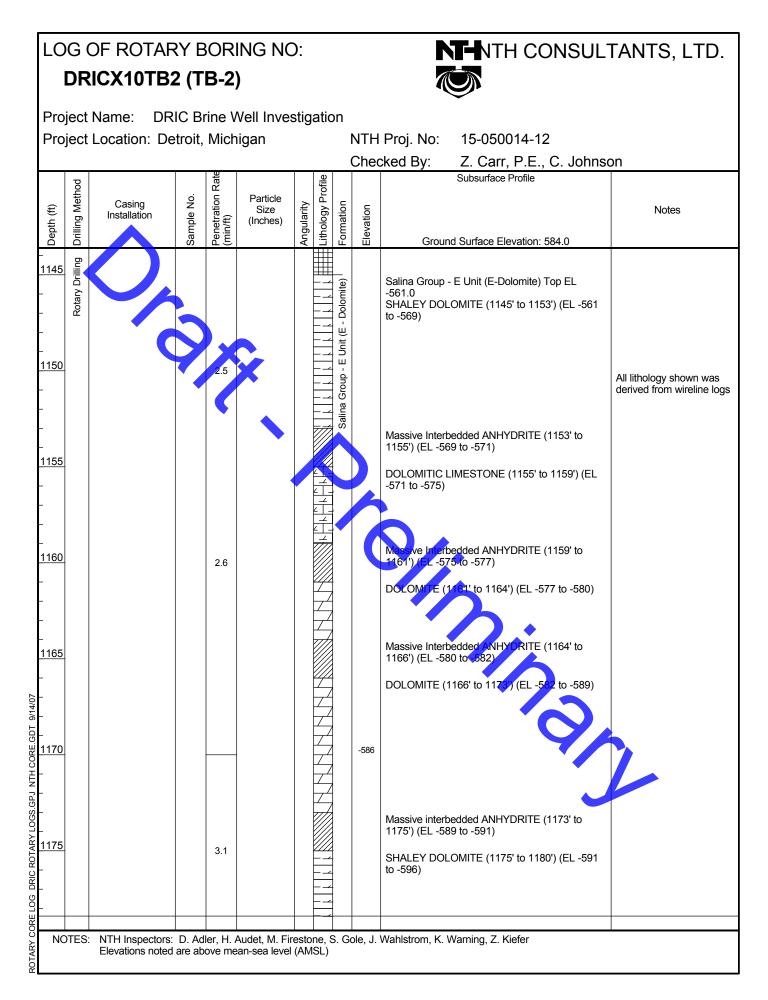


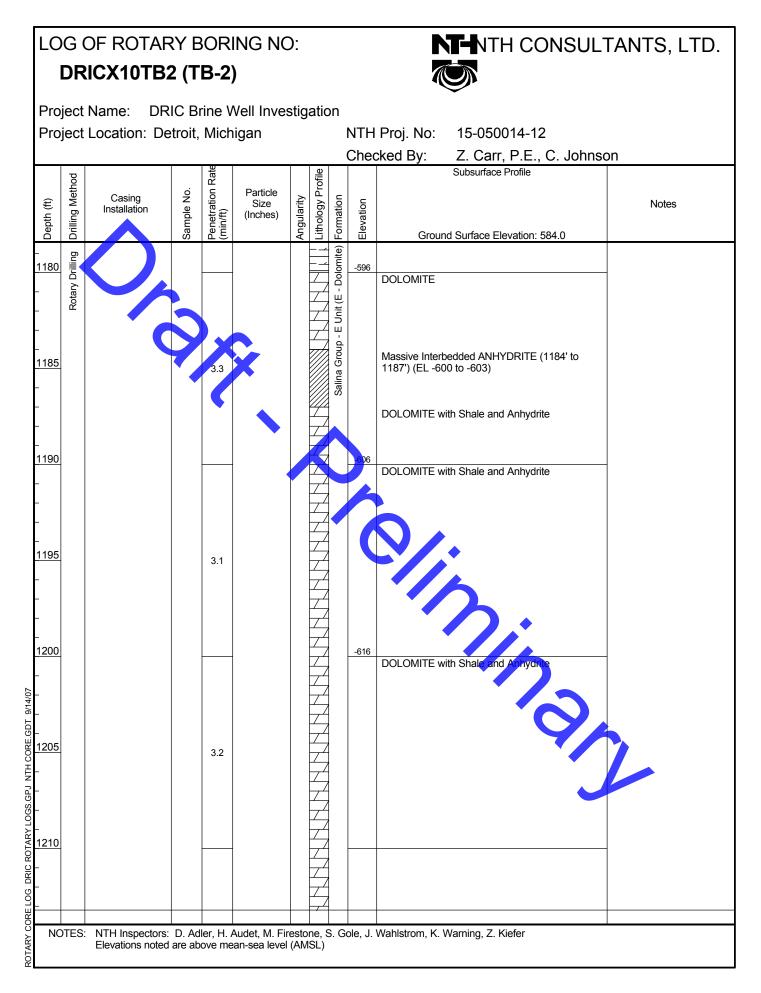


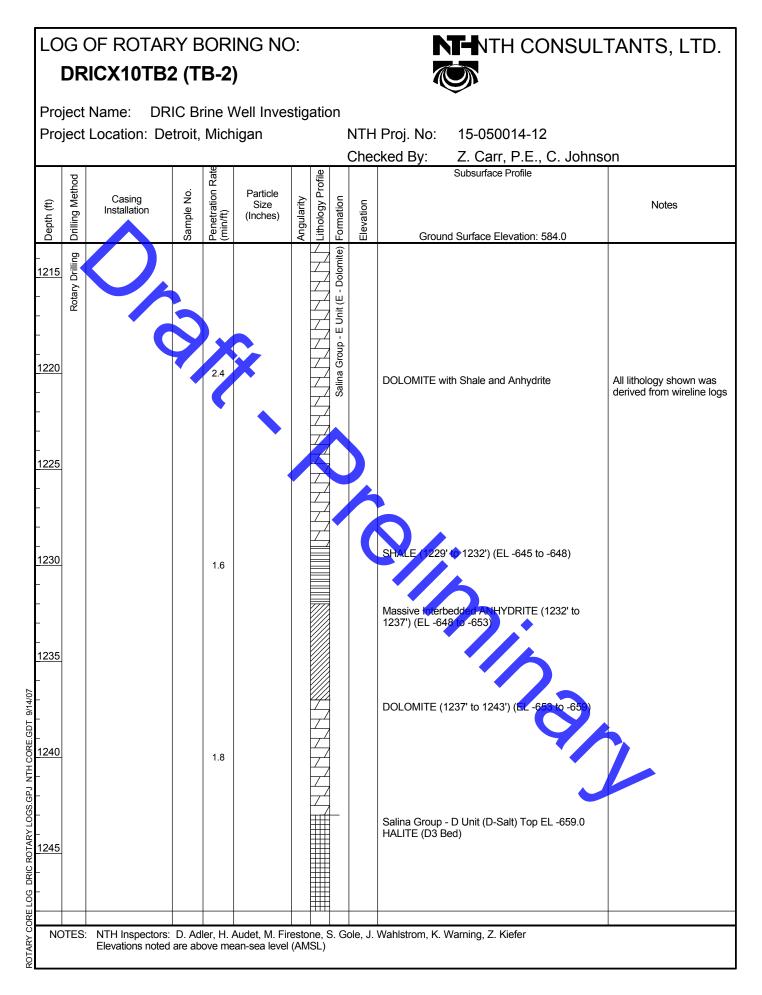


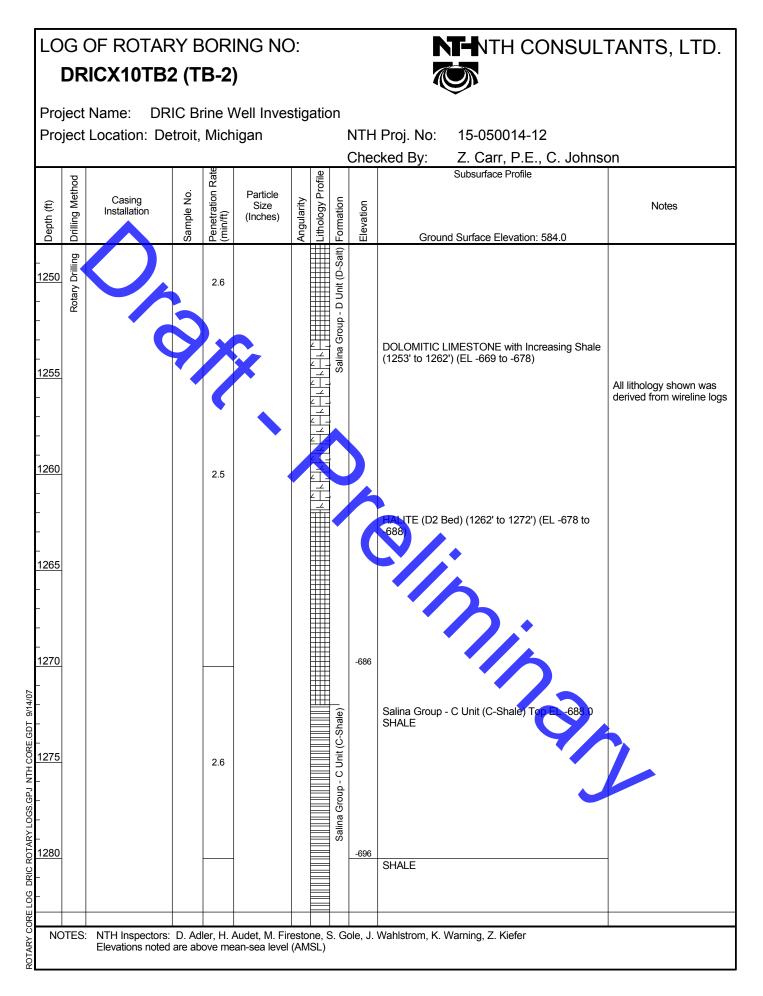


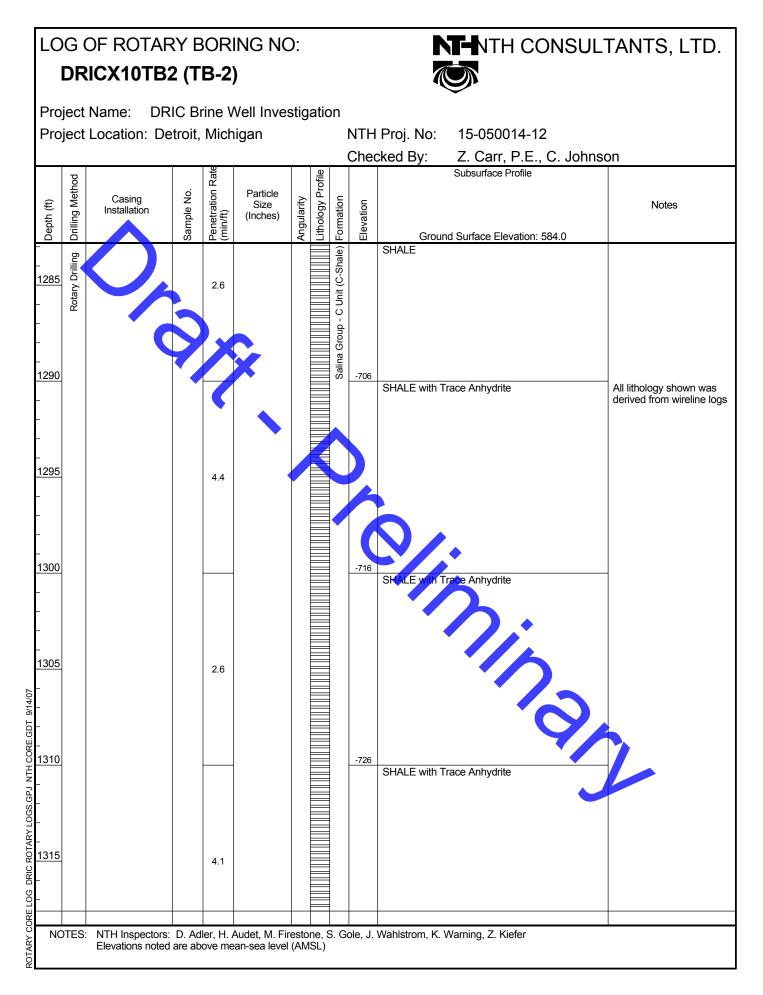


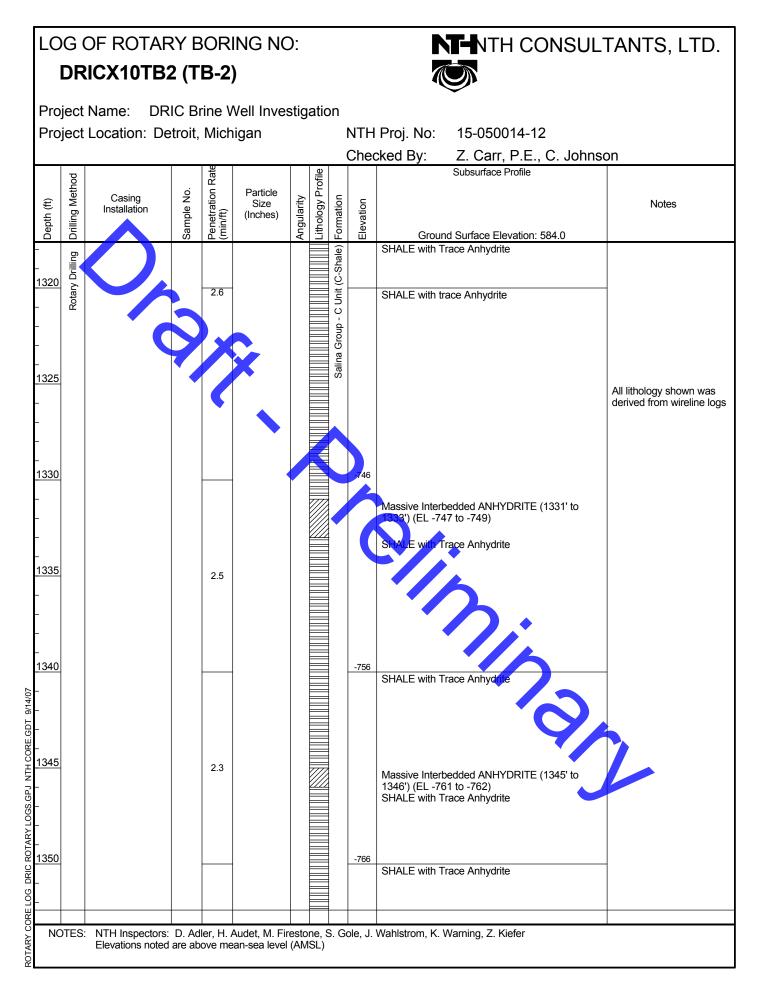


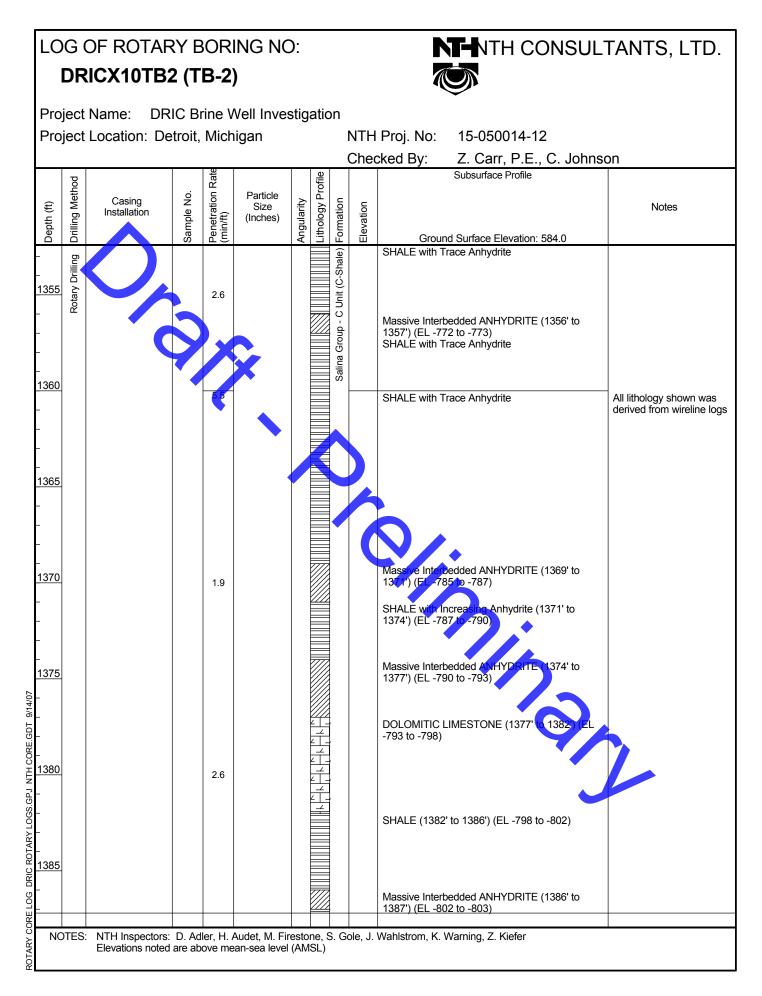


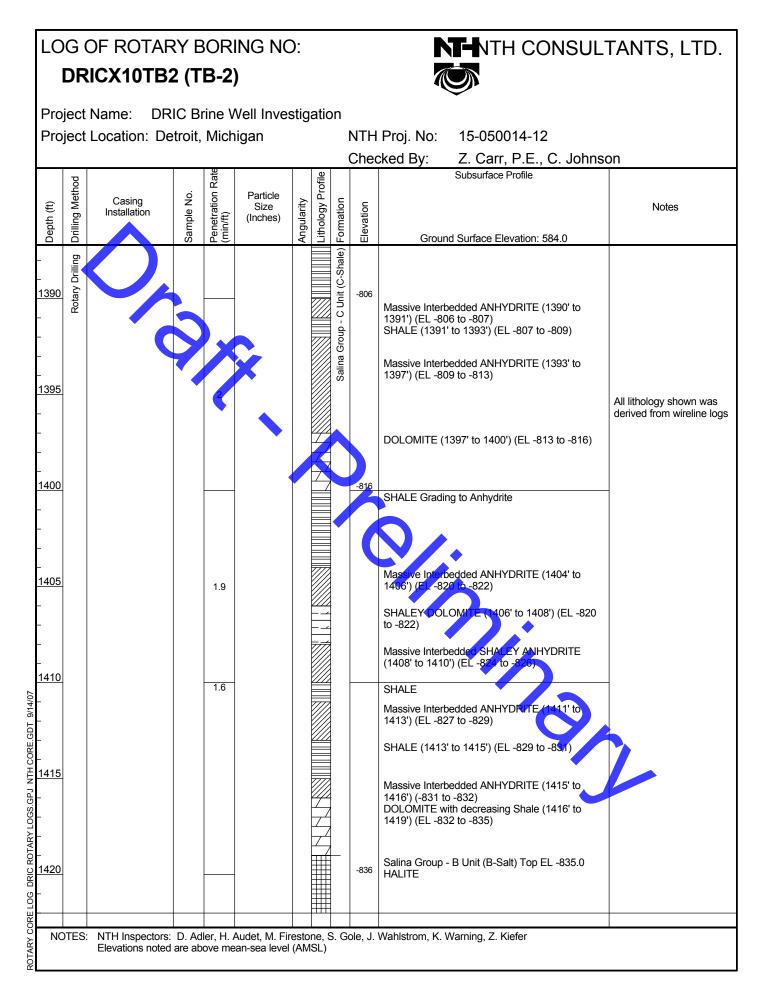


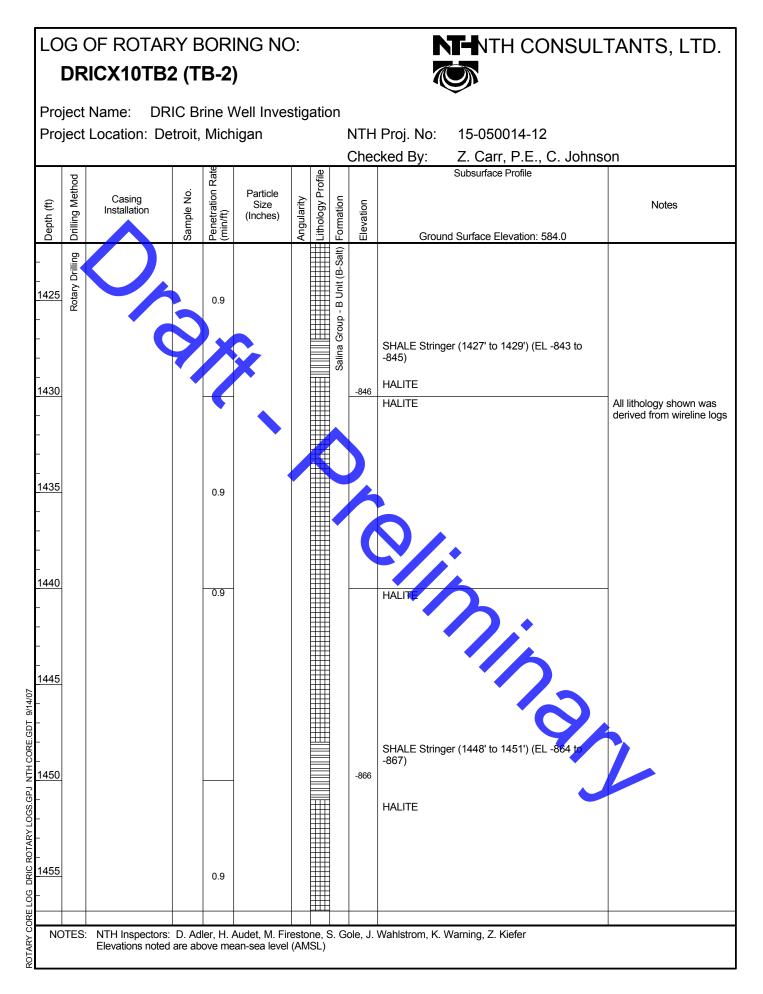


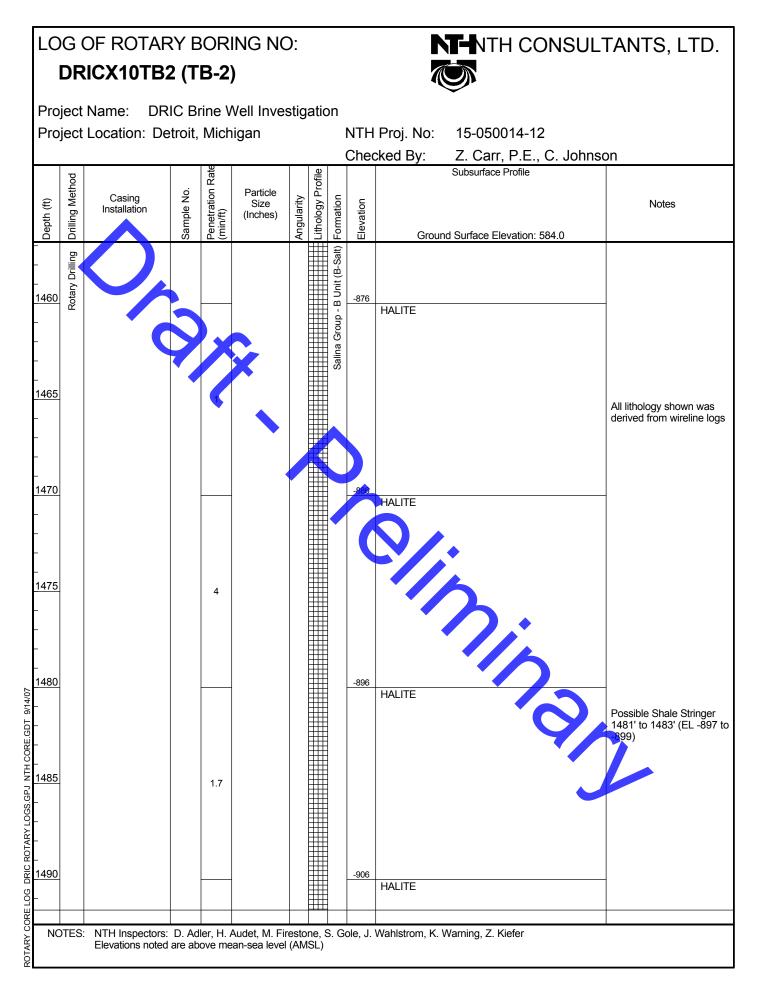


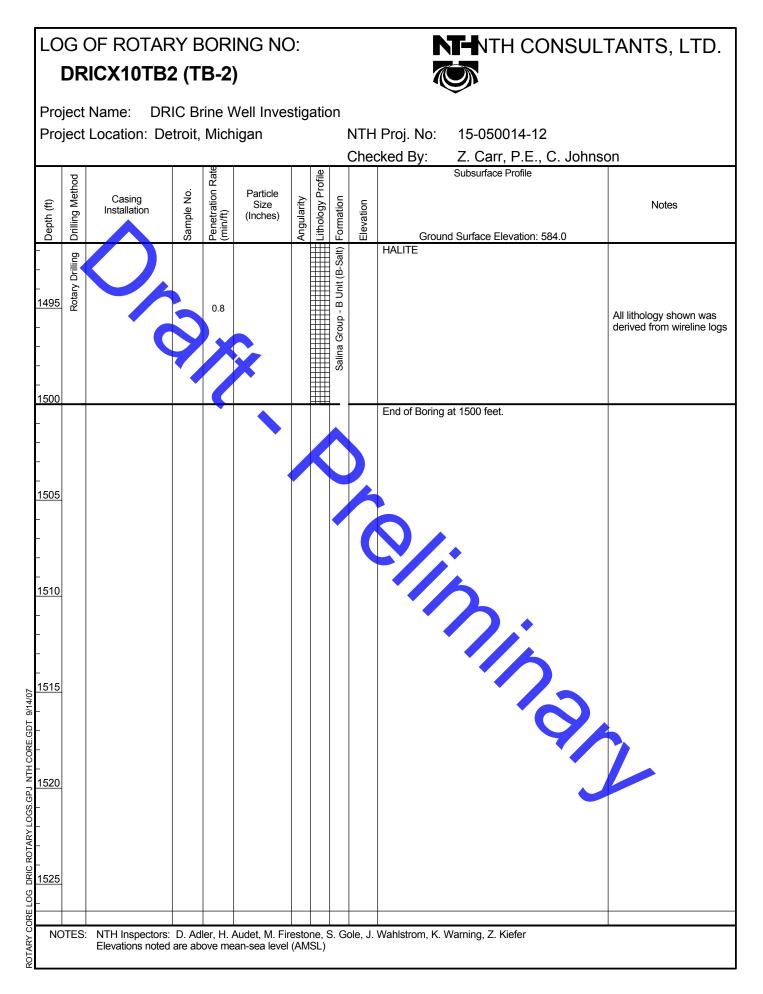


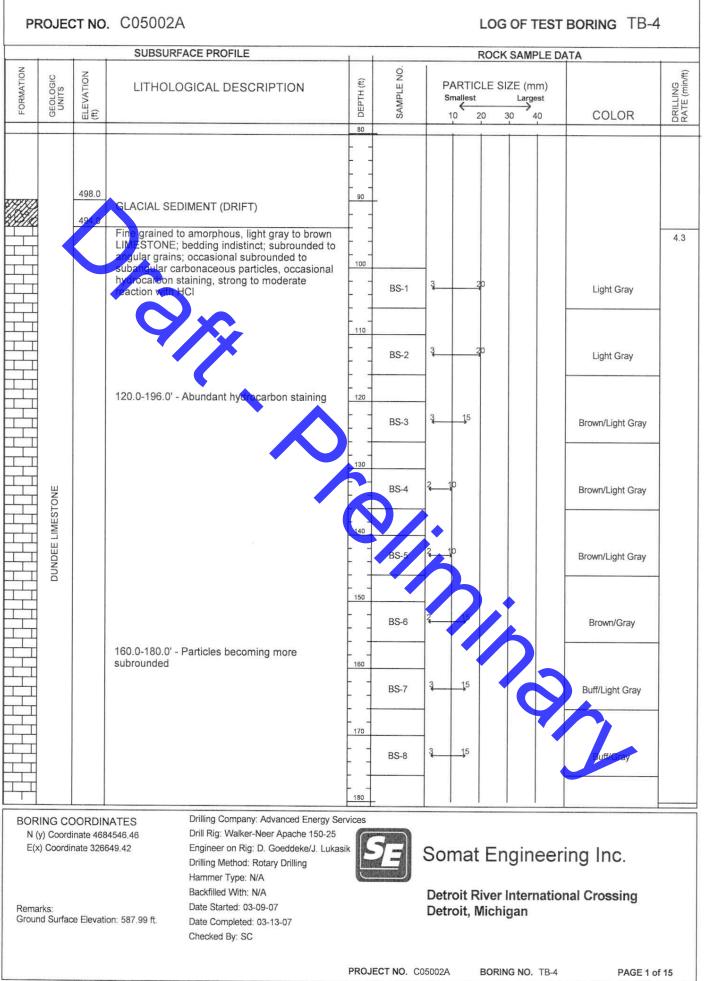


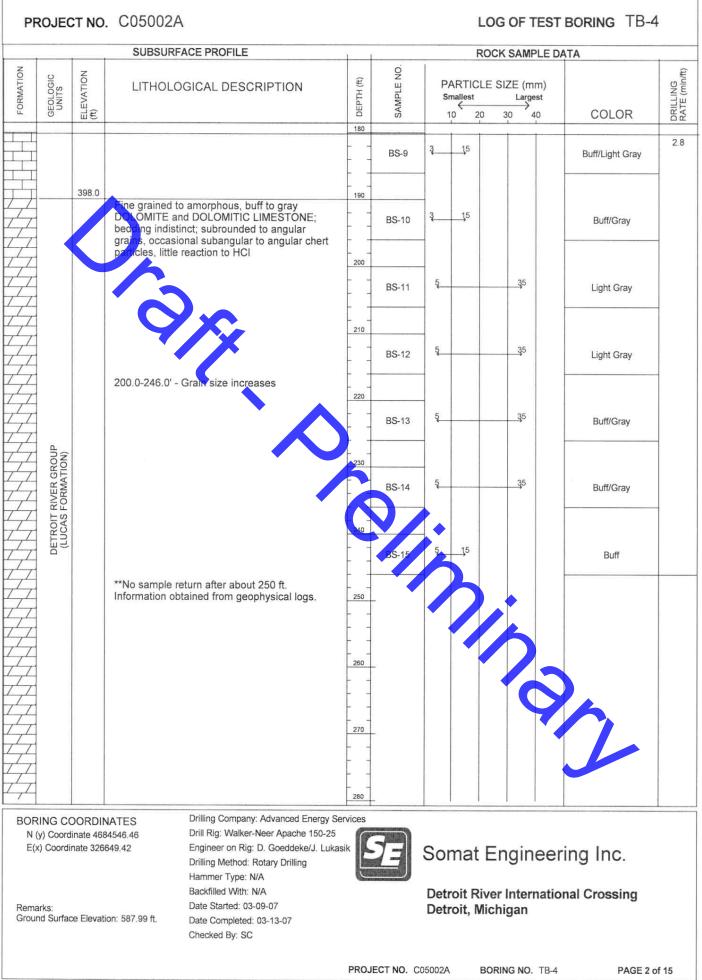








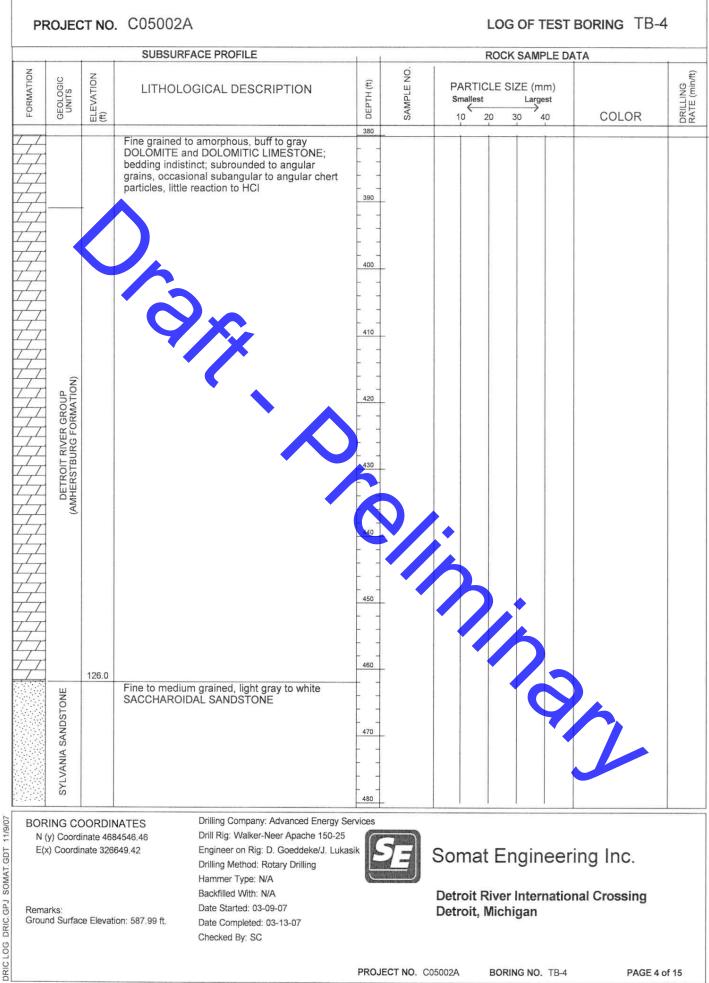






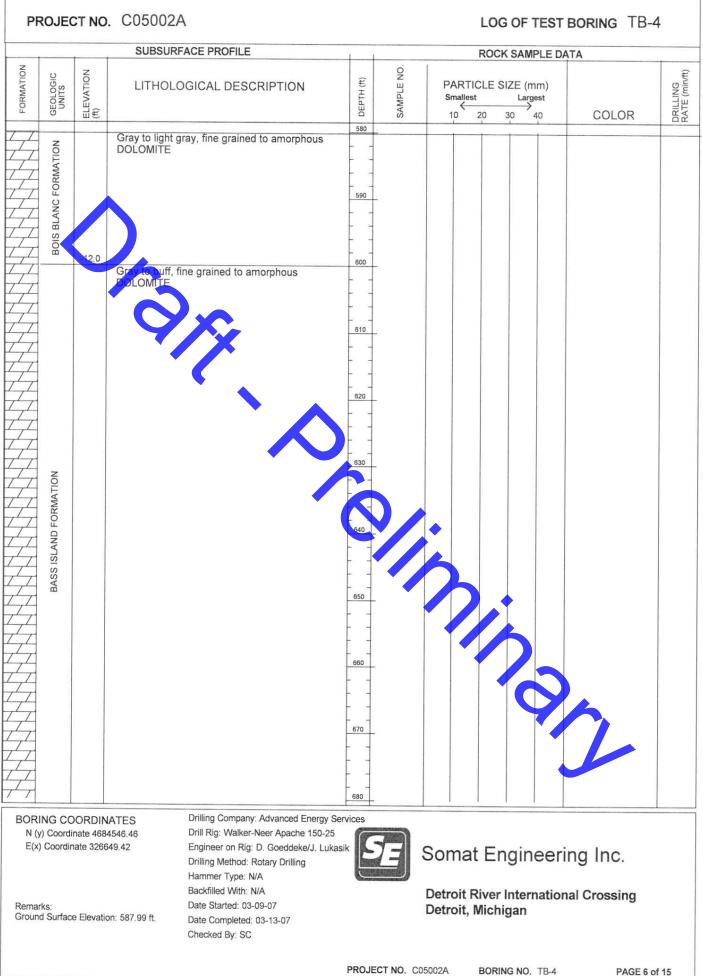
PROJECT NO. C05002A

PAGE 3 of 15



11/9/07



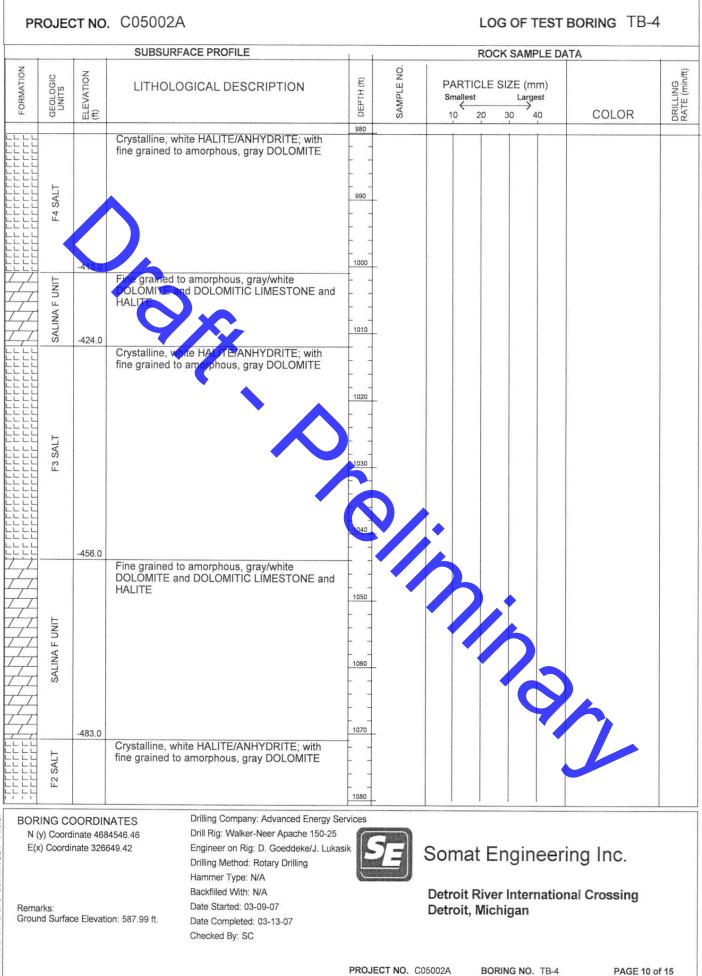


BORING NO. TB-4





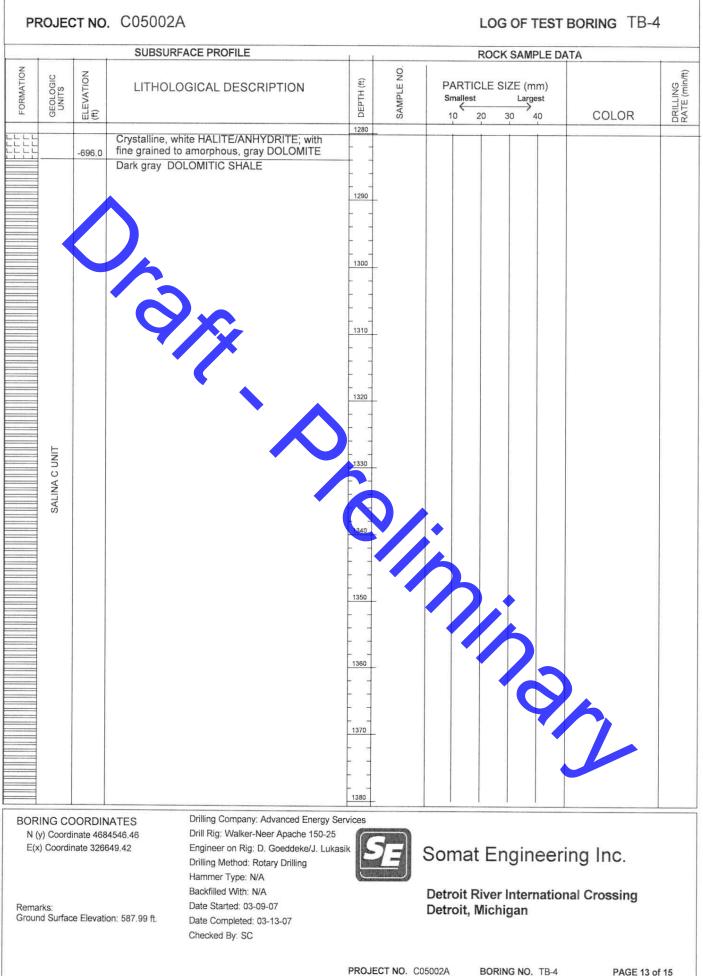


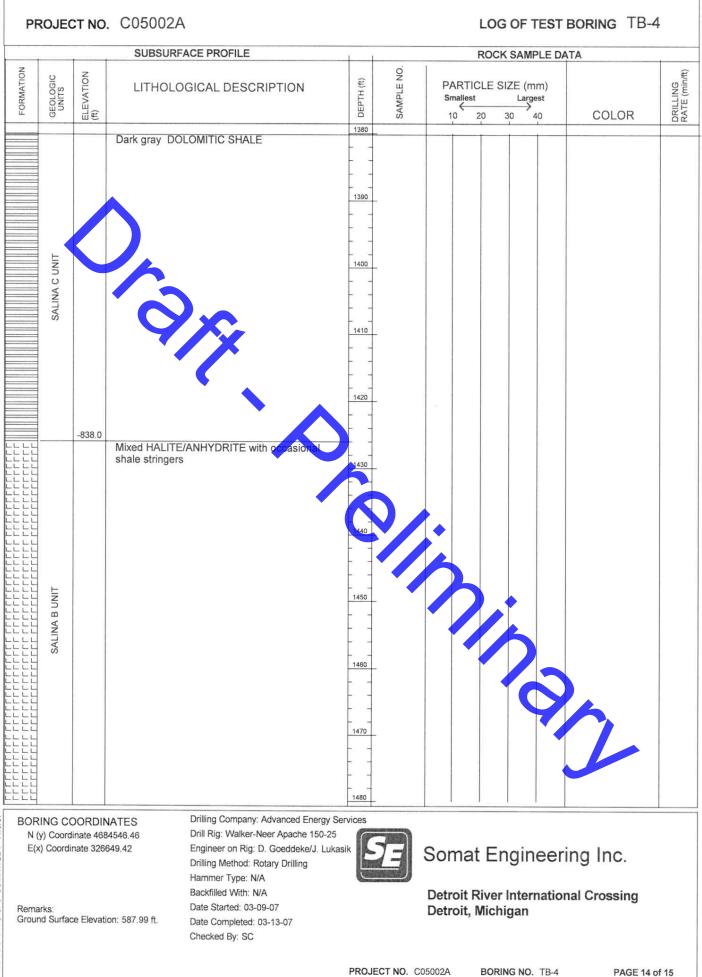


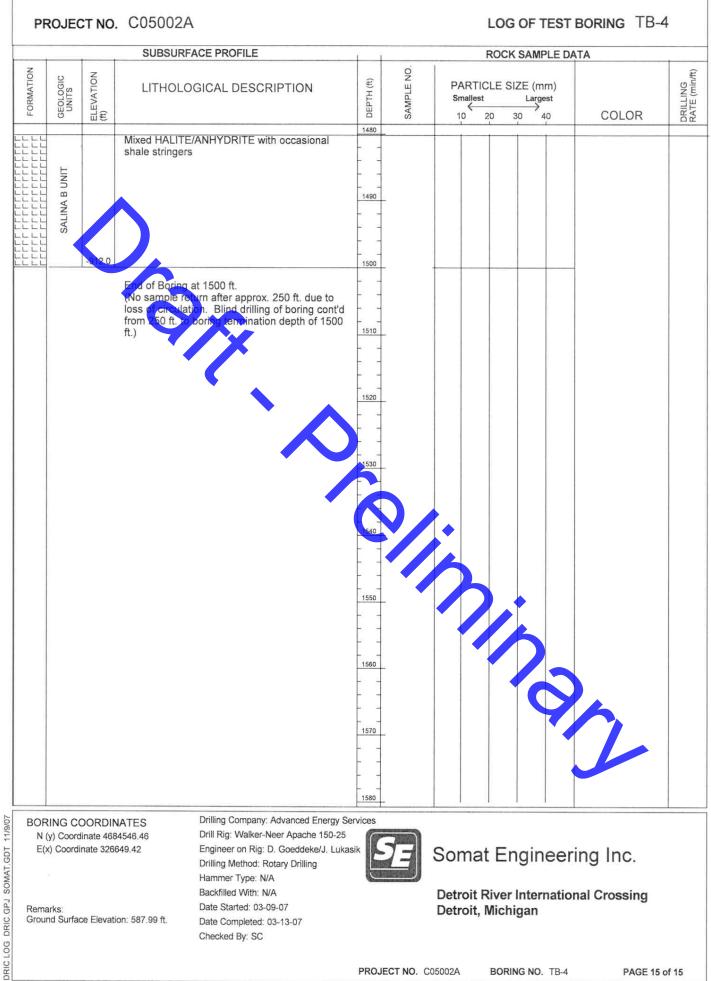


PF	ROJEC	T NO	C05002A					LOG OF	TEST	boring TE	3-4
			SUBSURFACE PROFILE					ROCK SA	MPLE DA	TA	
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLOGICAL DESC	RIPTION	DEPTH (ft)	SAMPLE NO.	PART Smalle:	ICLE SIZE ( st La	mm) rgest ≯ 40	COLOR	DRILLING RATE (min/ft)
	SALINA E UNIT	-665.0	Fine grained to amorphous, g DOLOMITE and DOLOMITIC HALITE	ray/white LIMESTONE and							
	D3 SALT	-677.0	Crystalline, white HALITE/ANI fine grained to amorphous, gra	ay DOLOMITE	  1260 	_					
	SALINA D UNIT	-685.0	Fine grained to amorphous, gr DOLOMITE and DOLOMITIC HALITE	LIMESTONE and	 1270	_				7_	
	D2 SALT		Crystalline, white HALITE/ANH fine grained to amorphous, gra	IYDRITE; with ay DOLOMITE	  1280	-					
BORING COORDINATES       Drilling Company: Advanced Energy Services         N (y) Coordinate 4684546.46       Drill Rig: Walker-Neer Apache 150-25         E(x) Coordinate 326649.42       Engineer on Rig: D. Goeddeke/J. Lukasik         Drilling Method: Rotary Drilling       Hammer Type: N/A         Backfilled With: N/A       Date Started: 03-09-07         Ground Surface Elevation: 587.99 ft.       Date Completed: 03-13-07         Checked By: SC       PROJECT NO. C									rnation	ng Inc. al Crossing PAGE 1	2 of 15

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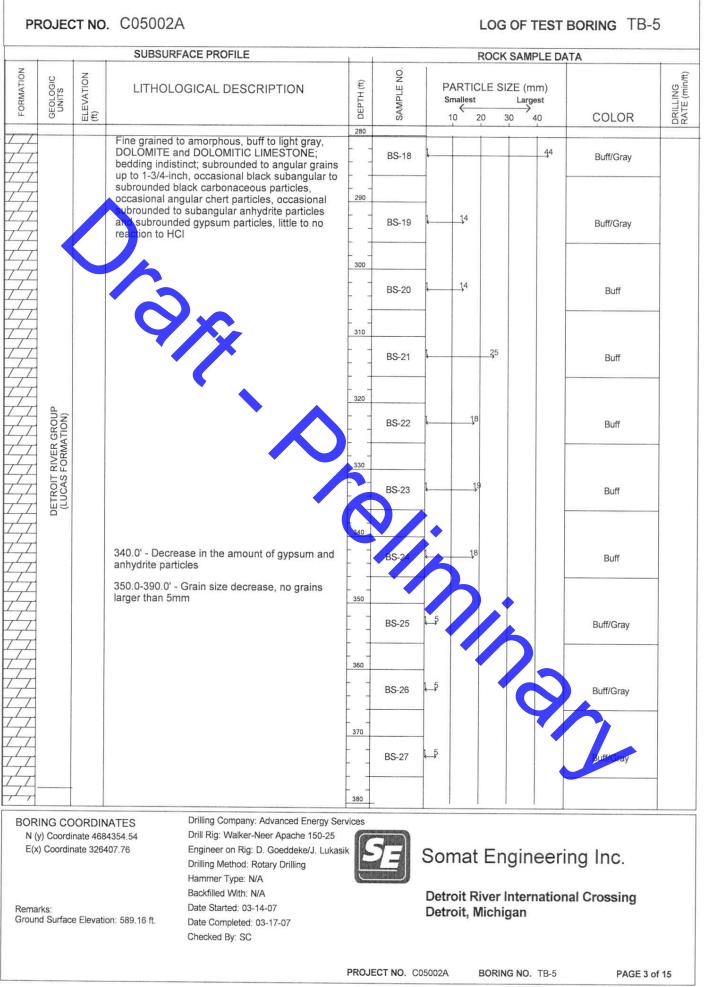


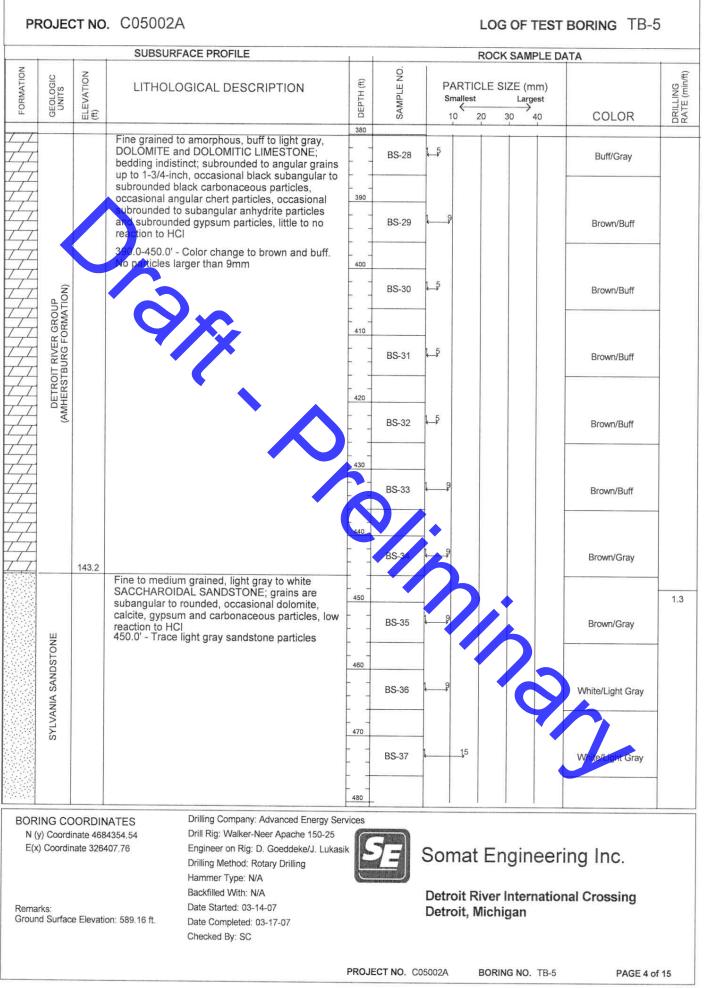


PR	ROJEC	T NO	. C05002A	Λ.				LOG OF	TEST	boring TB-	5
			SUBSURF	ACE PROFILE	+			ROCK SA	MPLE DA	TA	
FORMATION	GEOLOGIC	ELEVATION (ff)	LITHOLC	GICAL DESCRIPTION	DEPTH (ft)	SAMPLE NO.	PART Smalle	FICLE SIZE est La 20 30	(mm) argest → 40	COLOR	DRILLING RATE (min/ft)
THE			GLACIAL SEE	DIMENT (DRIFT)	80	-	+	1 1	1		
		500.2	LIMESTONE; subrounded gr	o amorphous, gray to buff bedding indistinct; angular to ains up to 3/4-inch; occasional lar to subrounded carbonaceous	90	-					
			patricles, occa occasional cal reaction with H	sional angular chert particles, cite particles, strong to moderate ICI	 <u>100</u>  	BS-1	1	5	-	Light Gray	3.4
	DNE			7	110	BS-2	-	_20	_	Buff/Gray	
	DUNDEE LIMESTONE				   130	BS-3	β β		-	Buff/Gray	
			150.0-160.0' -	Increase in hydrocarbon staining	140    	BS-4 BS-5 BS-6	β 13		_	Buff/Light Gray Buff/Gray Brown/Gray	-
		429.2	DOLOMITE an bedding indistii up to 1-3/4-incl subrounded bla occasional ang subrounded to and subrounde reaction to HCI	amorphous, buff to light gray, d DOLOMITIC LIMESTONE; not; subrounded to angular grains h, occasional black subangular to ack carbonaceous particles, jular chert particles, occasional subangular anhydrite particles d gypsum particles, little to no increase in hydrocarbon staining		BS-7 BS-8	- - - - - - -			Gray Suff/Errown	
N (y E(x) Remar	) Coordir rks:	nate 468 nate 3264	4354.54	Drilling Company: Advanced Energy Se Drill Rig: Walker-Neer Apache 150-25 Engineer on Rig: D. Goeddeke/J. Lukas Drilling Method: Rotary Drilling Hammer Type: N/A Backfilled With: N/A Date Started: 03-14-07 Date Completed: 03-17-07 Checked By: SC	G	5E	Detroit		ernation	ng Inc. al Crossing	
					PROJ	ECT NO. C	05002A	BORING N	0. TB-5	PAGE 1 o	of 15

PR	OJEC	T NO	. C05002A	Υ.				LOG	OF TEST	BORING TB-5	5
			SUBSURF	ACE PROFILE	1			ROCK	SAMPLE D	DATA	1
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLO	GICAL DESCRIPTION	DEPTH (ft)	SAMPLE NO.	PART Smalle	TICLE SIZ	Largest	COLOR	DRILLING RATE (min/ft)
			DOLOMITE ar bedding indisti up to 1-3/4-inc	o amorphous, buff to light gray, ad DOLOMITIC LIMESTONE; nct; subrounded to angular grains h, occasional black subangular to		BS-9	1 10			Buff/Brown/Gray	_
			occasional and subrounded to	ack carbonaceous particles, gular chert particles, occasional subangular anhydrite particles ed gypsum particles, little to no	<u>190</u>	BS-10	10	10		Light Gray/Buff	2.4
7					200	20.44	1 12				-
7			6	×.	210	BS-11	-			Light Gray/Buff	
	SOUP ON)				   220		_				
	DETROIT RIVER GROUP (LUCAS FORMATION)		220.0-250.0' - ( particles	Occasional large limestone	   230	BS-12	-	3		Gray/Buff	
	DETROIT R (LUCAS F					BS-13		20		Buff	
		subrounded gy	Occasional to frequent psum particles and subangular to		BS-14		20		Buff/Gray		
			subrounded anhydrite particles 260.0-350.0' - Occasional dolomite particles with "micro" pitting. Very thin to laminar bedding visible			BS-15				Buff/Gray/White	
				260	BS-16	- 	18	ム	Buff/Gray/White		
			270.0' - Decrea	ise in overall grain size	270	00.47		18			
					280	BS-17					
N (y	NG CC /) Coordi ) Coordir	nate 468	4354.54	Drilling Company: Advanced Energy Ser Drill Rig: Walker-Neer Apache 150-25 Engineer on Rig: D. Goeddeke/J. Lukas Drilling Method: Rotary Drilling Hammer Type: N/A	6	5E)				ring Inc.	
Remar Ground		e Elevati	on: 589.16 ft.	Backfilled With: N/A Date Started: 03-14-07 Date Completed: 03-17-07 Checked By: SC			Detroit Detroit,			nal Crossing	
					PRO.II	ECT NO. CO	5002A	BORING	<b>3 NO.</b> TB-5	PAGE 2 of	15

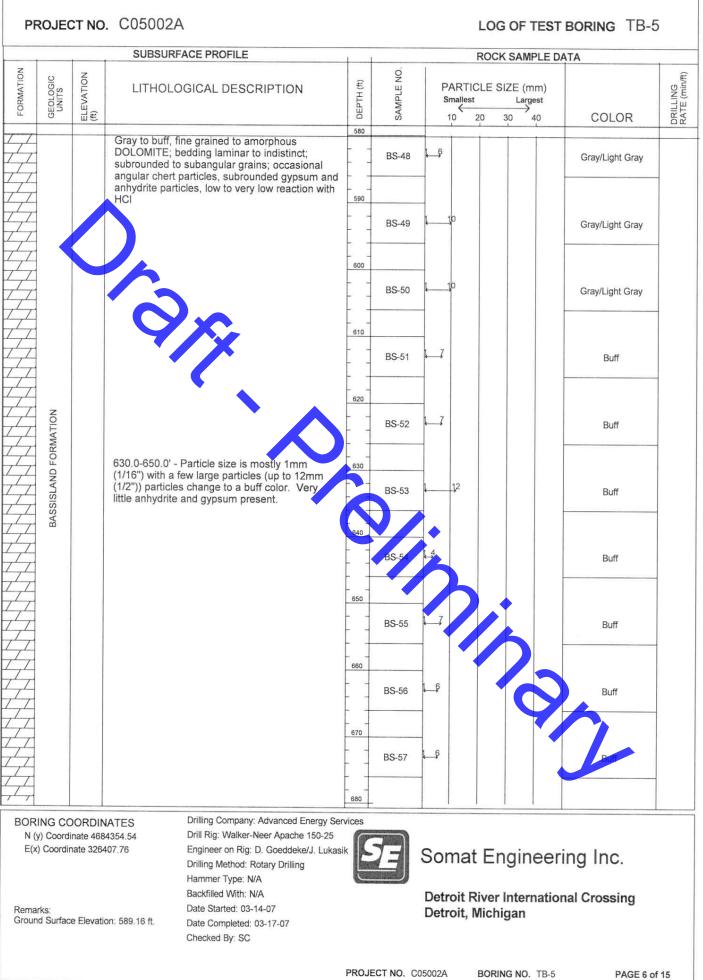
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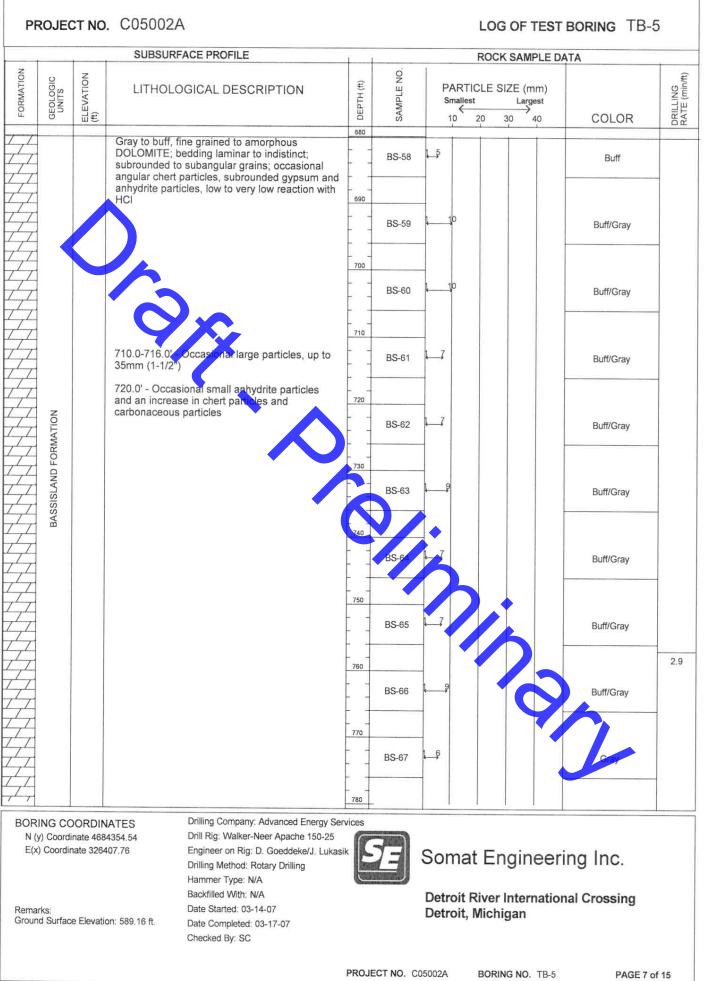


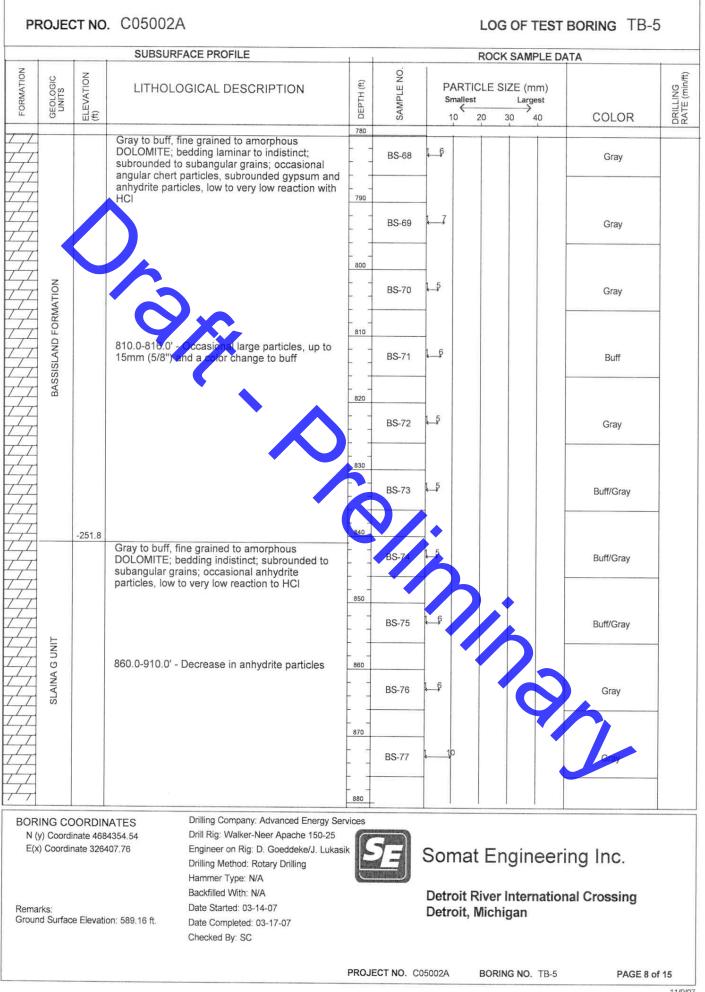


PF	ROJE	CT NO	. C05002A					L	og of	TEST	BORING TB-5	ō
			SUBSURFAC	E PROFILE	1			R	DCK SA	MPLE DA	ATA	
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLOGI	CAL DESCRIPTION	(ij) HLLH (ij)	SAMPLE NO.	Sr	ARTICLI		(mm) argest → 40	COLOR	DRILLING RATE (min/ft)
			SACCHAROIDAL subangular to rou calcite, gypsum a	rained, light gray to white SANDSTONE; grains are nded, occasional dolomite, nd carbonaceous particles, low		BS-38	↓₽				White/Light Gray	-
	IDSTONE		reaction to HCI		490	BS-39	1	14			White/Light Gray	
	SYLVANIA SANDSTONE		1		500	BS-40	9			White/Light Gray		
	S		8	\$	 510	BS-41	6				Mbite// intel 2	
		73.2	DOLOMITE; bedo subangular grains	tine grained to amorphous ling is indictinct; subrounded to ; occasional angular chert		въ-41					White/Light Gray	
			particles, subroun	ded gypsum and anhydrite ery low reaction to HCI		BS-42	<b>↓</b> 5				White/Light Gray	2.2
	DRMATION				530	BS-43	5				Gray/Light Gray	-
	30IS BLANC FORMATION				540  	BS-44	L_6				Gray/Light Gray	
	8				550	BS-45	L.F	2			Gray/Light Gray	
		25.2	Grav to buff, fine of	grained to amorphous	<u>560</u>	BS-46	<b>1</b> 6				Gray/Light Gray	
			DOLOMITE; bedd subrounded to sub angular chert parti anhydrite particles HCI	ing laminar to indistinct; bangular grains; occasional cles, subrounded gypsum and s, low to very low reaction with	570	BS-47	- 				Gray/White/Light	3.2
7			occasional gypsur	ed dolomite and sandstone, n and calcite particles	580	-						
N ()	/) Coord	DORDIN inate 468 nate 3264	4354.54 Dr 407.76 Er Dr Ha	illing Company: Advanced Energy Ser ill Rig: Walker-Neer Apache 150-25 ngineer on Rig: D. Goeddeke/J. Lukas illing Method: Rotary Drilling ammer Type: N/A	G	E			-		ing Inc.	
Remar Groun		e Elevati	Da on: 589.16 ft. Da	ackfilled With: N/A ate Started: 03-14-07 ate Completed: 03-17-07 necked By: SC				oit Riv oit, Mie			nal Crossing	
					PROJI	ECT NO. CO	)5002A	вс	RING N	0. TB-5	PAGE 5 of	f 15

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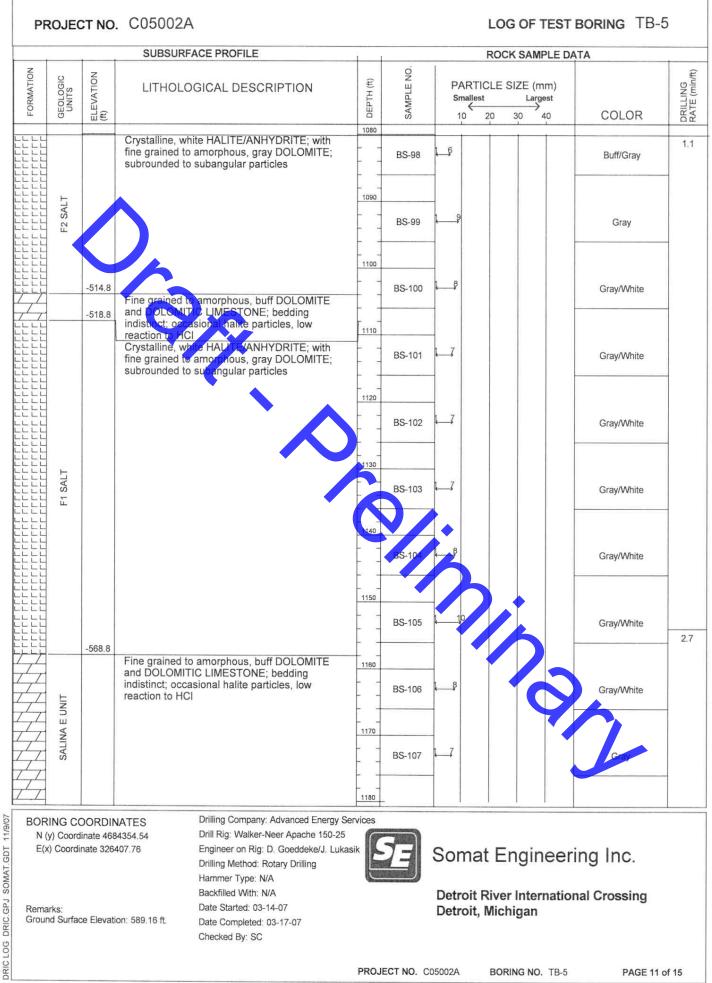


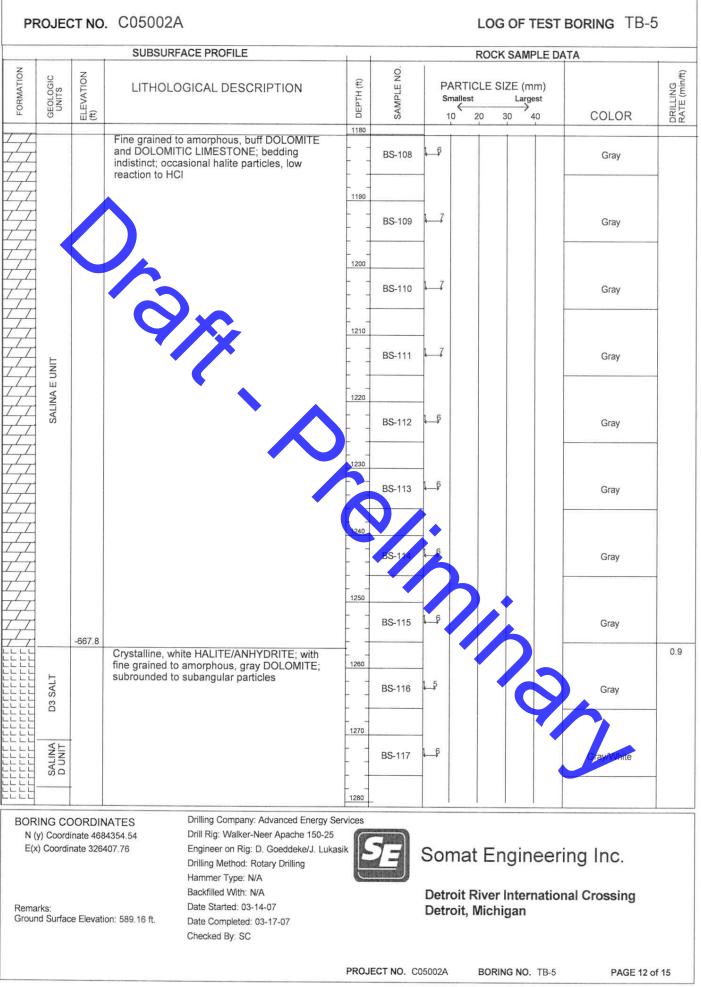


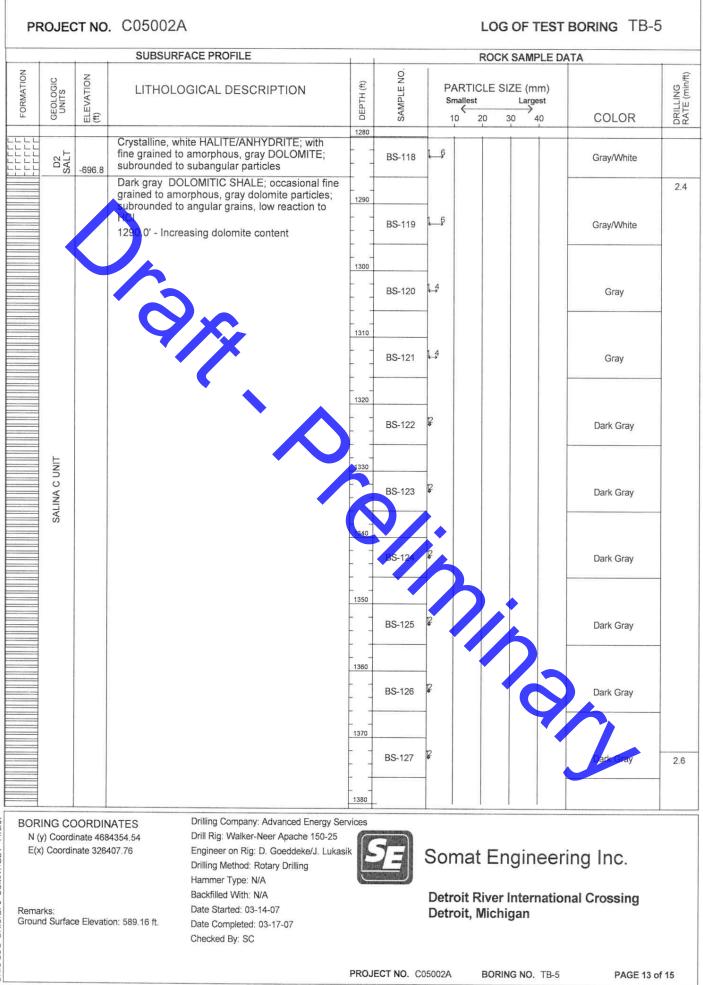
PF	ROJEC	CT NO	. C05002A			LOG OF	TEST BORING TB-5	
			SUBSURFACE PROFILE	-	1	ROCK SAM	IPLE DATA	
FORMATION	GEOLOGIC	ELEVATION (ff)	LITHOLOGICAL DESCRIPTION	DEPTH (ft)	SAMPLE NO.	PARTICLE SIZE (n smallest Larg 10 20 30		DRILLING RATE (min/ft)
			Gray to buff, fine grained to amorphous DOLOMITE; bedding indistinct; subrounded to subangular grains; occasional anhydrite particles, low to very low reaction to HCI		BS-78	1_6 	Gray	
	SLAINA G UNIT			 	BS-79	6	Gray	
	SLAIN			900	BS-80	1-5	Gray	
		-320.8	910.0 Increase in aphydrite particles Crystalling, white HALTE/ANHYDRITE; with fine grained to amorphous, gray DOLOMITE;	910				0.8
			subrounded to subangular particles	920	BS-81		Gray	
					BS-82	<u>↓_</u> 7 -	Gray	
	F6 SALT			930	BS-83	- Ιβ	Gray/White	
				940	BS-84	β	Gray/White	
	E	-365.8	Fine grained to amorphous, gray/white	950	BS-85		Gray/White	
	SALINA F UNIT	-375.8	DOLOMITE and DOLOMITIC LIMESTONE and HALITE; subrounded to angular grains, occasional angular chert particles, very low reaction to HCI	 960	BS-86		Gray/White	2.7
	F5 SALT S	-382.8	Crystalline, white HALITE/ANHYDRITE; trace fine grained to amorphous, gray DOLOMITE; subrounded to subangular particles	970				
			Fine grained to amorphous DOLOMITE and DOLOMITIC LIMESTONE; bedding indistinct; subangular to angular particles, occasional halite particles, low to very low reaction to HCI	980	BS-87	- 10 -	Crav/White	
N () E(x) Remai	y) Coordi ) Coordir rks:	DORDIN inate 468 nate 3264 e Elevati	4354.54 Drill Rig: Walker-Neer Apache 150-25	6	5 <u>e</u>	Somat Engin Detroit River Inter Detroit, Michigan	neering Inc.	
				PROJ	ECT NO. C	05002A BORING NO.	. TB-5 PAGE 9 of 1	11/9/07

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PR	OJEC	CT NO	. C05002A					LOG	OF TEST	BORING TB-	5
			SUBSURF	ACE PROFILE	-++			ROCK	SAMPLE DA	TA	-
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLO	GICAL DESCRIPTION	DEPTH (ft)	SAMPLE NO.	PAF Sma 10	RTICLE SIZ	Largest	COLOR	DRILLING RATE (min/ft)
	SALINA F UNIT		DOLOMITIC LI subangular to a	amorphous DOLOMITE and MESTONE; bedding indistinct; angular particles, occasional low to very low reaction to HCI	980	BS-88	<b>↓</b> β			Buff/Gray	
	SAL	-399.8	fine grained to	te HALITE/ANHYDRITE; with amorphous, gray DOLOMITE; subangular particles	990	BS-89	16			Buff/Gray	1.4
	F4 SALT		1		1000	BS-90	- 1			Gray/White	
		-422.8	Q	×.	  _ 1010						
	SALINA F UNIT		Fine grained and DOLOMIT indistinct; occa reaction to HCI	amorphous, buff DOLOMITE IC HMESTONE; bedding signal halite particles, low		BS-91	↓9 			Gray/White	_
	SALIN	-433.8	fine grained to	te HALITE/ANHYDRITE, with amorphous, gray DOLONITE; subangular particles	1020	BS-92	- - -		-	Gray/White	
	F3 SALT					BS-93 BS-94			-	Gray Gray/White	
		-461.8	and DOLOMITI indistinct; occa	amorphous, buff DOLOMITE C LIMESTONE; bedding sional halite particles, low	1050	BS-95				Gray/White	2.0
	SALINA F UNIT		reaction to HCI		 1060 	BS-96	_ 18		6	Gray/White	
	SALIN				1070	BS-97	?			Butti	
7		-490.8	1080.0-1090.0'	- Sandy texture	1080	-					
BORING COORDINATES       Drilling Company: Advanced Energy S         N (y) Coordinate 4684354.54       Drill Rig: Walker-Neer Apache 150-25         E(x) Coordinate 326407.76       Engineer on Rig: D. Goeddeke/J. Luk         Drilling Method: Rotary Drilling       Hammer Type: N/A         Backfilled With: N/A       Date Started: 03-14-07         Ground Surface Elevation: 589.16 ft.       Date Completed: 03-17-07					F	DE)	Detro	5	ternation	ng Inc. nal Crossing	
					PROJE	E <b>CT NO</b> . C	05002A	BORING	<b>NO.</b> TB-5	PAGE 10	of 15





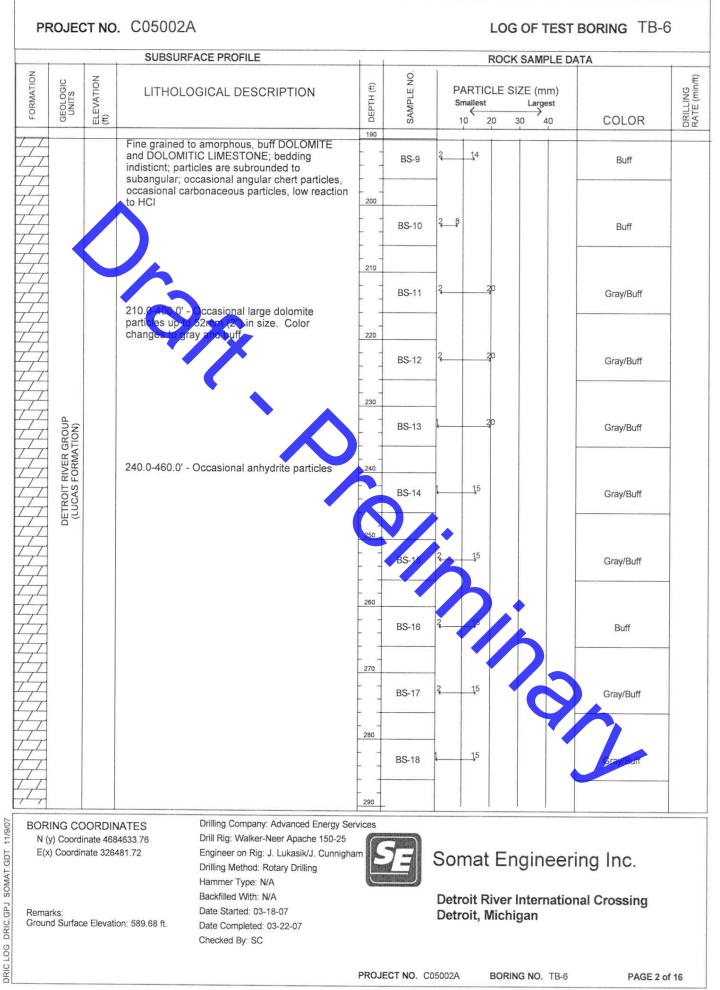


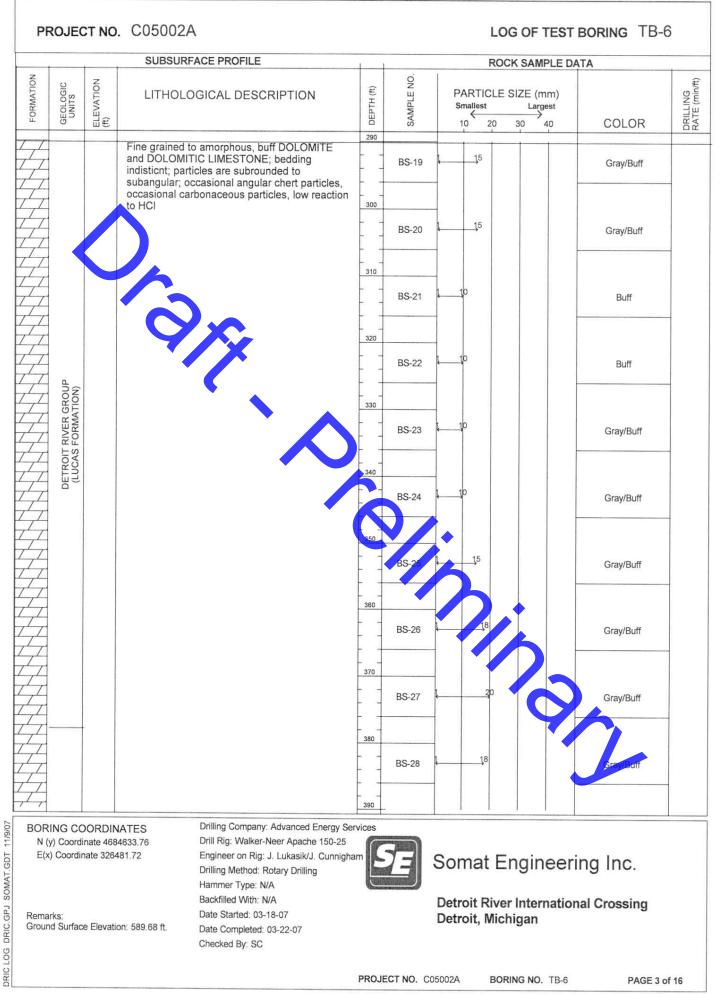


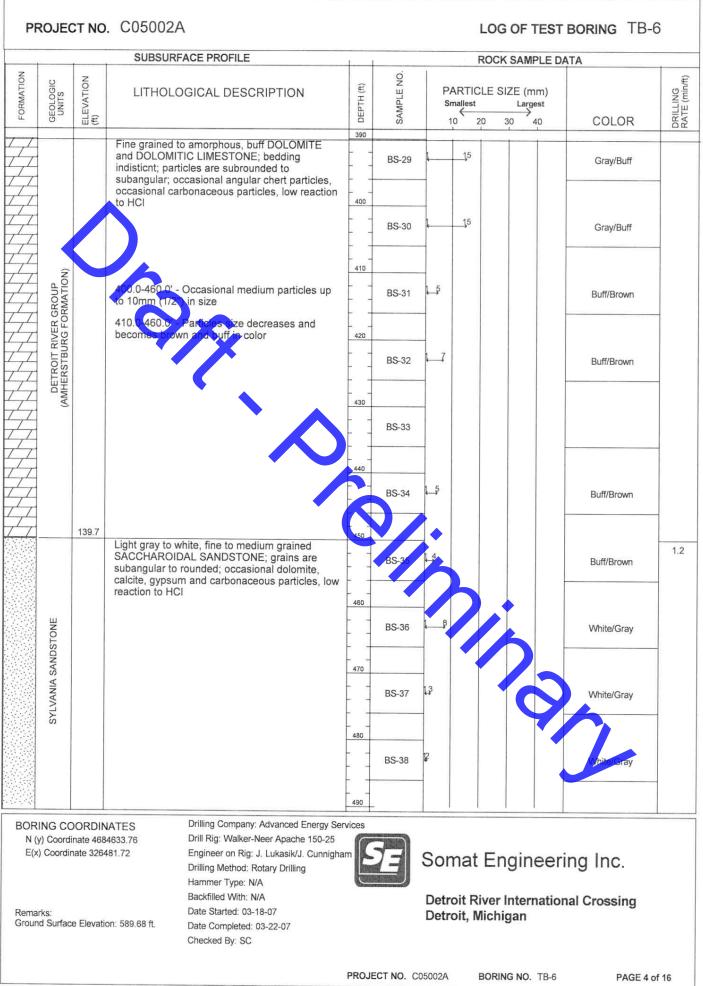


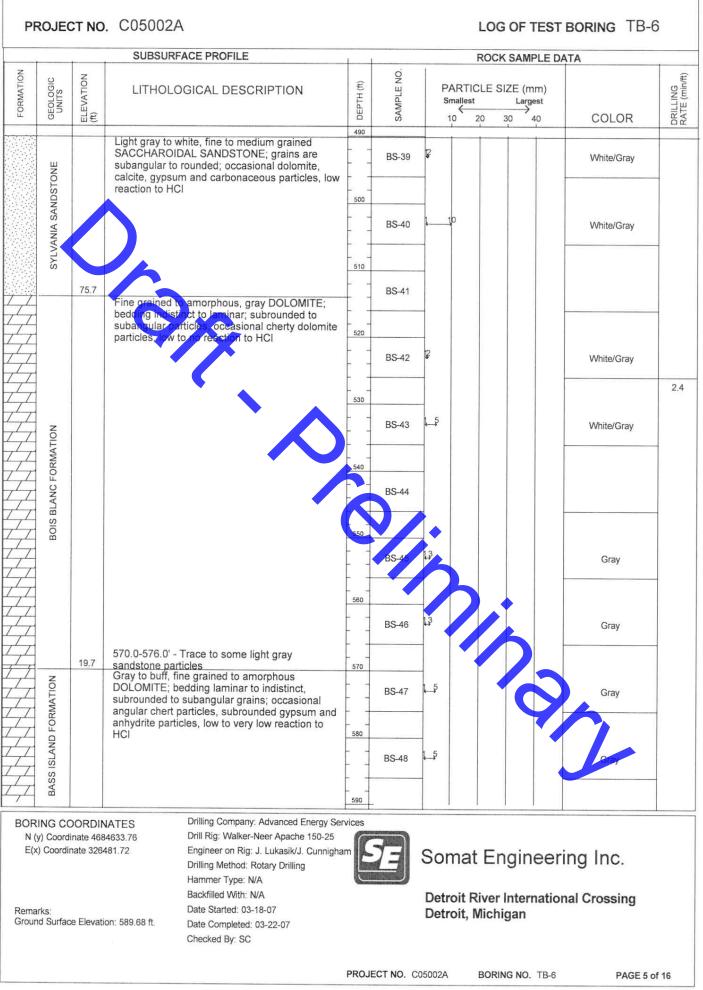
PF	ROJEC	CT NO	. C05002A	A					LOG	OF TE	ST BOF	ring TB-	6
			SUBSURF	ACE PROFILE				1	ROCK	SAMPLE	E DATA		
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLC	GICAL DESCRIPTION	DEPTH (ft)	SAMPLE NO.	6	PARTICLE SIZE (mm) Smallest Largest 10 20 30 40				COLOR	DRILLING RATE (min/ft)
		495.7	GLACIAL SEE	DIMENT (DRIFT)	90								
			LIMESTONE; subrounded to	o amorphous, gray and buff bedding indistinct; particles are subangular; occasional calcite isional angular chert particles, tion to HCI	100								
			1			BS-1			25			Gray/Buff	3.8
	111		120.0-150.0'-	Particles are mostly buff in color	  120								
	DUNDEE LIMESTONE					BS-2	2	<u>1</u> 4				Buff	
	DUNDEE L				 130  	BS-3		14				Buff	_
					140	BS-4	3	_14				Buff	
						BS-5	3	_14				Buff	2.7
		429.7	and DOLOMIT indisticnt; parti subangular; oc	amorphous, buff DOLOMITE IC LIMESTONE; bedding cles are subrounded to ccasional angular chert particles,	160	BS-6	33	10				Buff	
			occasional car to HCl	bonaceous particles, low reaction	 170	BS-7	3	_14			5	Buff	
			180.0-210.0' -	Particles become more angular	 180  	BS-8	2	14				Burr	
	ING CC		JATES 4633.76	Drilling Company: Advanced Energy Se Drill Rig: Walker-Neer Apache 150-25	190 ervices								
E(x Rema	:) Coordir rks:	nate 3264		ham E	DE	Detr	oit R		nterna	ering tional C	Inc. Crossing		
					PROJ	ECT NO. CO	05002A		BORIN	<b>g no</b> . Te	3-6	PAGE 1	of 16

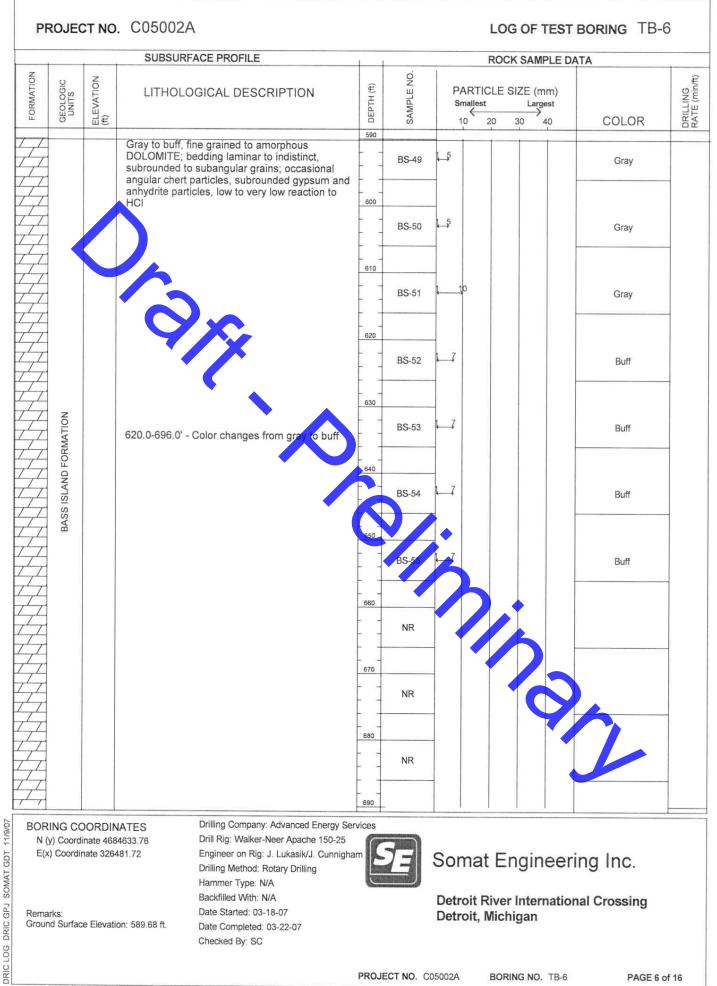
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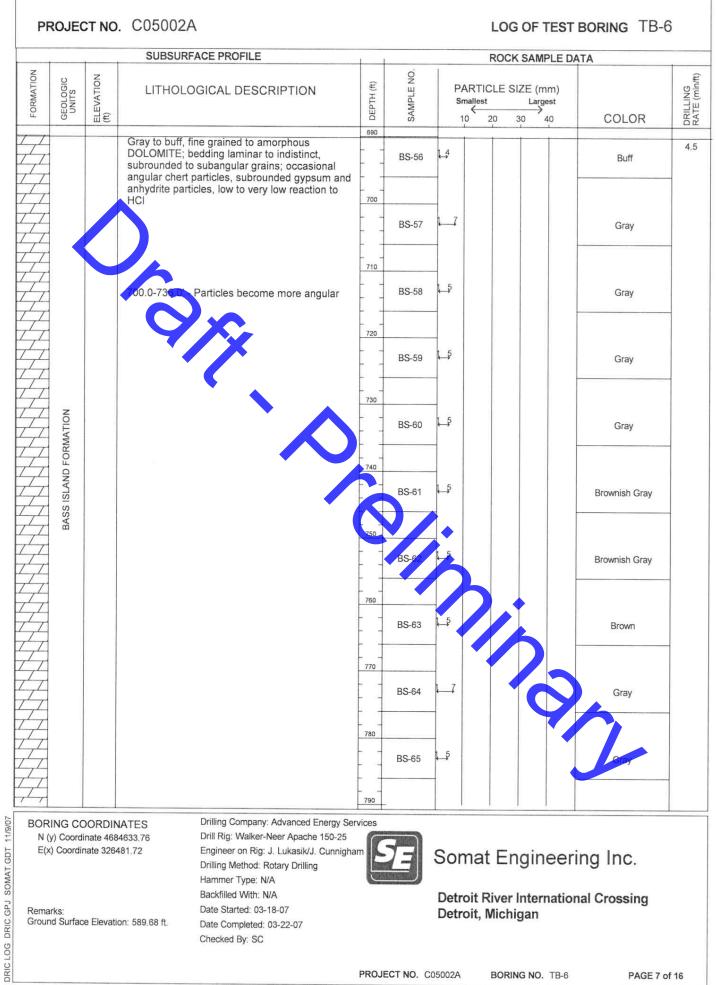


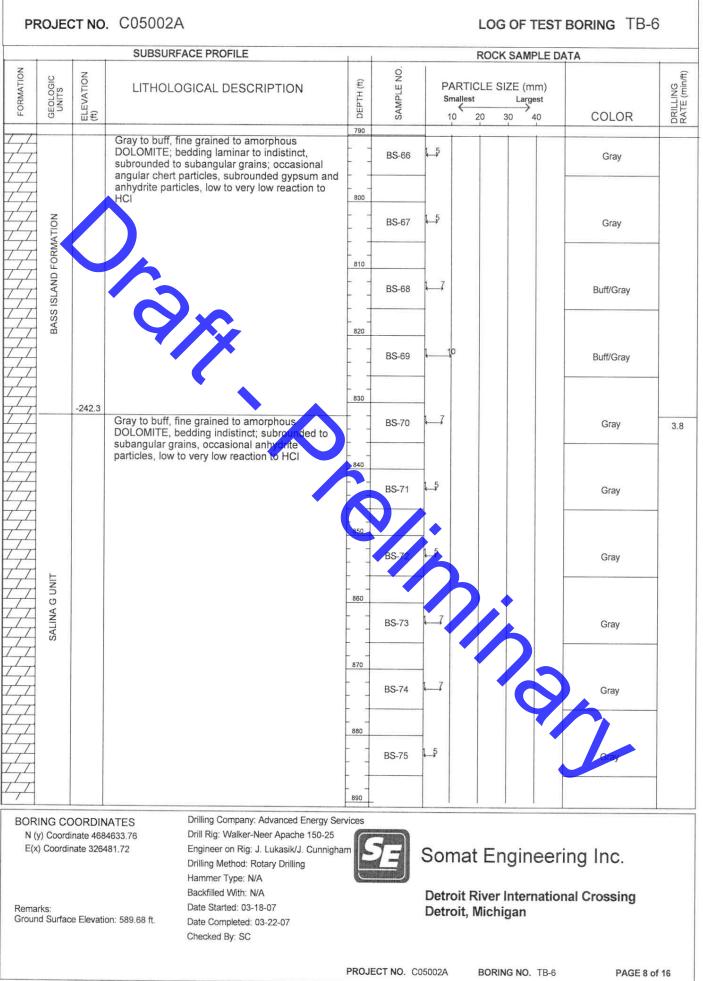






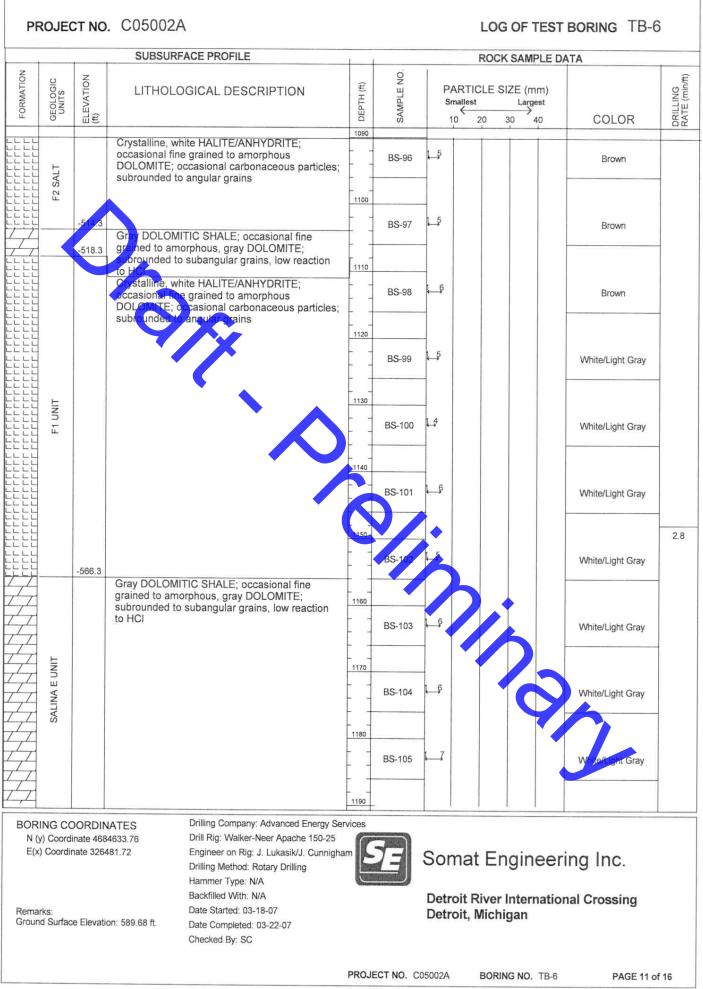




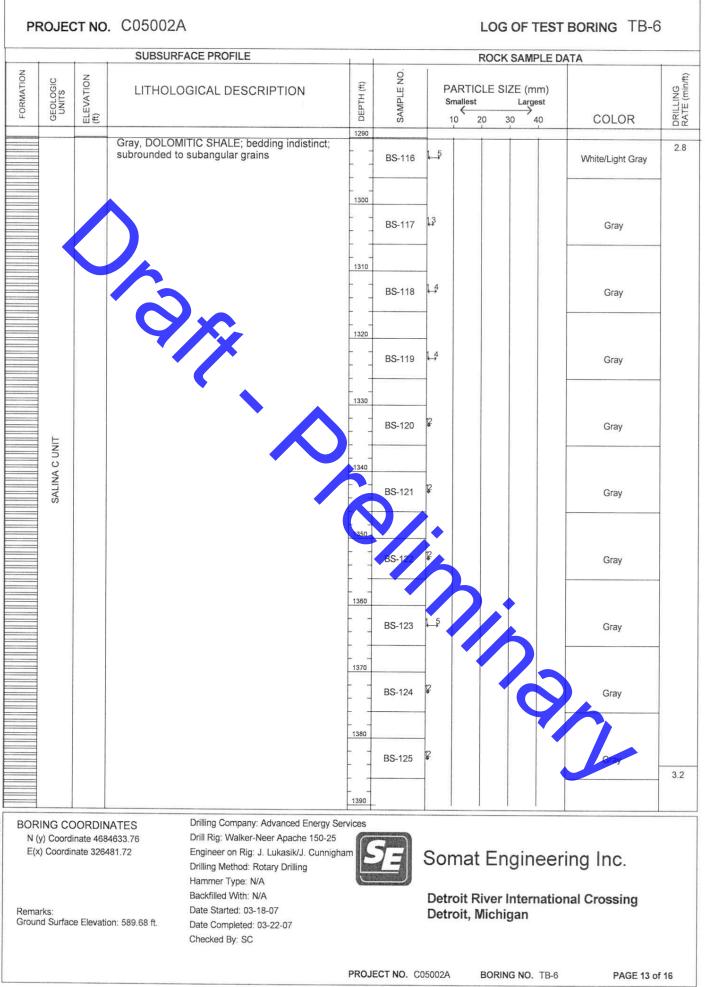


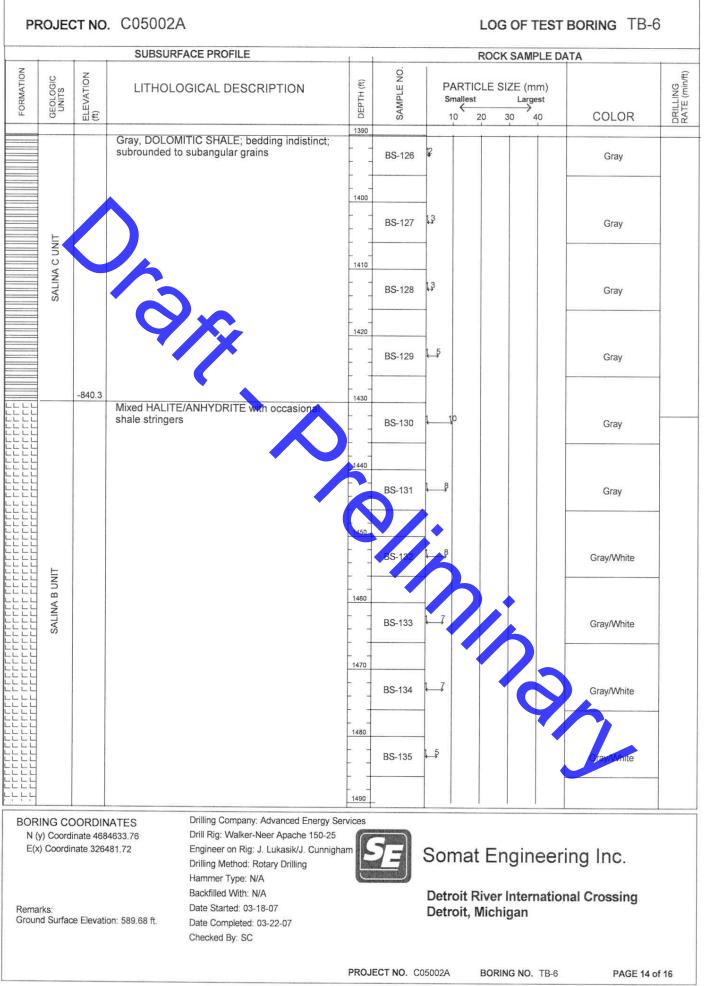
PR	OJE	CT NO	. C05002A				LOG OF TE	ST BORING TB-6	
			SUBSURFACE PROFILE		[		ROCK SAMPLE	DATA	
FORMATION	GEOLOGIC	ELEVATION (ff)	LITHOLOGICAL DESCRIPTION	068 DEPTH (ft)	SAMPLE NO.		RTICLE SIZE (mm)	COLOR	DRILLING RATE (min/ft)
			Gray to buff, fine grained to amorphous DOLOMITE, bedding indistinct; subrounded to subangular grains, occasional anhydrite particles, low to very low reaction to HCI		BS-76	1_4		Gray	
	G UNIT			900	BS-77	17		Gray	
	SALINA (			910	BS-78	1_5		Gray	
		-334.3	Q/S	 920	BS-79	5			
		-336.3	Fine grained to an orphous, brown DOLOMITE; bedding indistinct, occasional halite, anhydrite and chert particles, low reaction to HCI Crystalline HALITE/ANHYDR DE; occasional fine grained, gray DOLOMITE; particles are	930	DO-19			Gray	0.9
	F6 SALT		fine grained, gray DOLOMITE; particles are subangular, no reaction to HCI	   940	BS-80	5		Gray	
	щ	201.0			BS-81	5		Gray	
	SALINA F UNIT	-361.3	Fine grained to amorphous, brown DOLOMITE; bedding indistinct; occasional halite, anhydrite and chert particles, low reaction to HCI Crystalline, white HALITE/ANYHDRITE and fine		BS-82			Gray	2.5
	F5 SALT		grained to amorphous, gray DOLOMITE; subrounded to subangular particles	960	BS-83			Gray	
	NIT	-380.3	970.0-976.0' - Overal particles size decreases Fine grained to amorphous, buff to gray DOLOMITE; bedding indistinct; occasional	970	BS-84	4		Gray	
	SALINA F UNIT		chert, halite and anhydrite particles, low reaction to HCI 980.0-986.0' - Occasional halite and anhydrite particles. Color changes to brown.	980					
	F4 SALT	-394.3			BS-85	<b>↓</b> 4		BrowniGray	10
N (y E(x) Remar	ING CC /) Coordi ) Coordir rks:	DORDIN nate 468 nate 3264 e Elevatio	4633.76 Drill Rig: Walker-Neer Apache 150-25	C	SE)	Detro	nat Enginee it River Internat it, Michigan	•	1.2
				PROJ	ECT NO. C	05002A	BORING NO. TB	-6 PAGE 9 of	16

PF	ROJEC	CT NO	. C05002A					L	OG O	F TEST	BORING TB-6	6
		1	SUBSURF	ACE PROFILE	1	1		R	DCK SA	MPLE D	ĄTA	
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLO	GICAL DESCRIPTION	DEPTH (ft)	SAMPLE NO.	S	ARTICLI		(mm) argest → 40	COLOR	DRILLING RATE (min/ft)
			occasional fine DOLOMITE: o	ite HALITE/ANHYDRITE; grained to amorphous ccasional carbonaceous particles; angular grains ' - Increase in small halite and	990	BS-86	1_5				Brownish Gray	
	F4 SALT		1000.0-1010.0 anhydrite partie	<ul> <li>Increase in small halite and cles. Increase in chert particles.</li> </ul>	 	BS-87	↓5				Gray	
		420.3	DOLOMITE: b	amorphous, buff to gray edding indistinct; occasional	1010	BS-88	14				Gray	
	SALINA F UNIT	-432.3	chert, halite an to HCI	d anhydrite particles, low reaction	1020							
			occasional fine	Ite HALITE/ANHYDRITE; grained to amorphous ccasional carbonaceous particles; angular grains		BS-89	<b>↓</b> 5				White/Light Gray	-
	F3 SALT			`∧`		BS-90	- - -				White/Light Gray	
		-460.3			1040	BS-91	5				White/Light Gray	
			grained to amo	FIC SHALE; occasional fine rphous, gray DOLOMITE; subangular grains, low reaction		BS-92	L_6				White/Light Gray	4.3
	SALINA F UNIT				1060	BS-93	L4				Brown	
	S	-488.3			1070	BS-94	<b>1</b> _6			0	Brown	
	F2 SALT	100.0	occasional fine	te HALITE/ANHYDRITE; grained to amorphous :casional carbonaceous particles; angular grains		BS-95	6				Prown	0.9
N (y E(x) Remar	/) Coordi ) Coordir rks:	DORDIN nate 468 nate 3264 e Elevatio	4633.76	Introduction of the second sec	5 <u>E</u>	Detr		er Int	ernation	ing Inc. nal Crossing		
					PROJ	IECT NO. CO	)5002A	вс	ORING N	<b>IO.</b> TB-6	PAGE 10 o	f 16

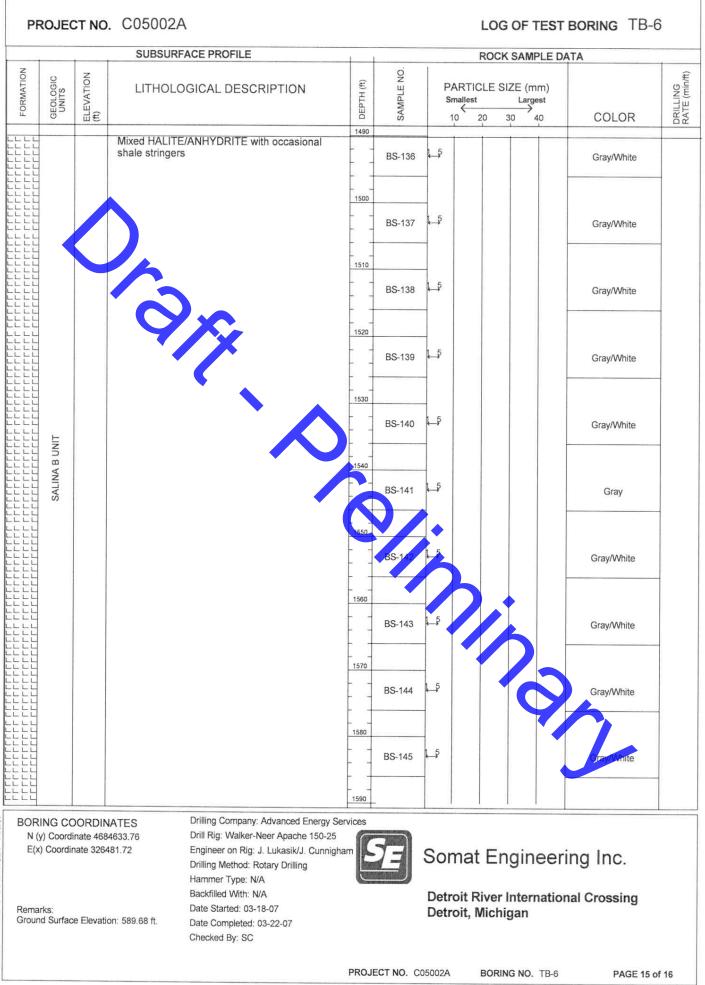


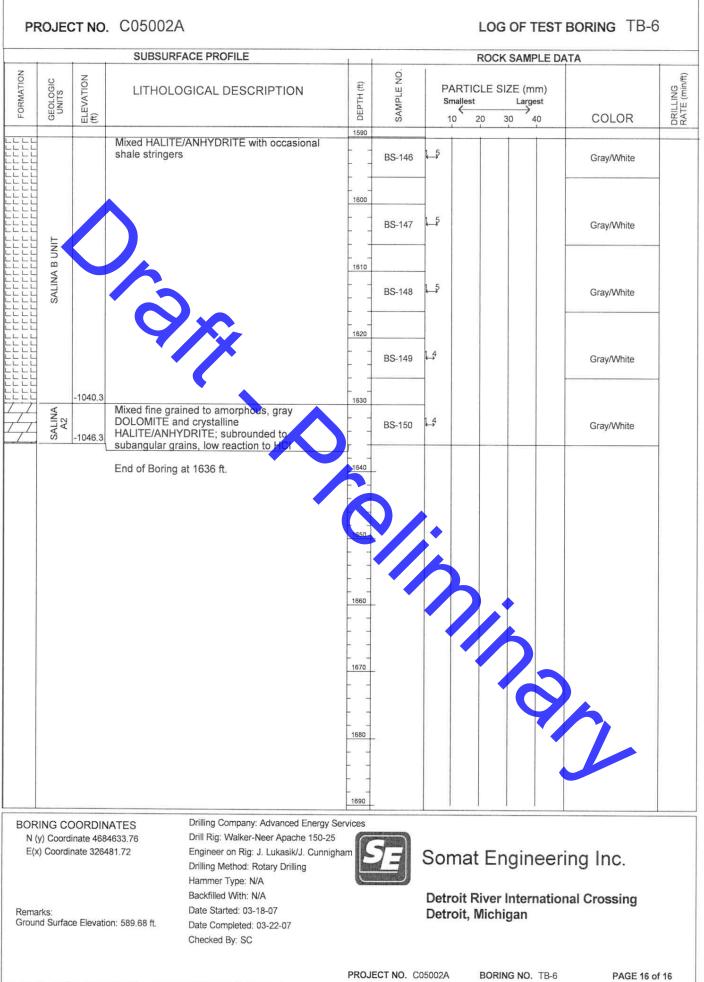
PR	ROJEC	T NO	C05002A					LOG 0	F TEST B	ORING TB-	6
			SUBSURFACE PROFILE		1			ROCK S	MPLE DAT	A	1
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLOGICAL DESCRIPTION		(1) HLd30 1190	SAMPLE NO.	PARTICLE SIZE (mm) Smallest Largest 10 20 30 40			COLOR	DRILLING RATE (min/ft)
			Gray DOLOMITIC SHALE; occasio grained to amorphous, gray DOLOI subrounded to subangular grains, lo to HCI	nal fine MITE;		BS-106	1_5			Buff/Gray	
				-	1200	BS-107	 ↓_4			Gray	
				-	1210	BS-108	1_5			Gray	
	TINL		<b>Y</b>	-	1220						
	SALINA E UNIT			-		BS-109	<b>↓</b> 5 _			Gray	
	S				1230	BS-110	_ 1_5 -			Gray	_
					1240	BS-111	5			Gray	
		-668.3		-		BS-112	L 7			Buff/Gray	_
	D3 SALT		Crystalline, white HALITE/ANHYDR occasional fine grained to amorpho DOLOMITE; occasional carbonaced subrounded to angular grains	us	1260	BS-113	1_7			Gray	2.6
	SALINA D UNIT	-680.3	Gray DOLOMITIC SHALE; occasion grained to amorphous, gray DOLOM subrounded to subangular grains, lo to HCI	nal fine MITE:	1270	BS-114	-7			Gray	
	D2 SALT	-689.3	Crystalline, white HALITE/ANHYDR occasional fine grained to amorpho DOLOMITE; occasional carbonaced subrounded to angular grains	us L	1280	BS-115	7			Gray	_
BORING COORDINATES       Drilling Company: Advanced Energy Serv.         N (y) Coordinate 4684633.76       Drill Rig: Walker-Neer Apache 150-25         E(x) Coordinate 326481.72       Engineer on Rig: J. Lukasik/J. Cunnighar         Drilling Method: Rotary Drilling       Hammer Type: N/A         Backfilled With: N/A       Backfilled With: N/A         Remarks:       Date Started: 03-18-07         Ground Surface Elevation: 589.68 ft.       Date Completed: 03-22-07         Checked By: SC       SC											
				F	PROJE	ECT NO. C	)5002A	BORING	NO. TB-6	PAGE 12	of 16

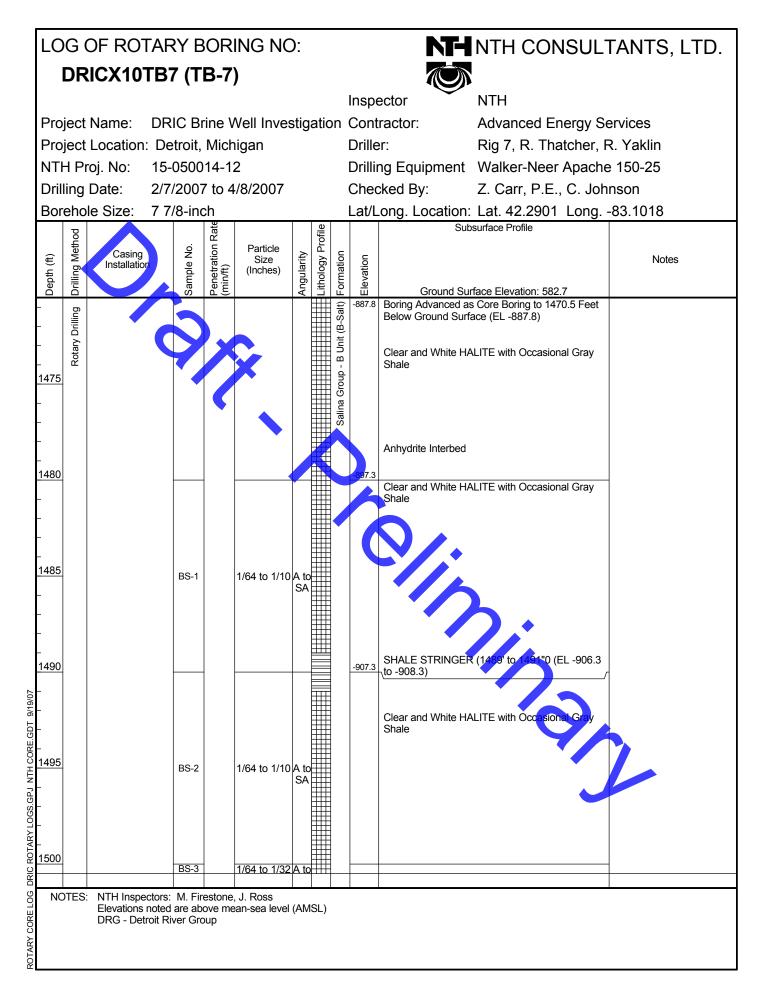


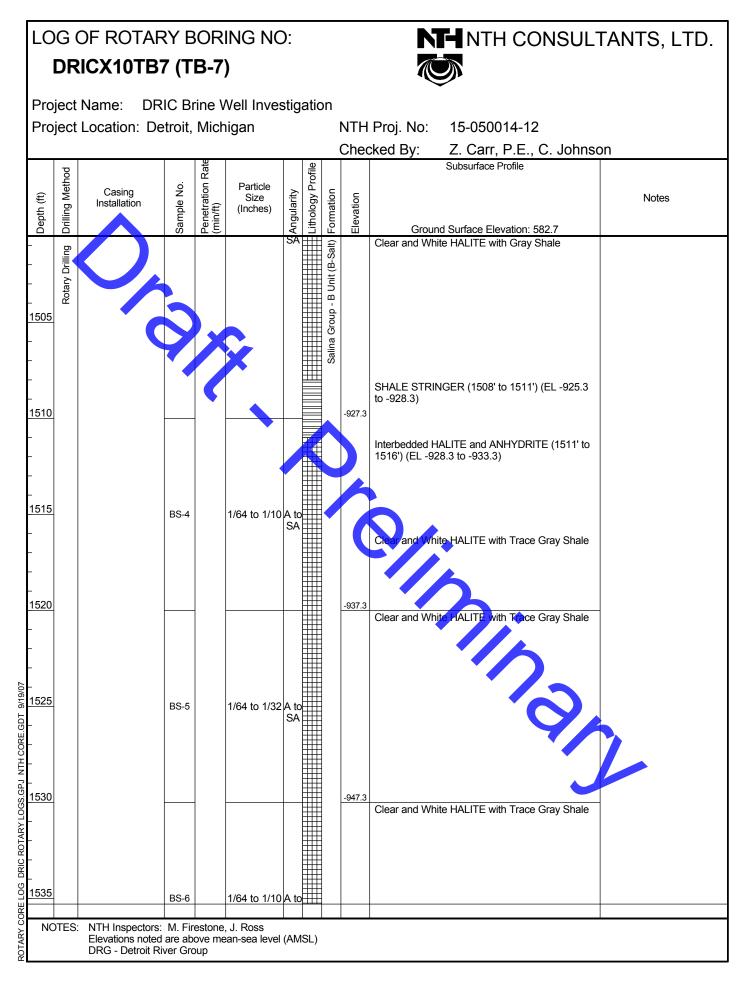


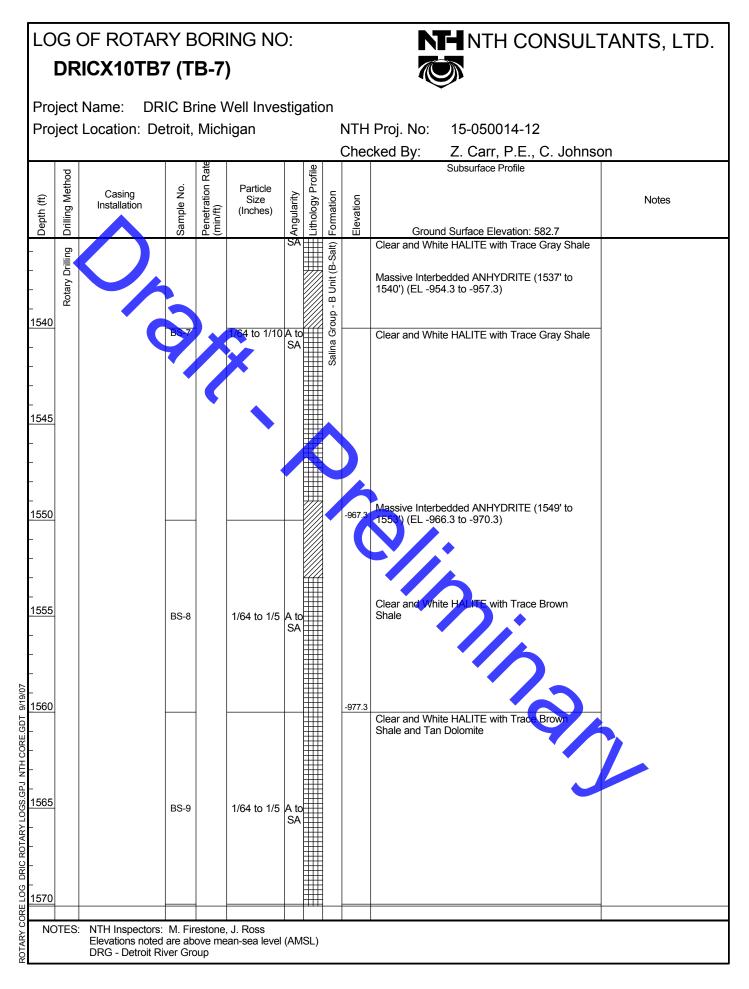
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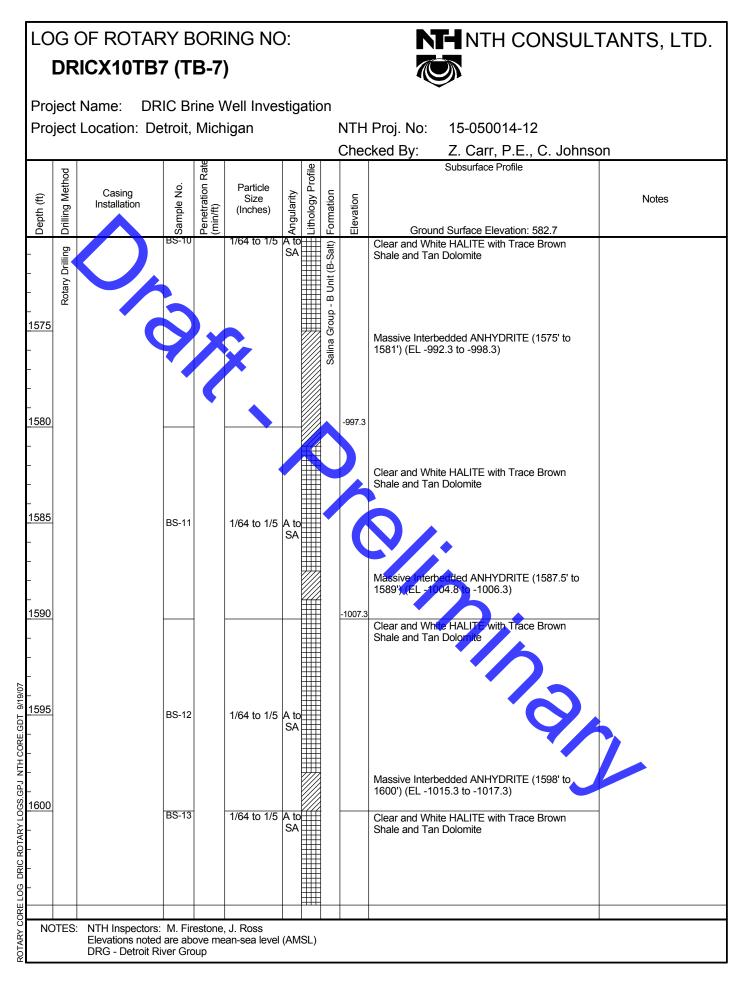


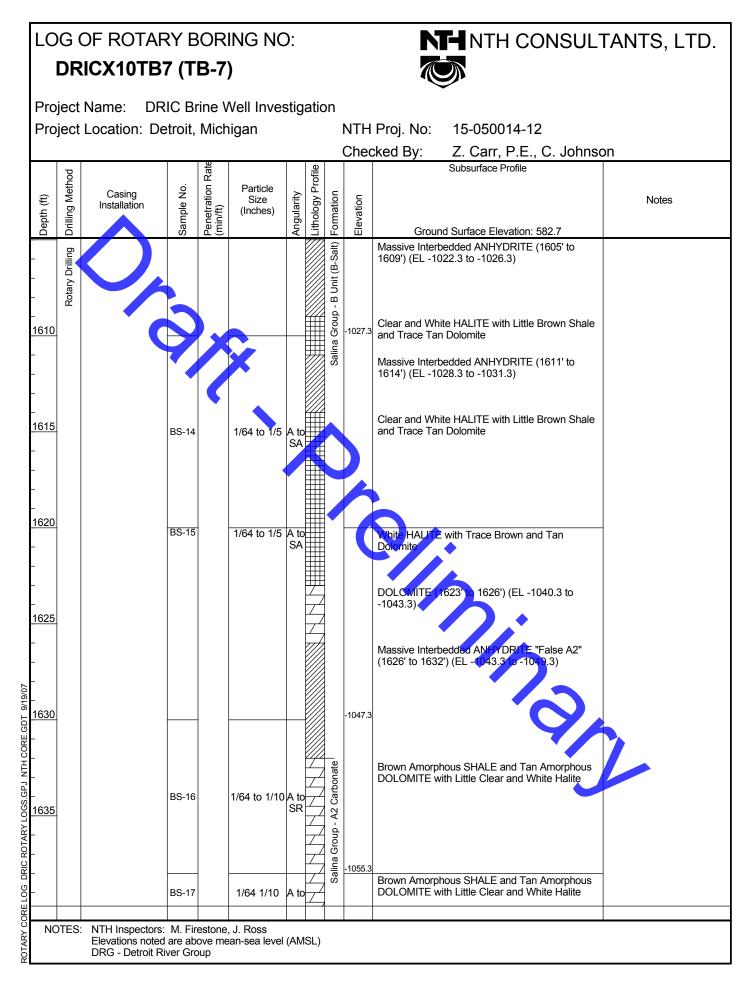


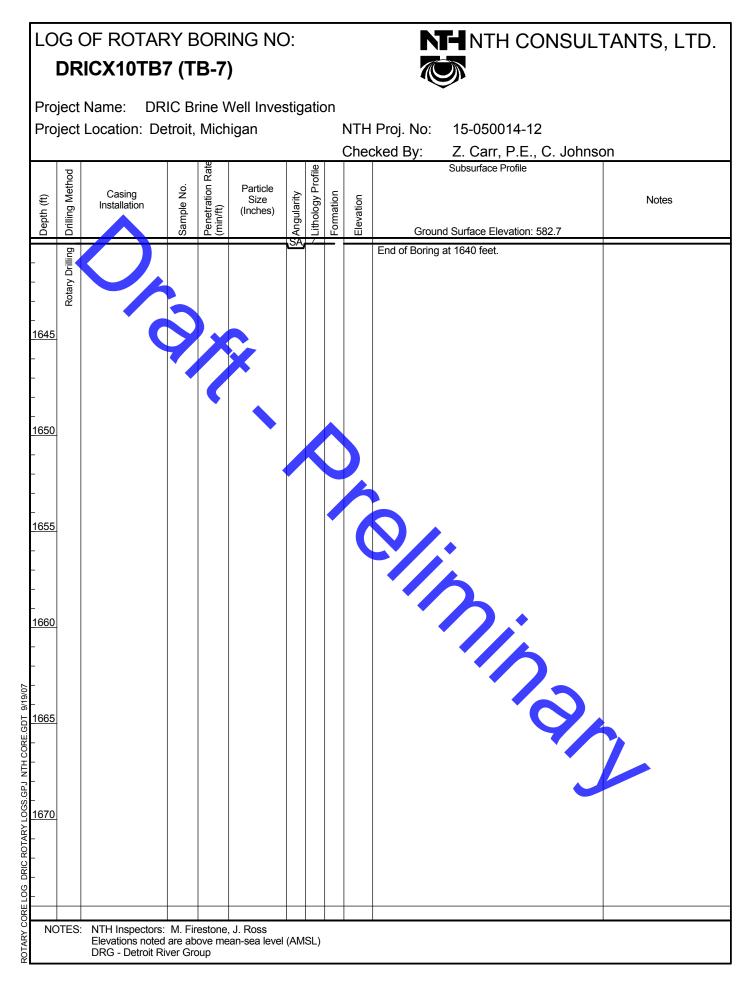


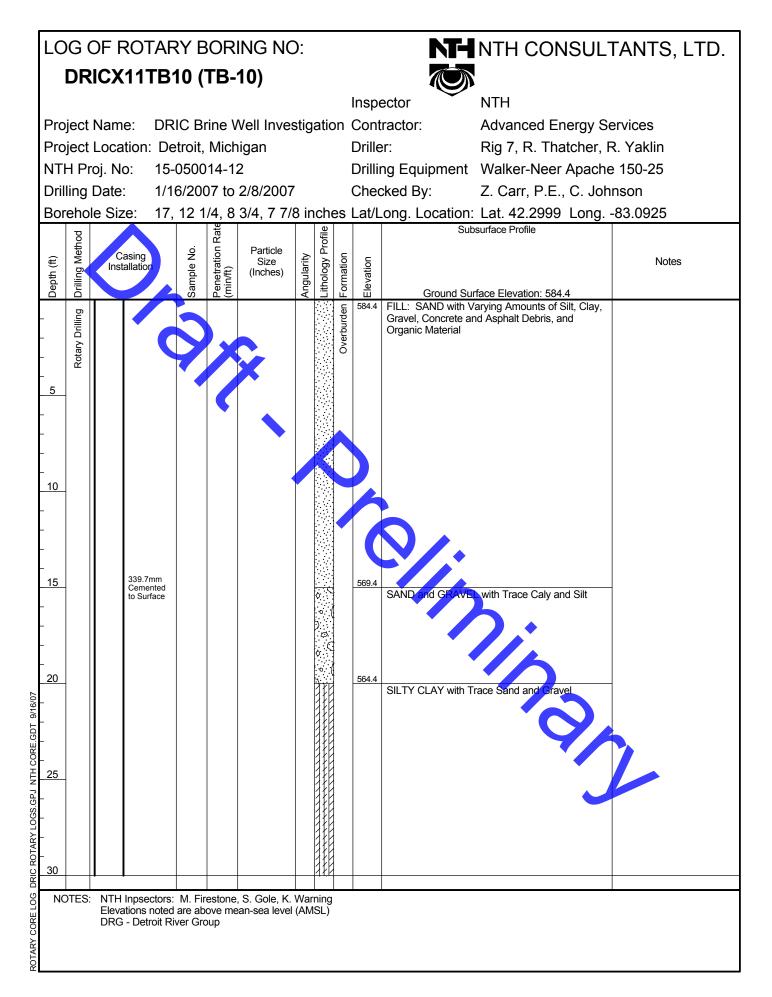


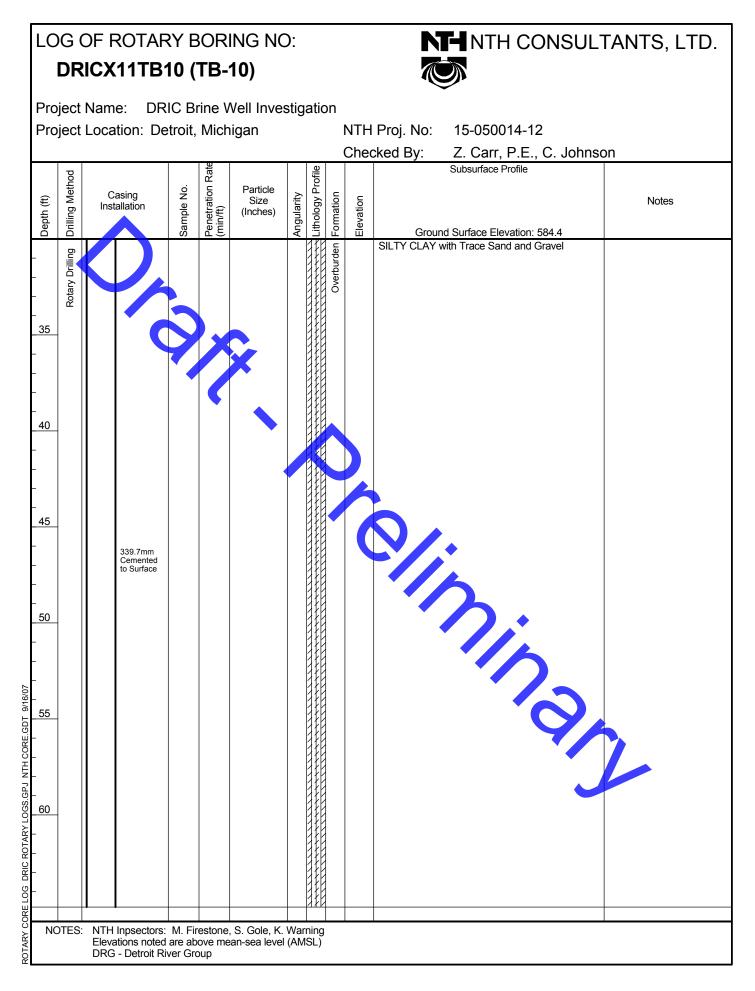


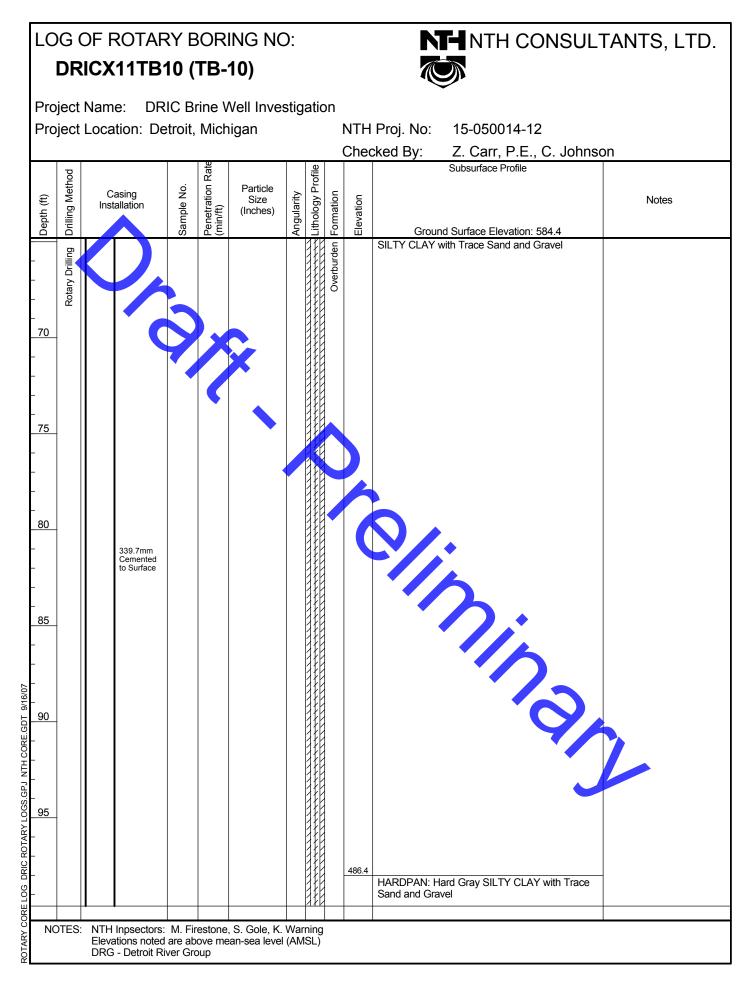


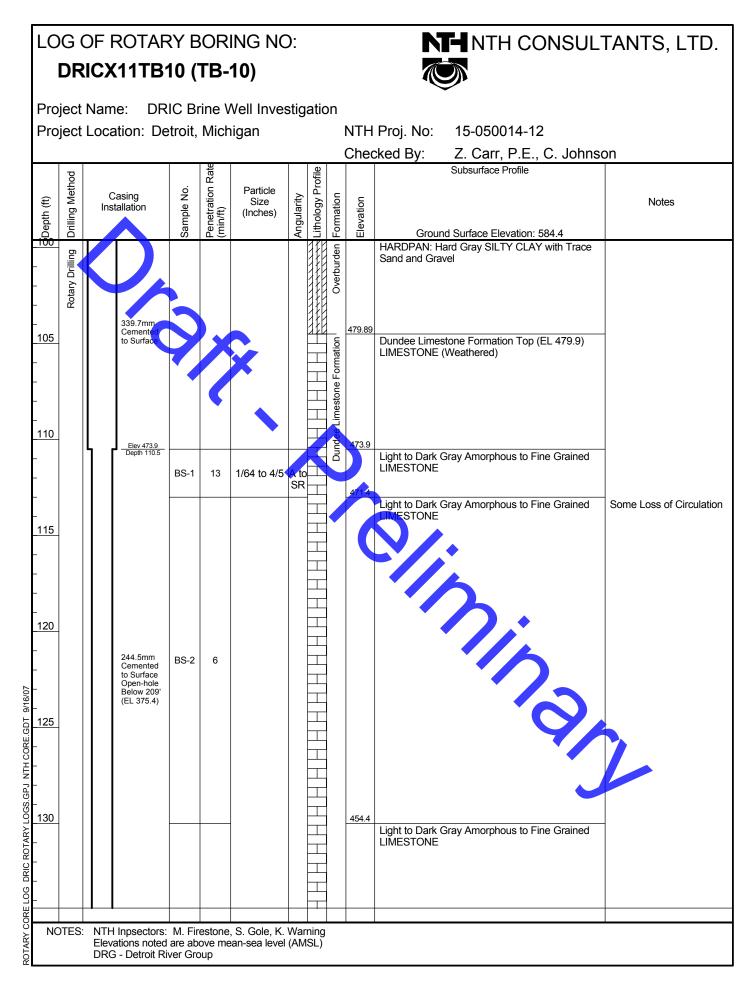


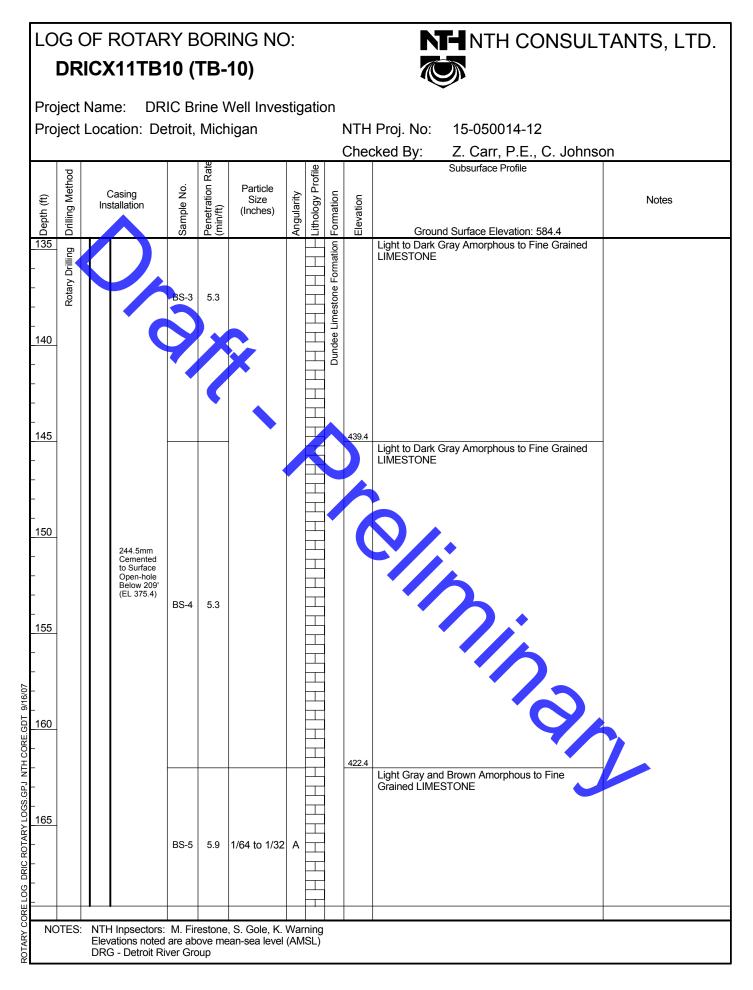


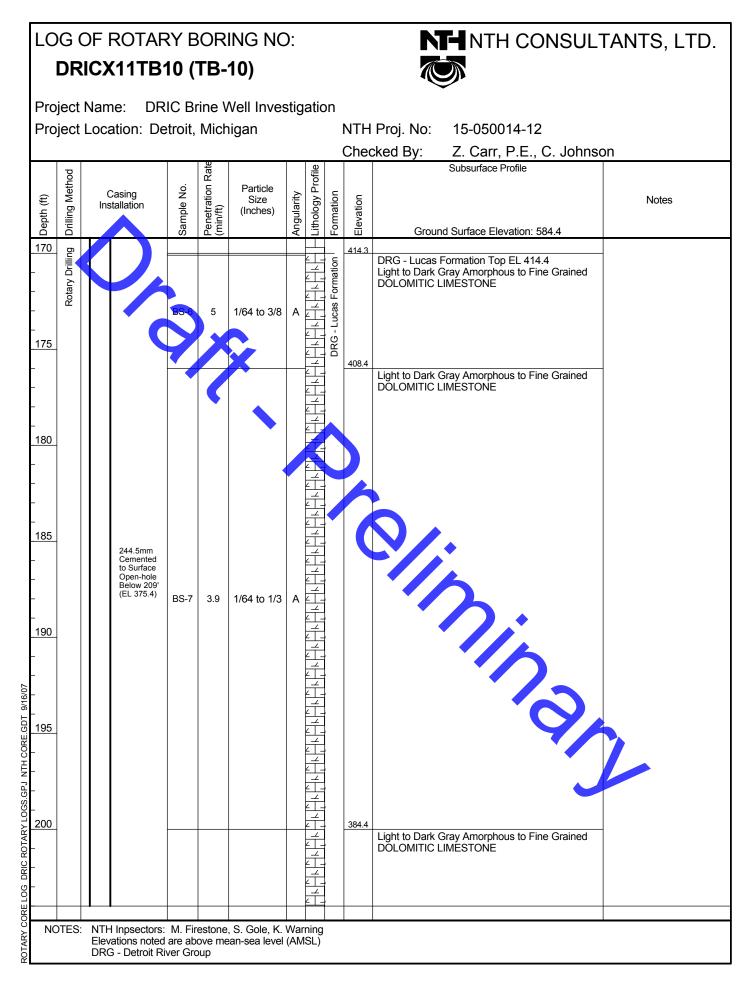


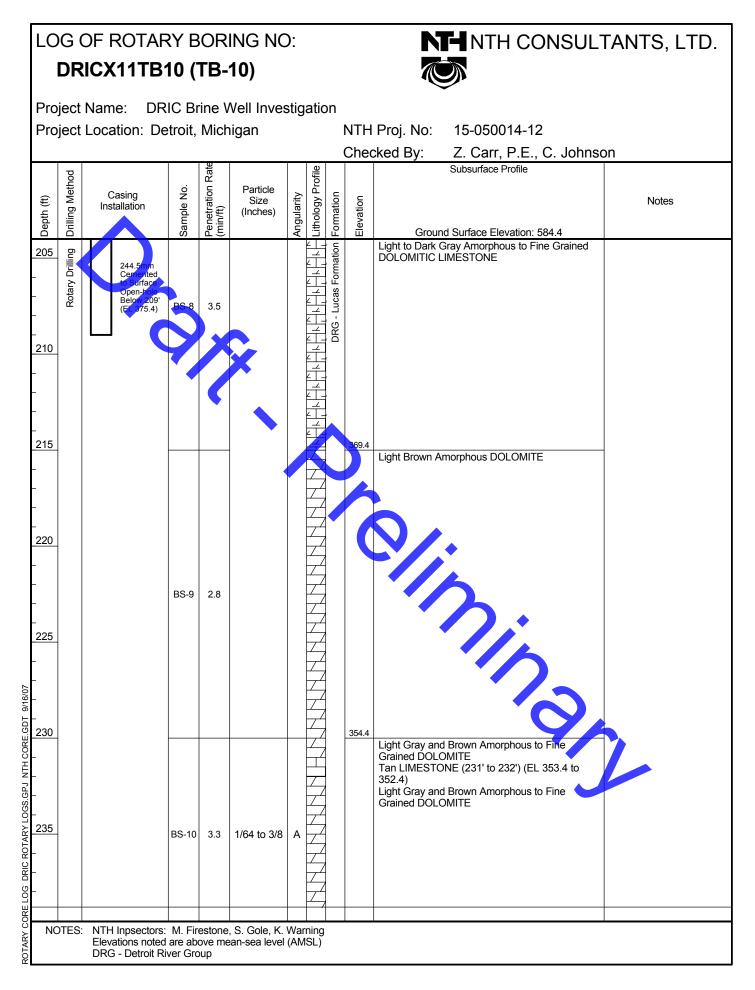


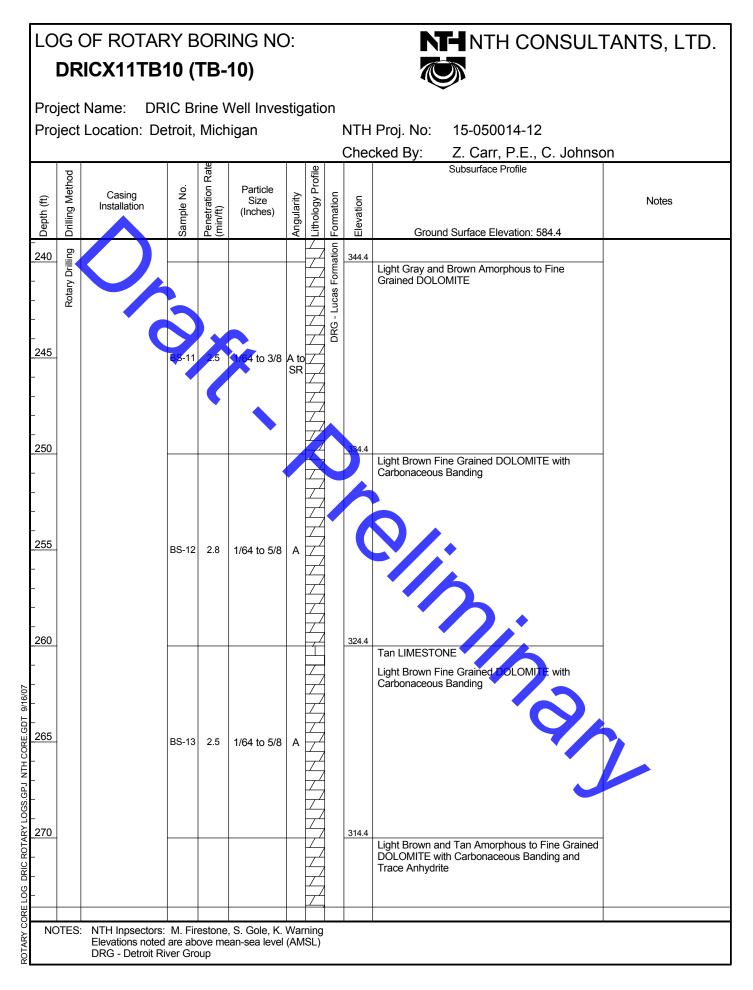


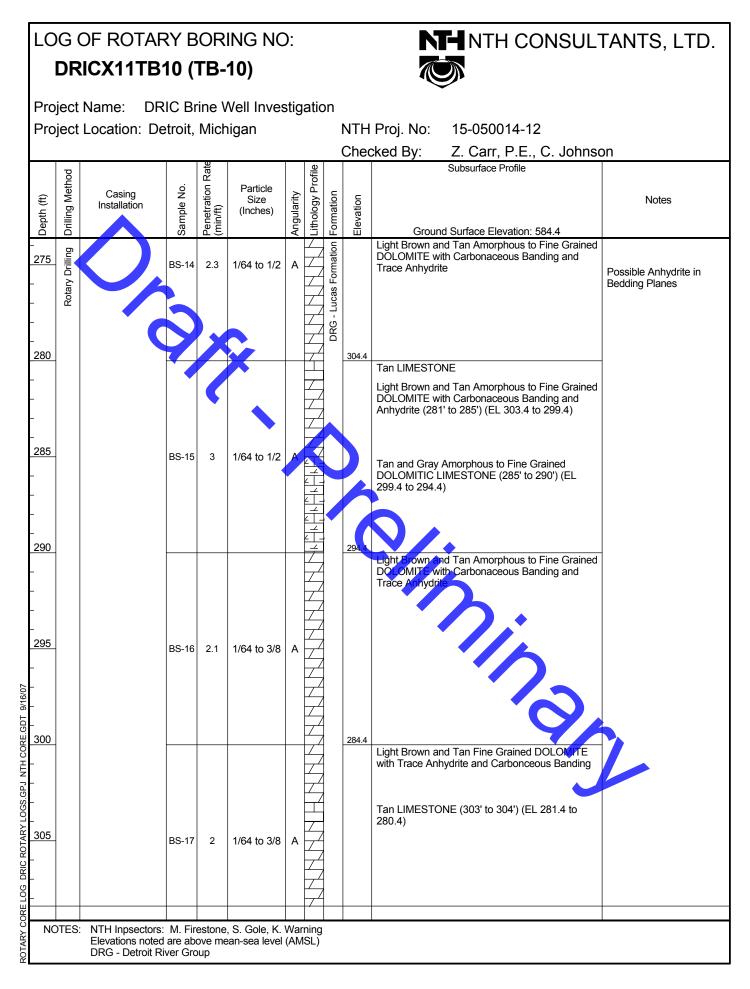


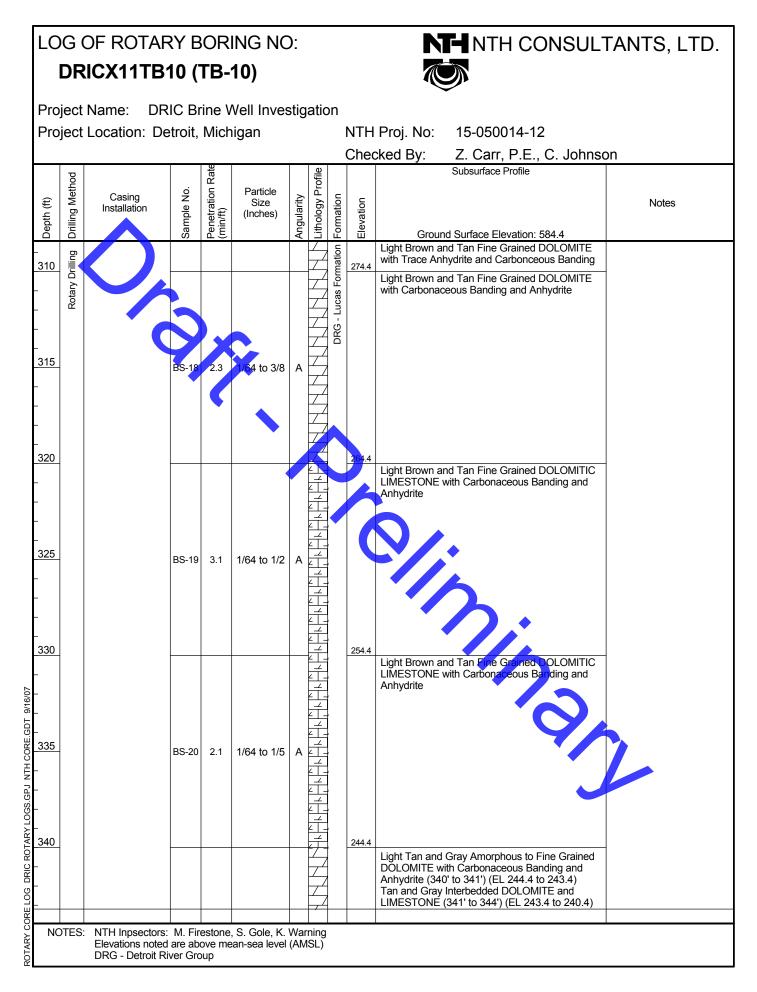


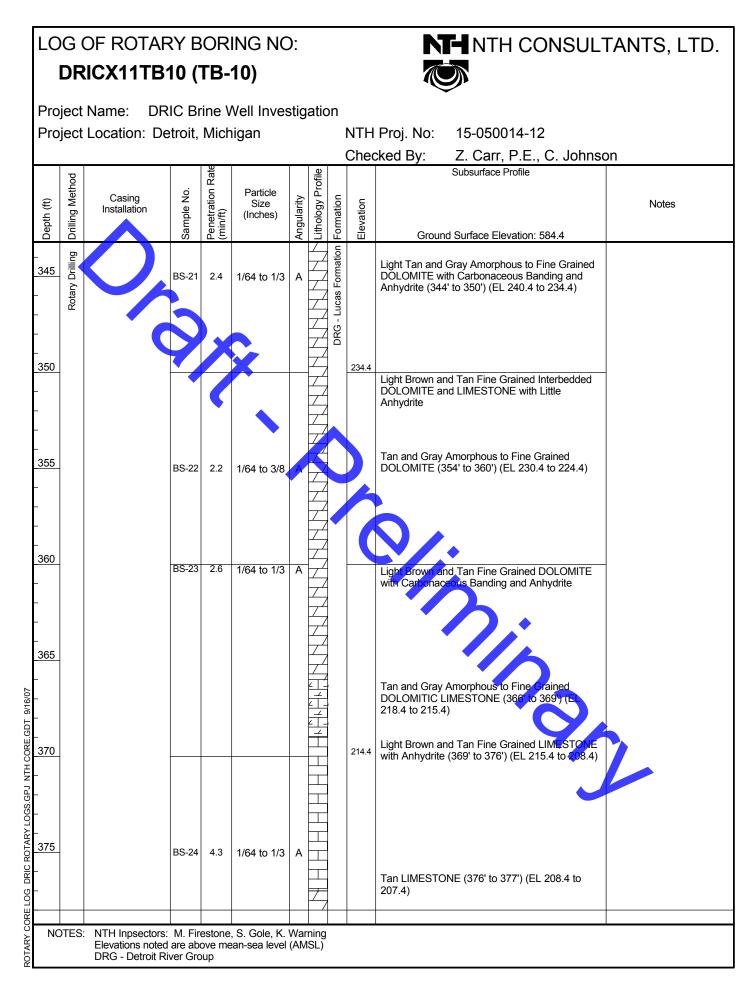


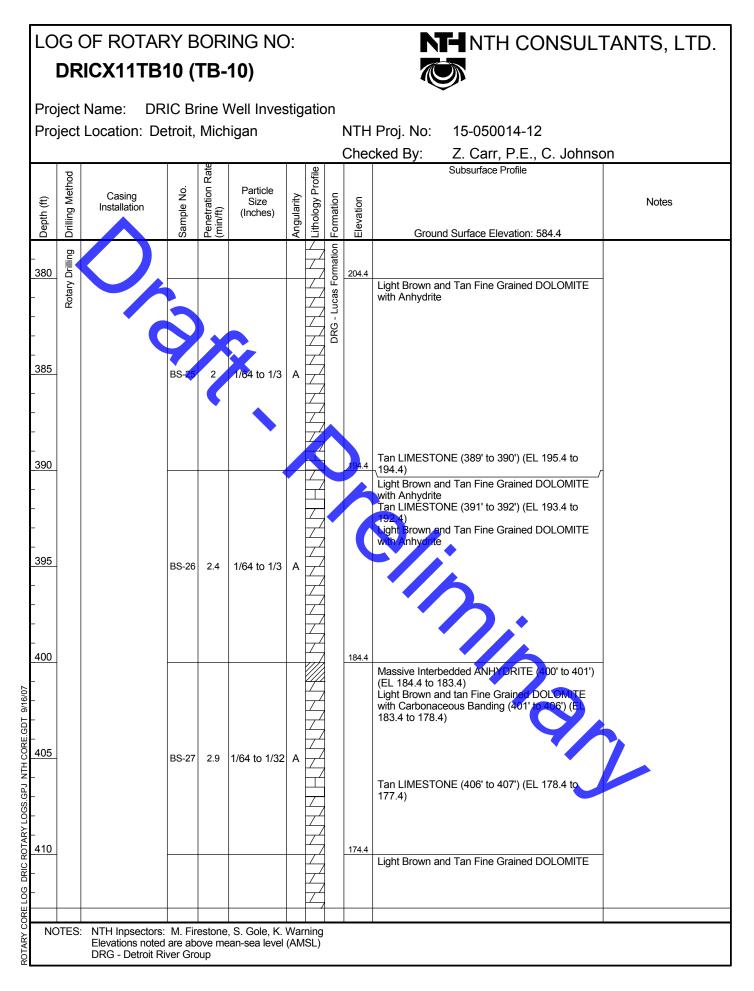


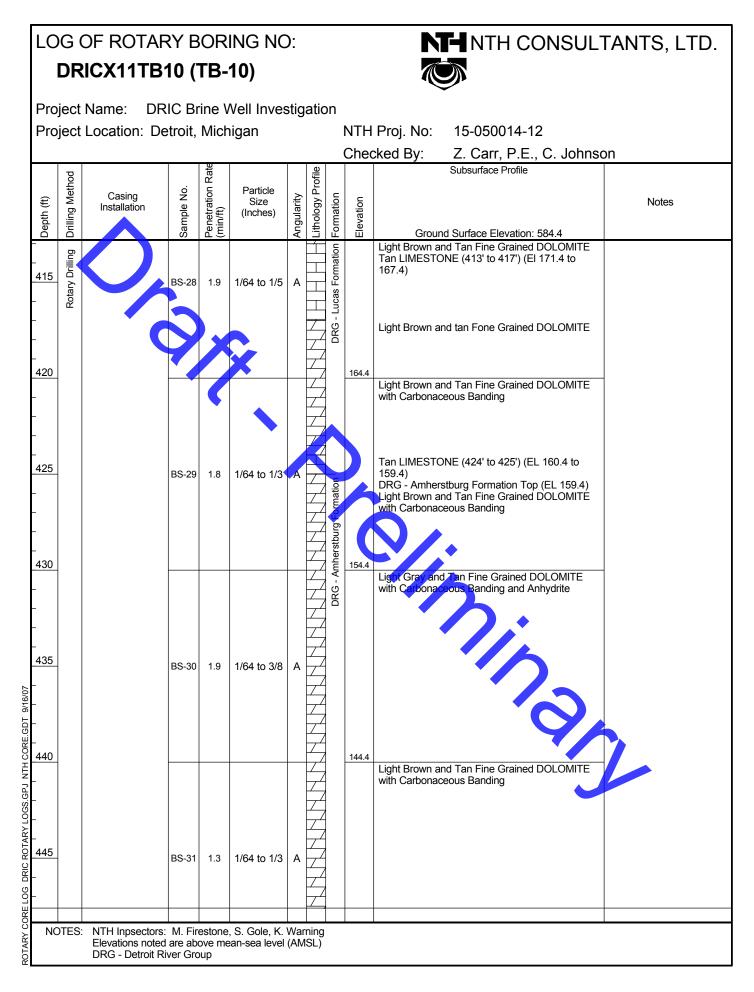


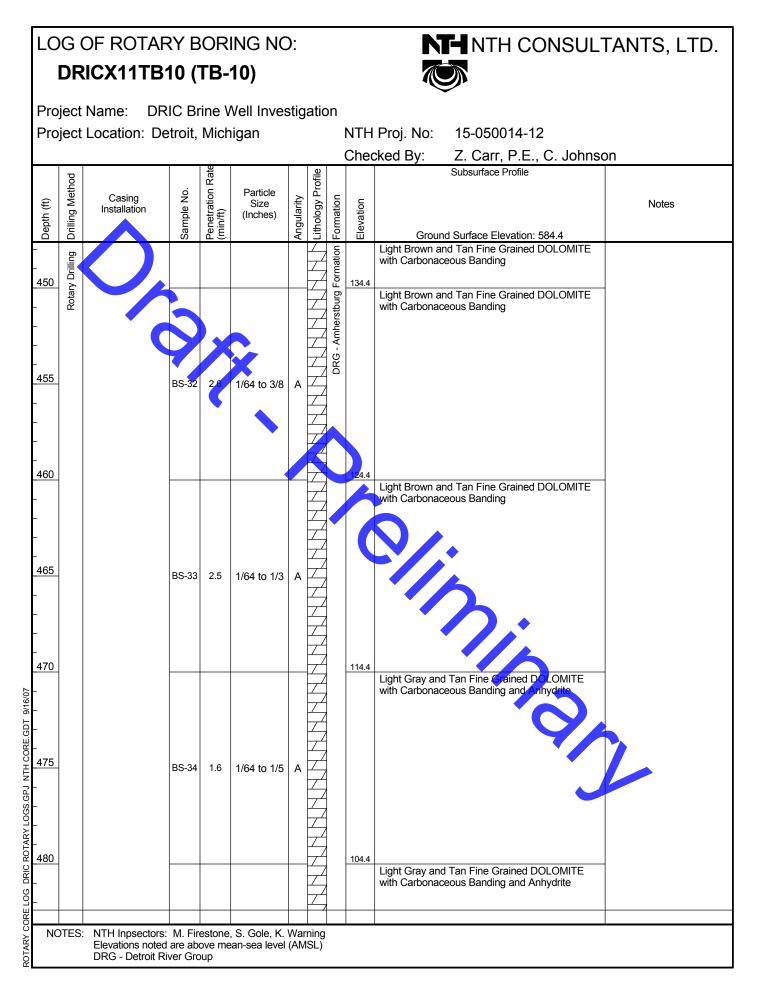


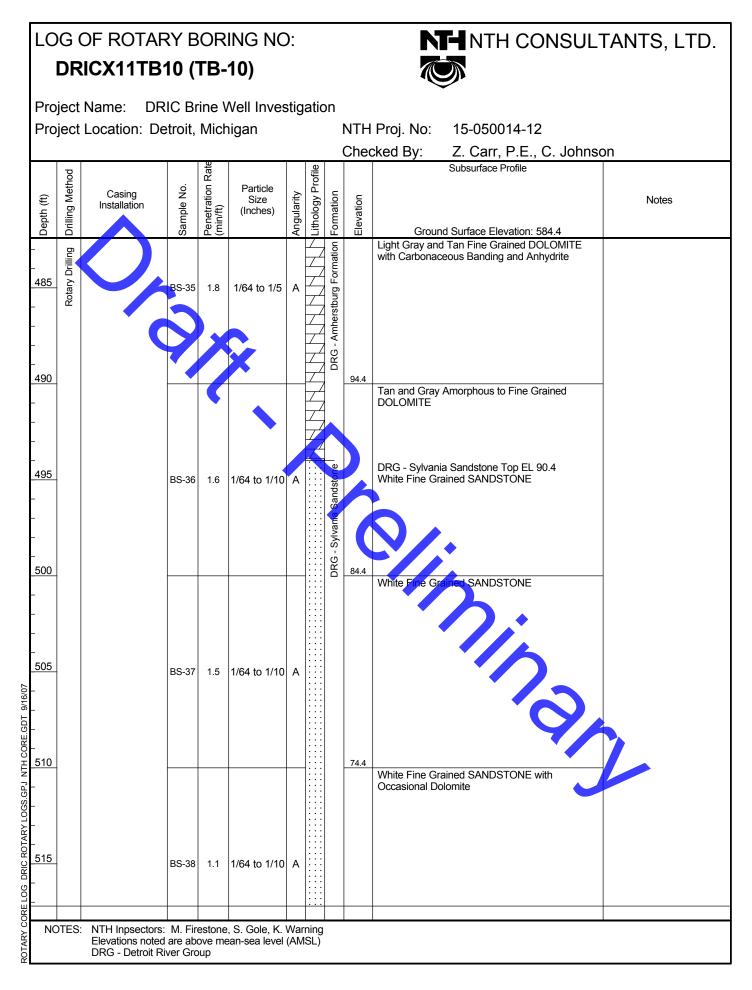


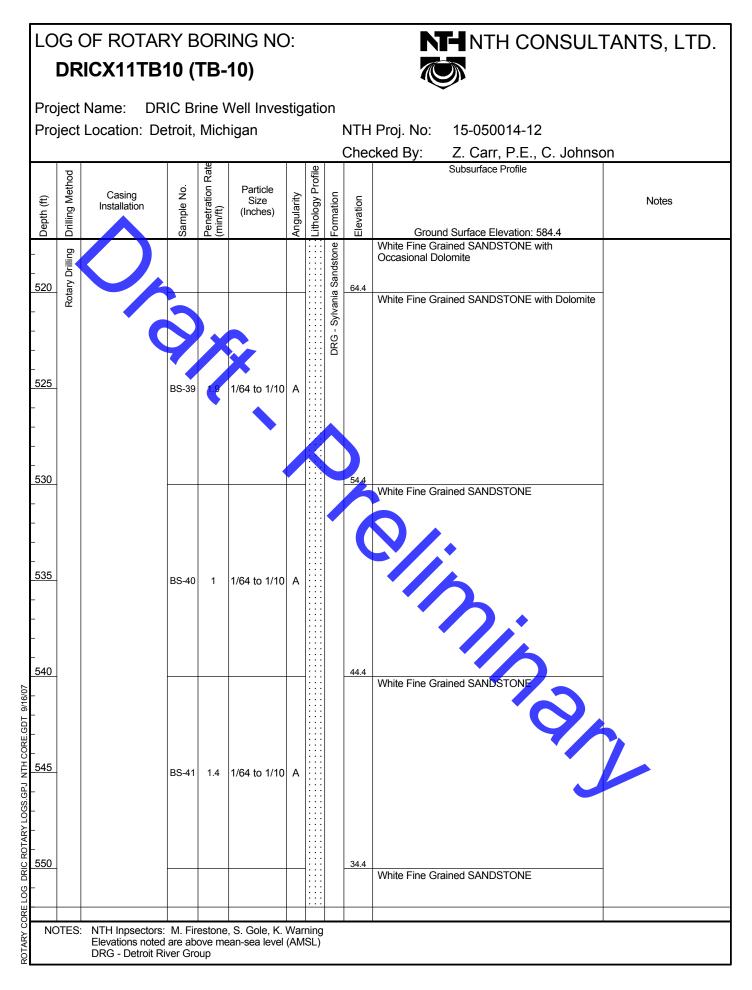


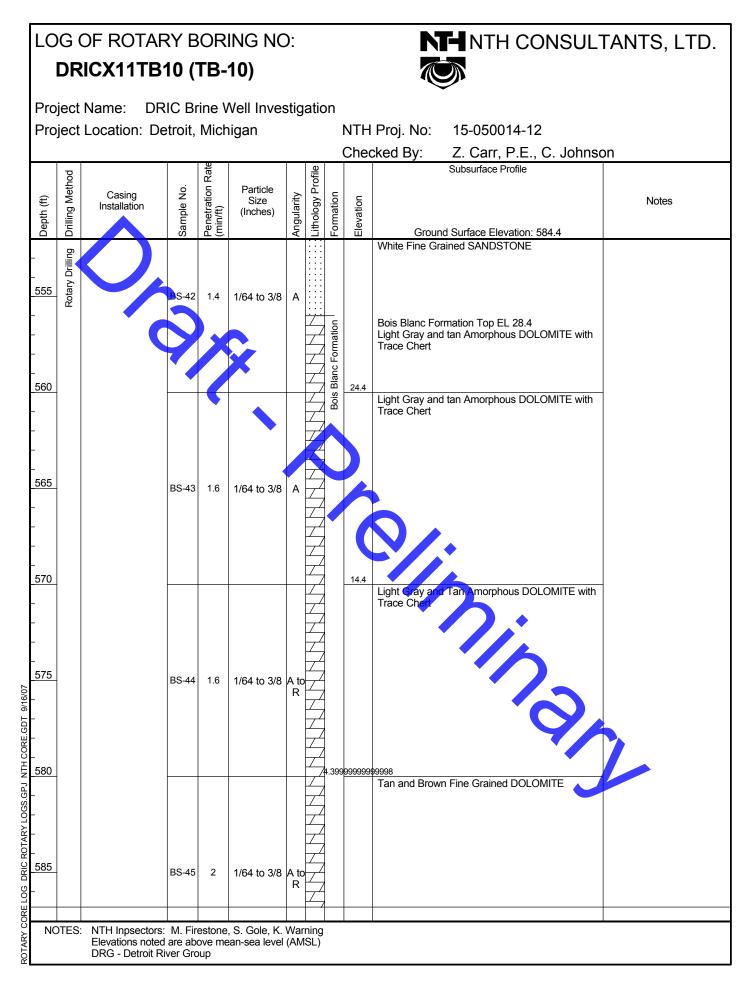


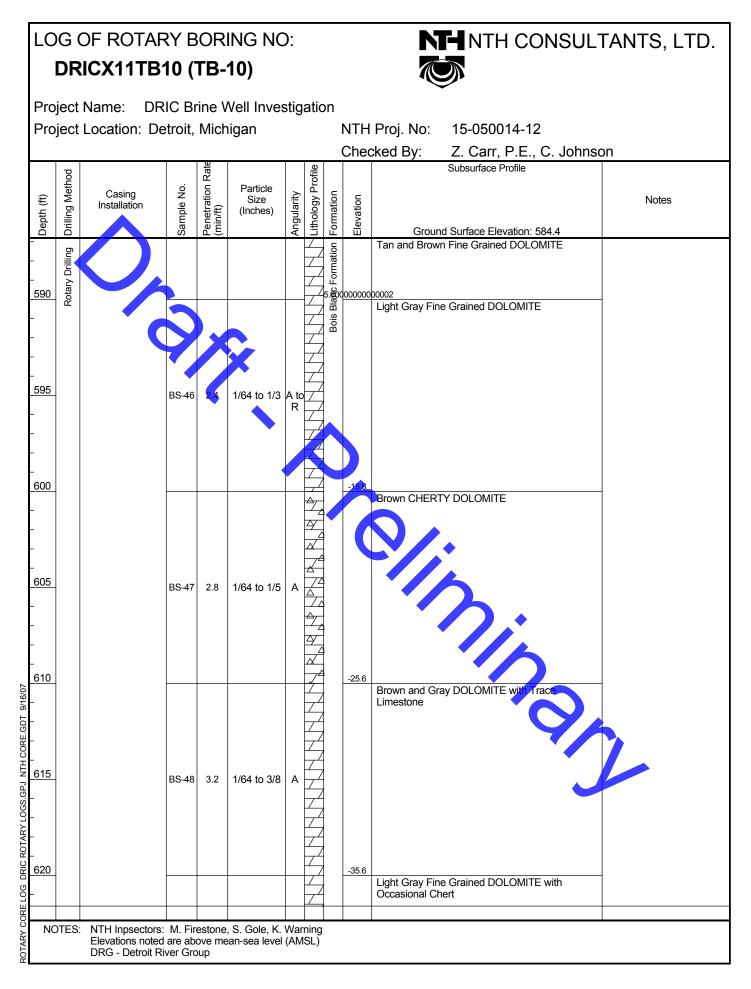


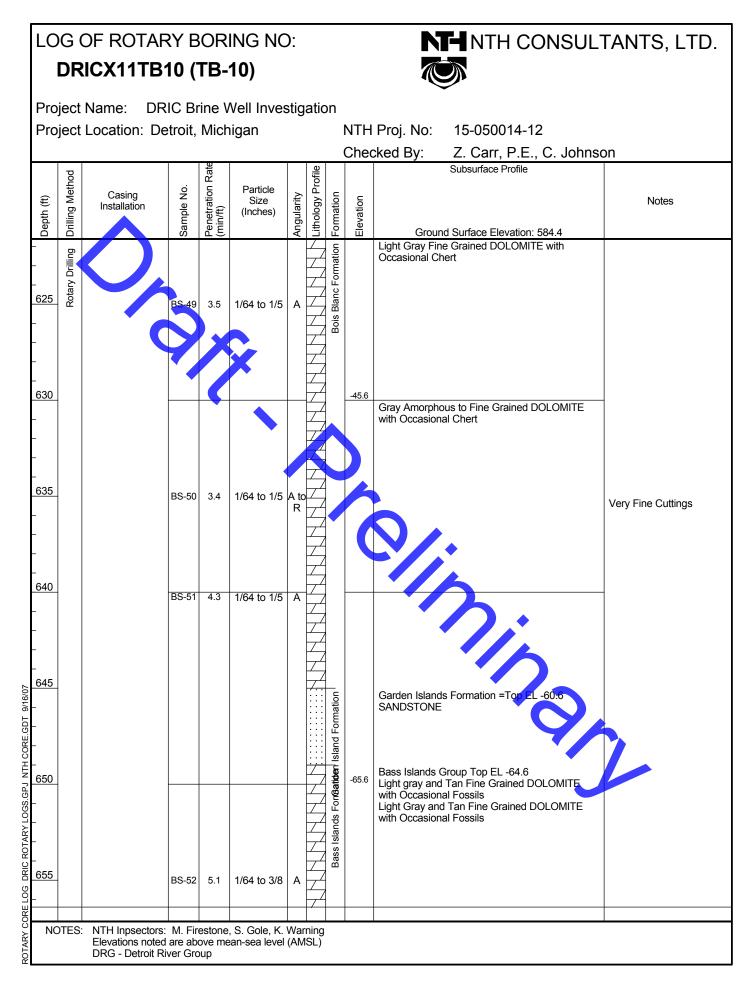


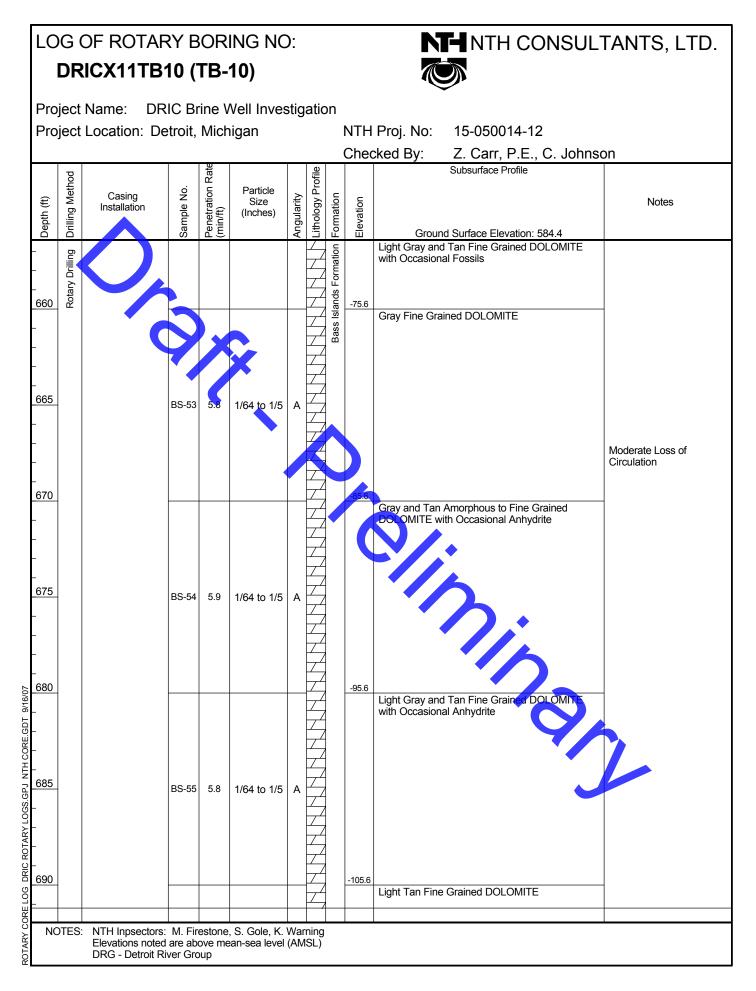


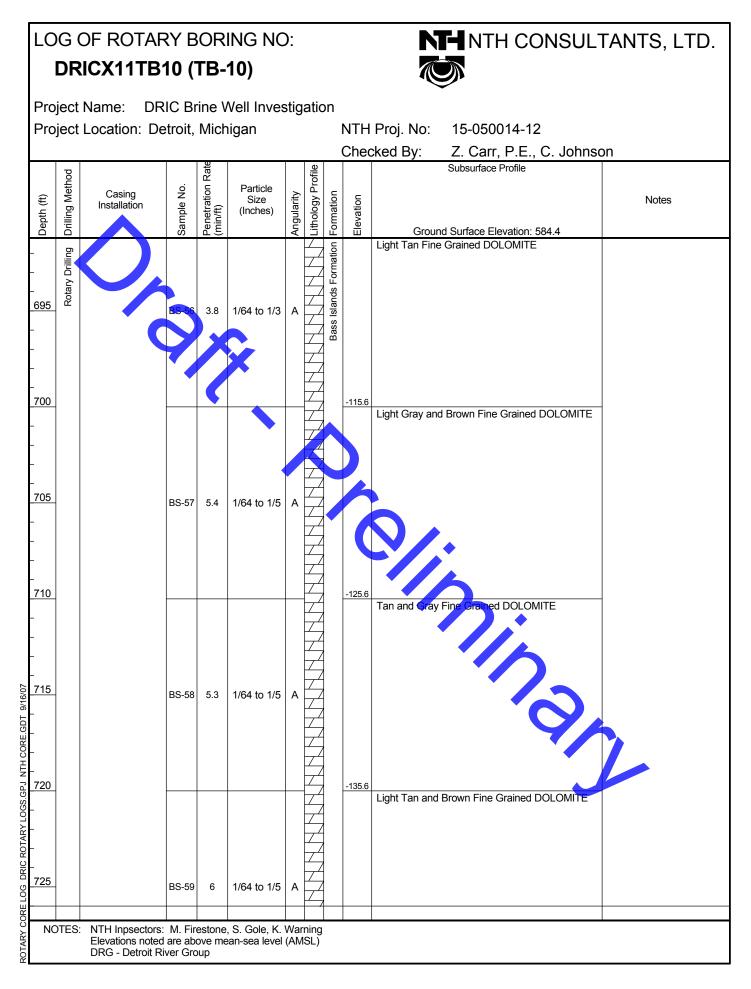


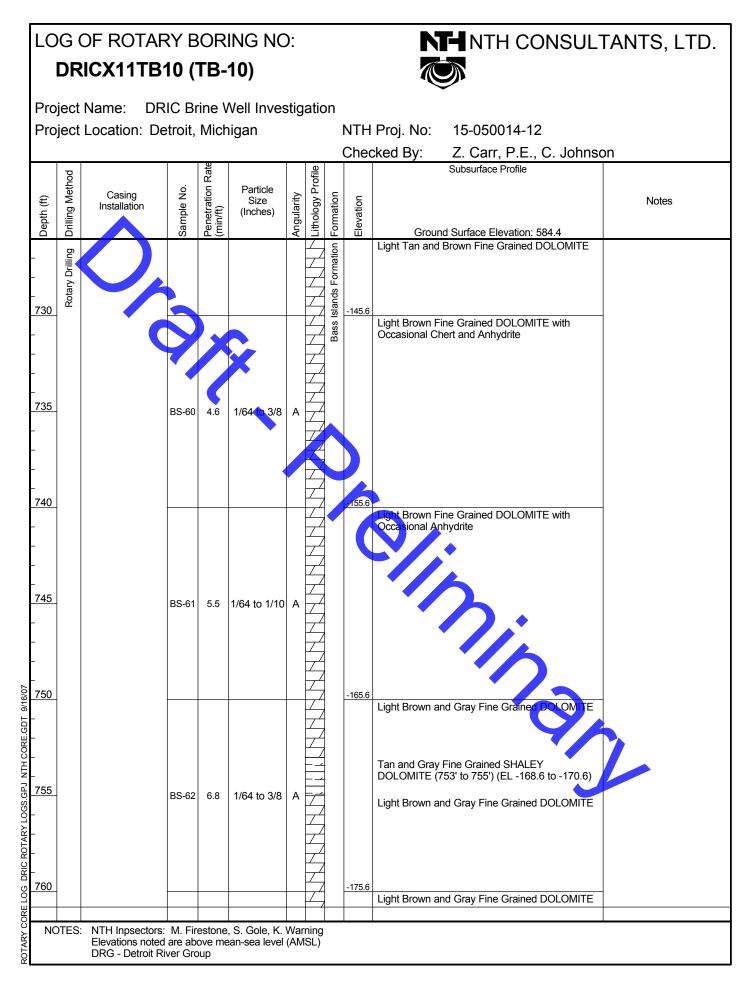


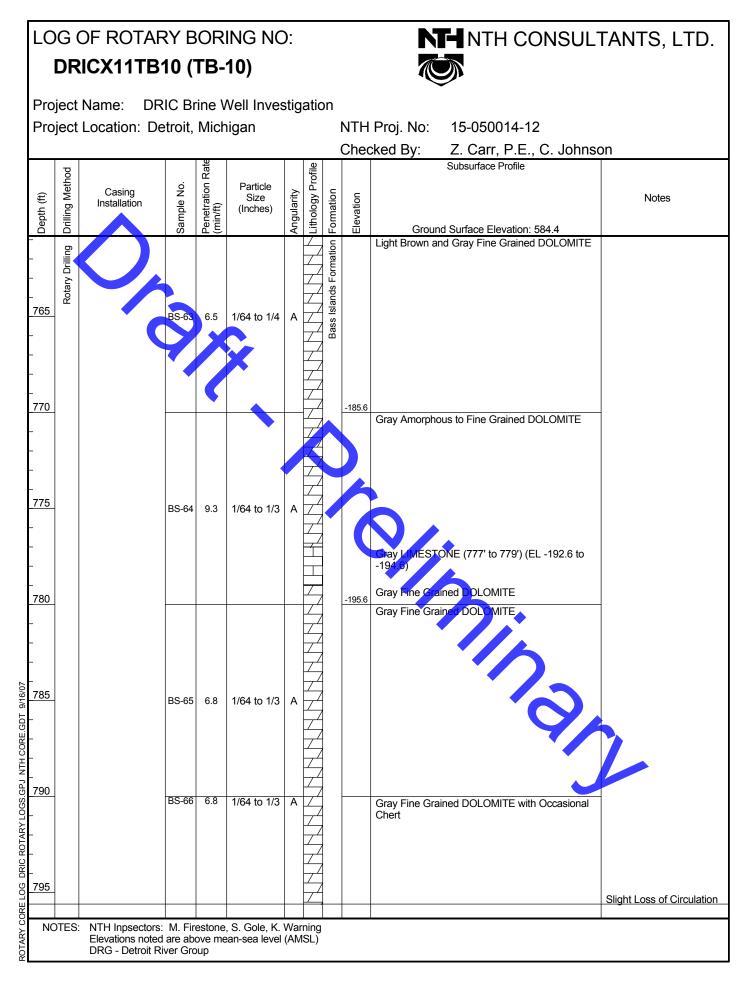


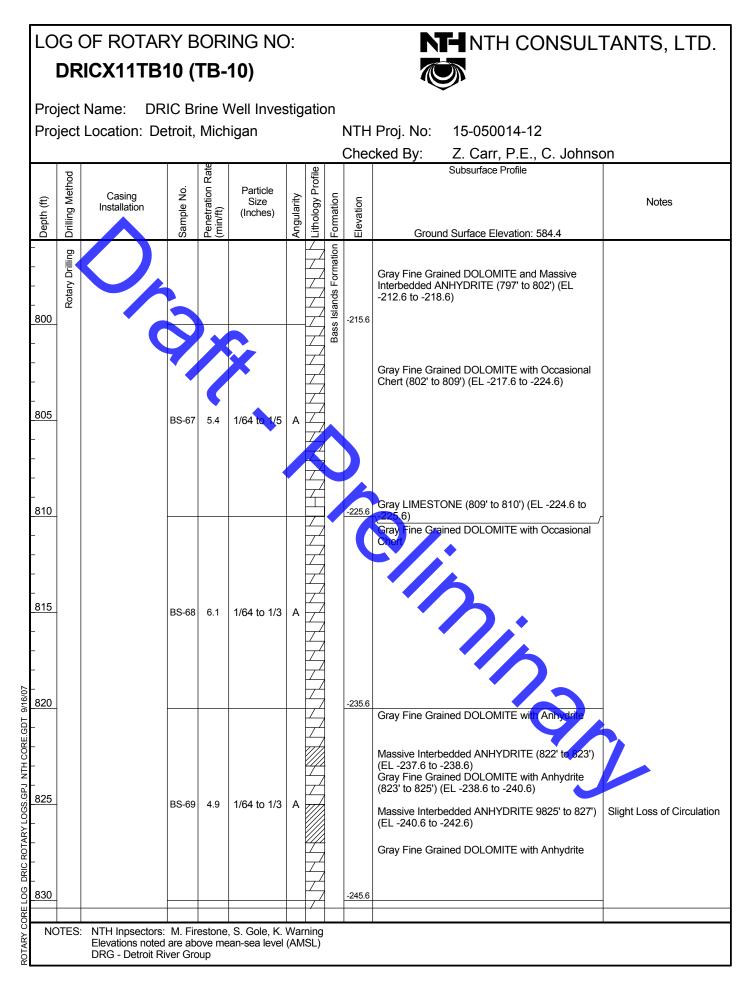


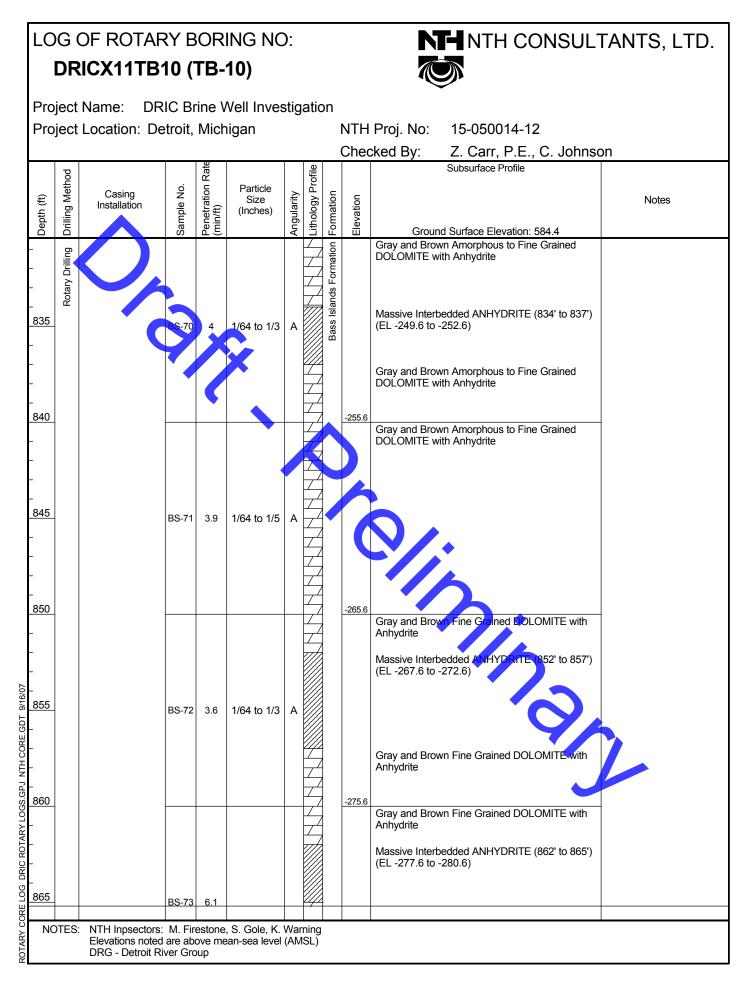


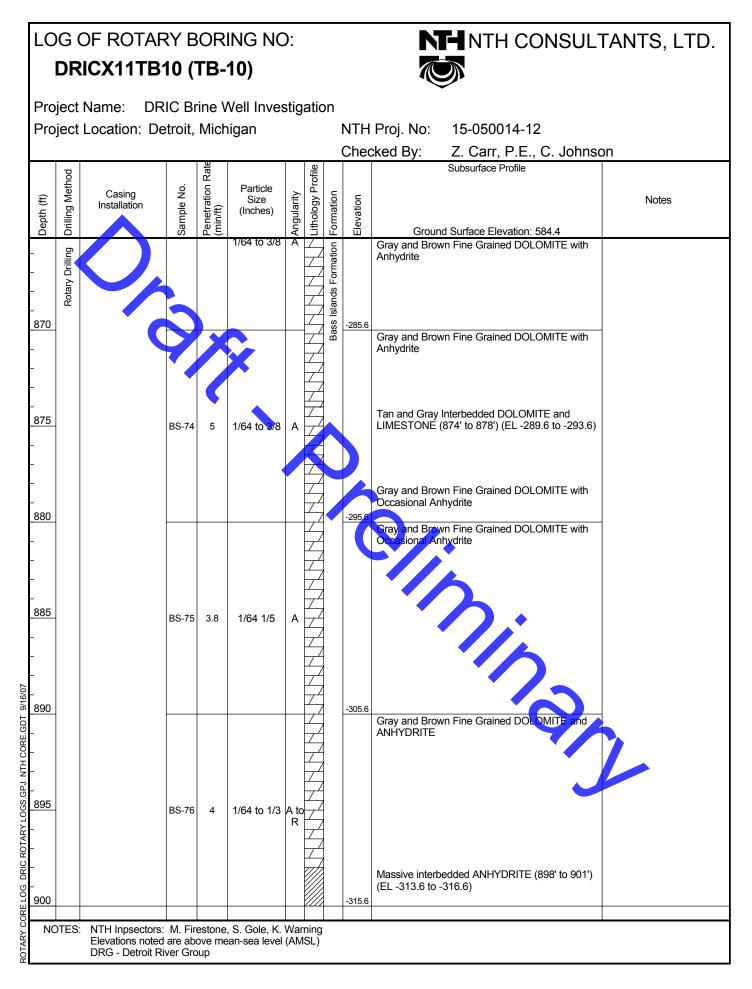


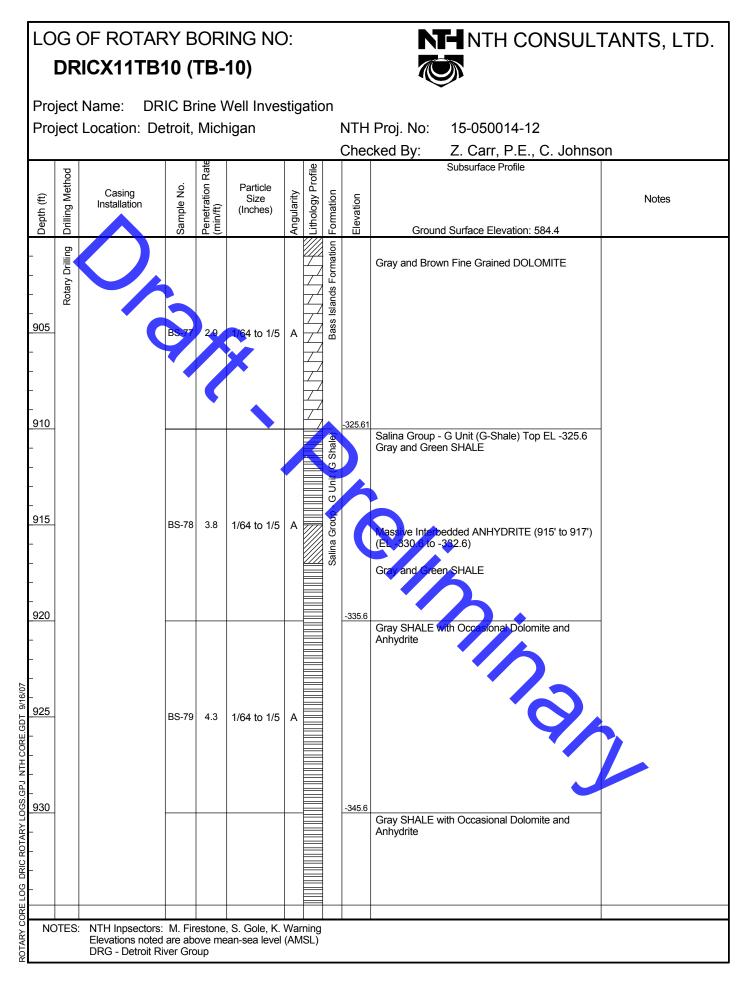


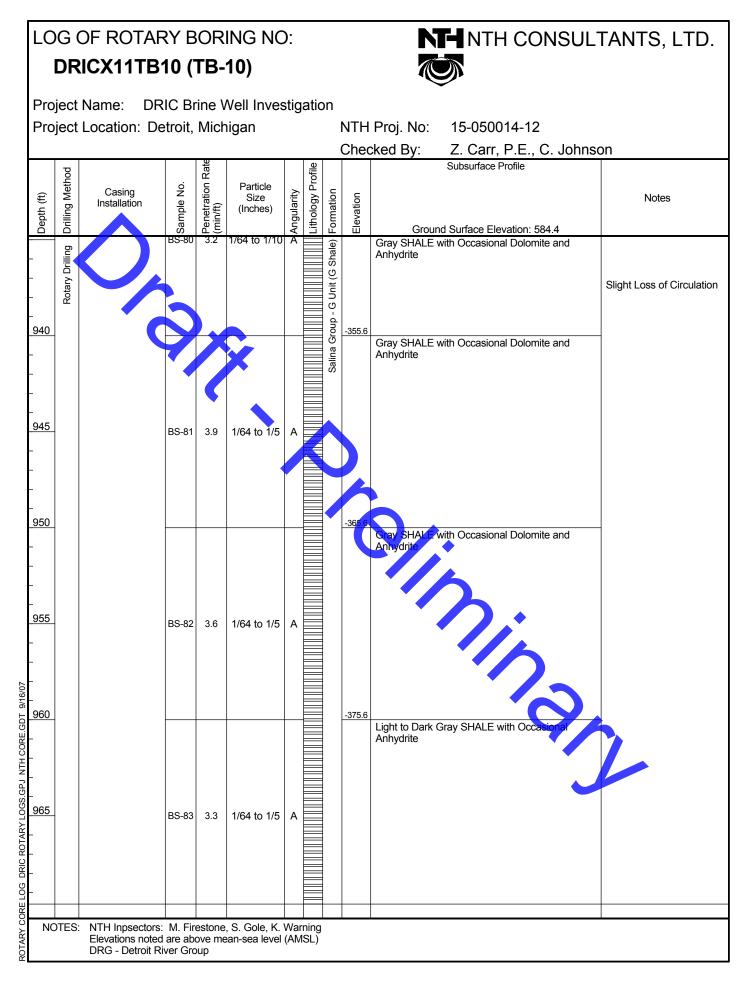


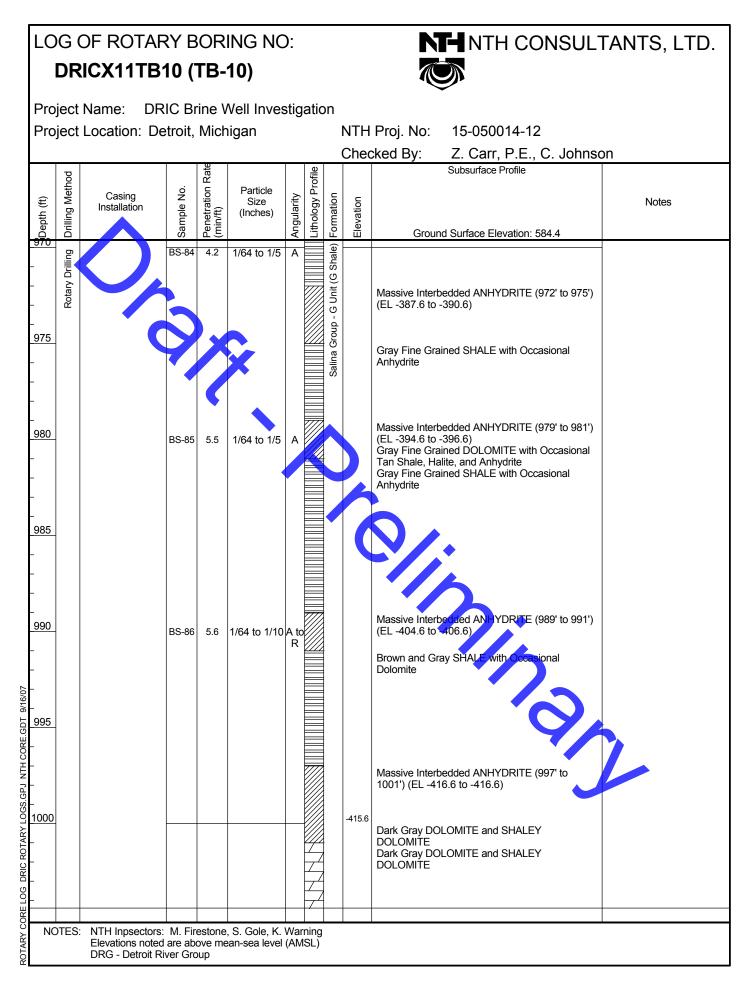


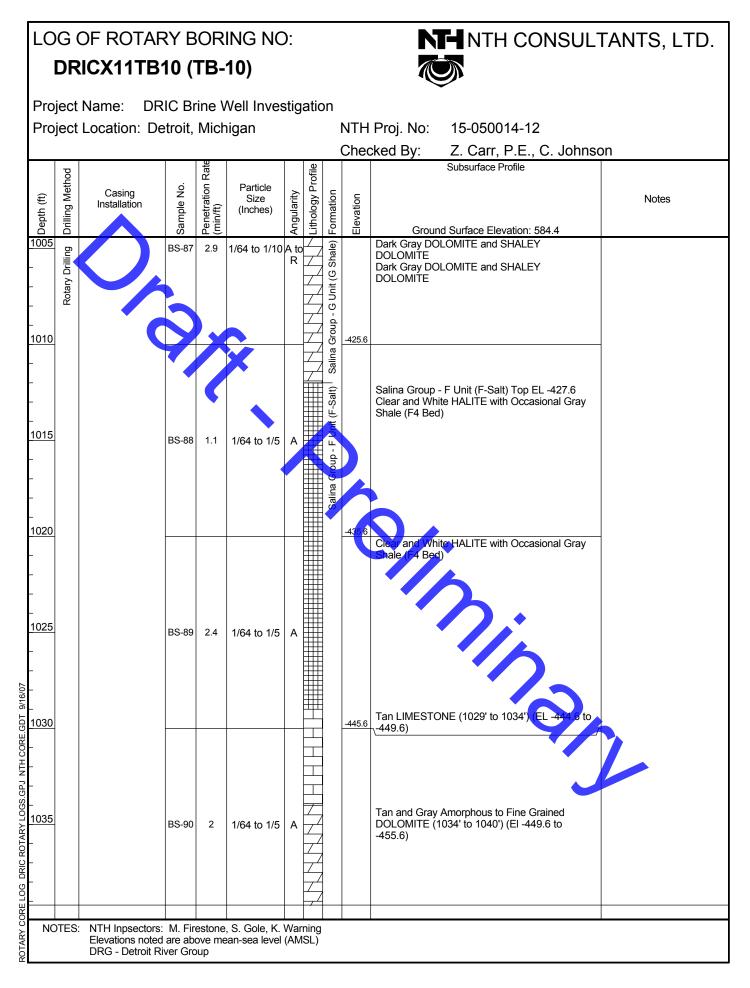


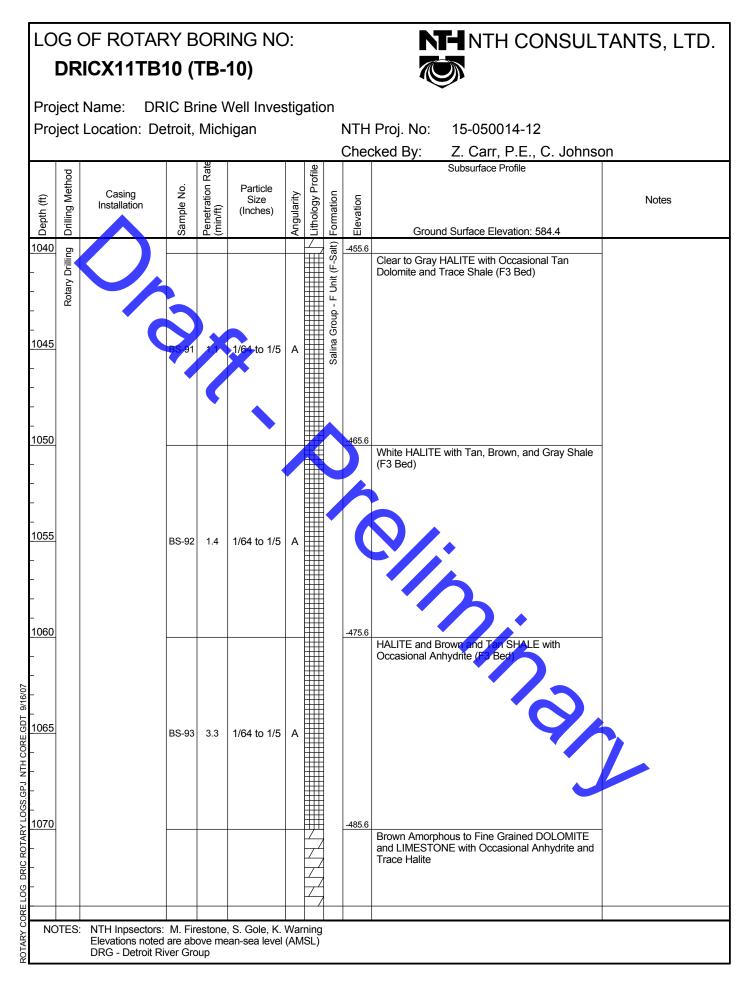


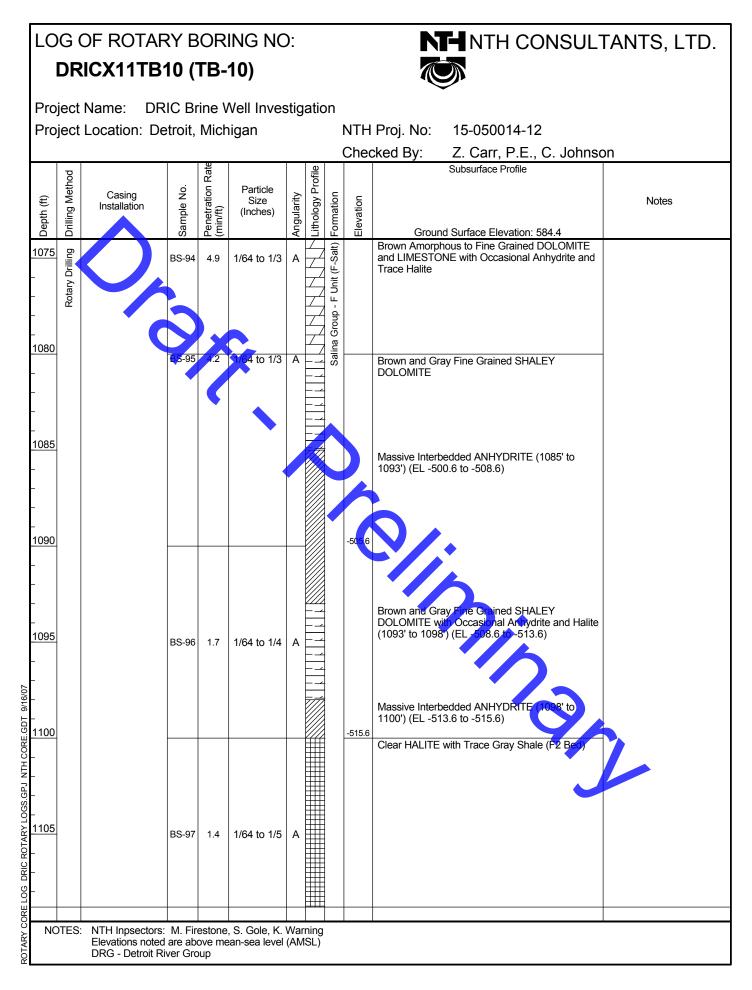


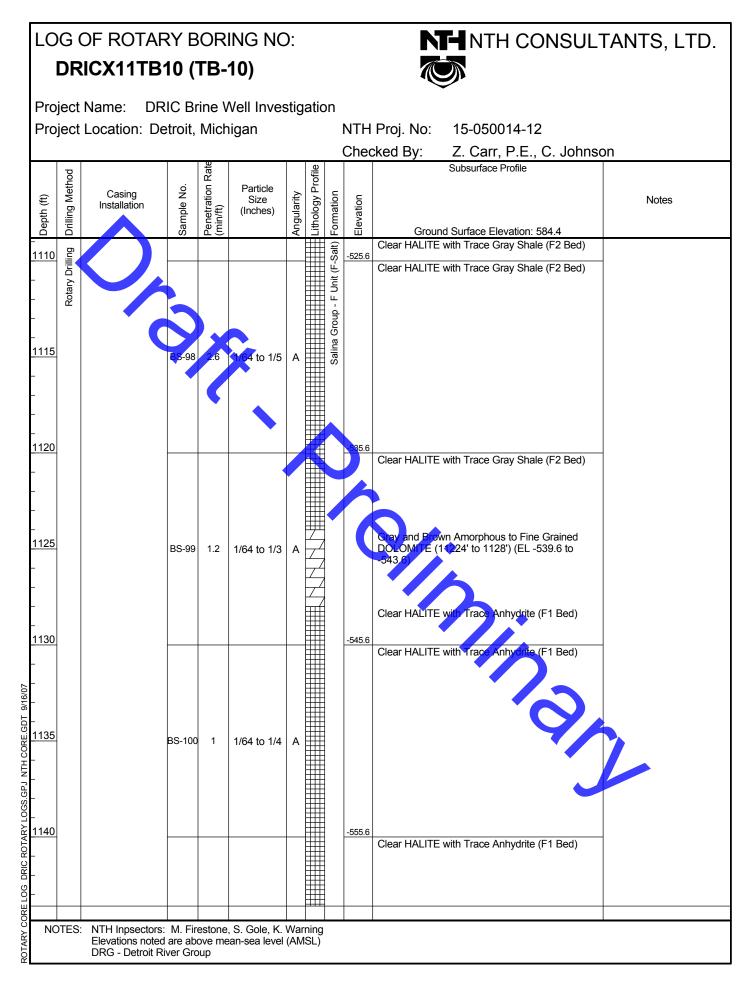


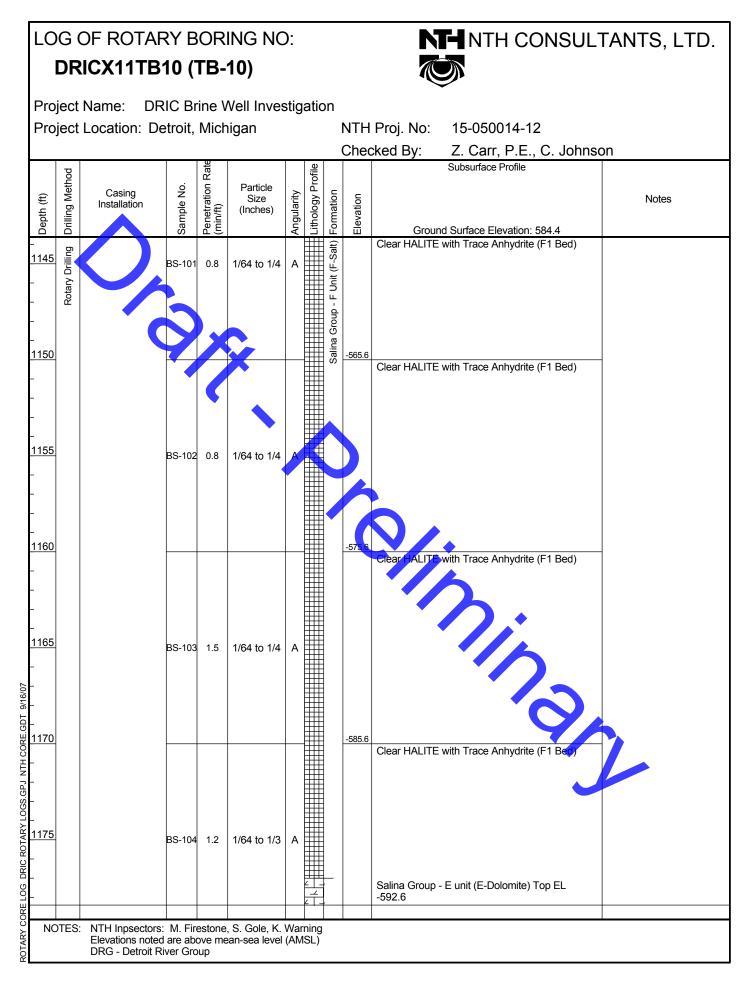


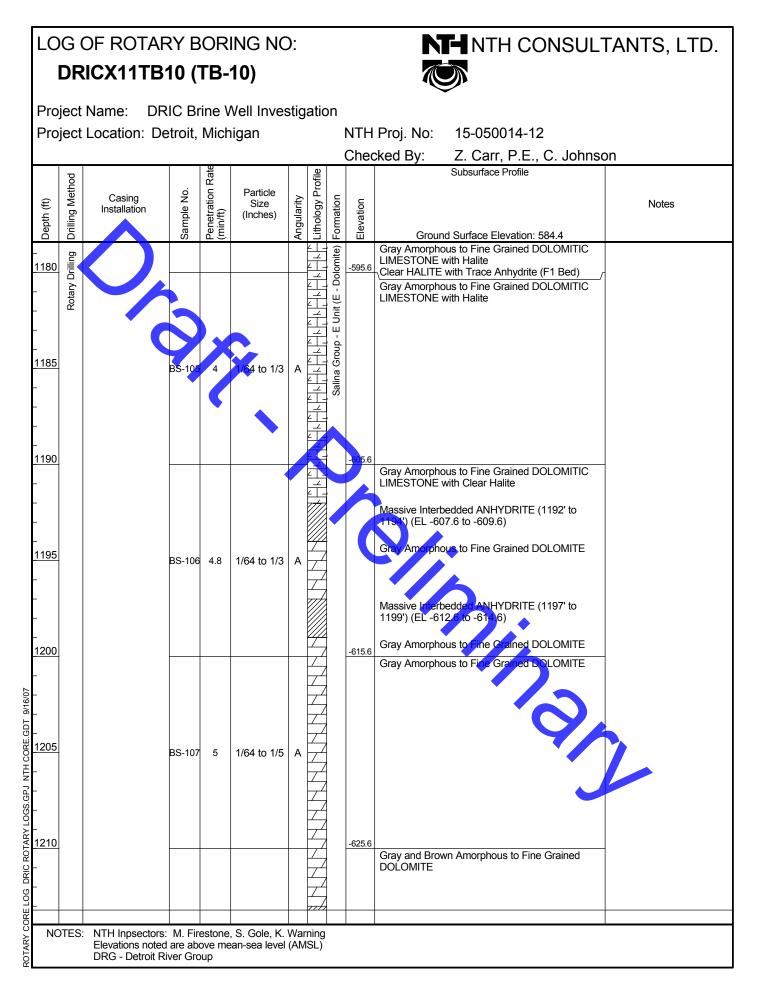


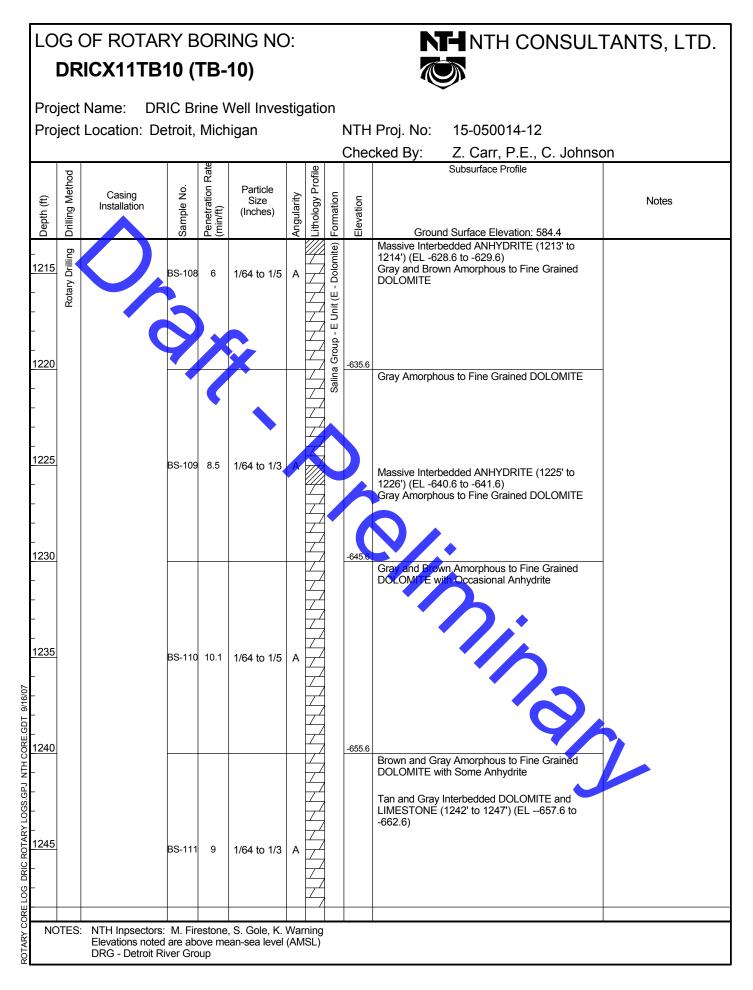


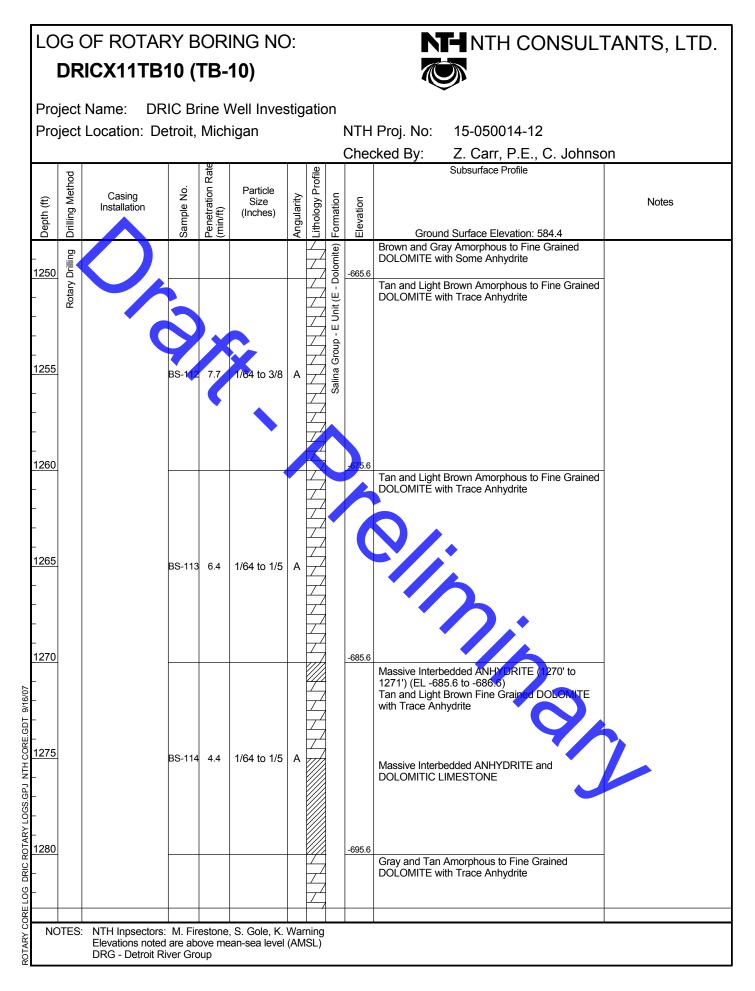


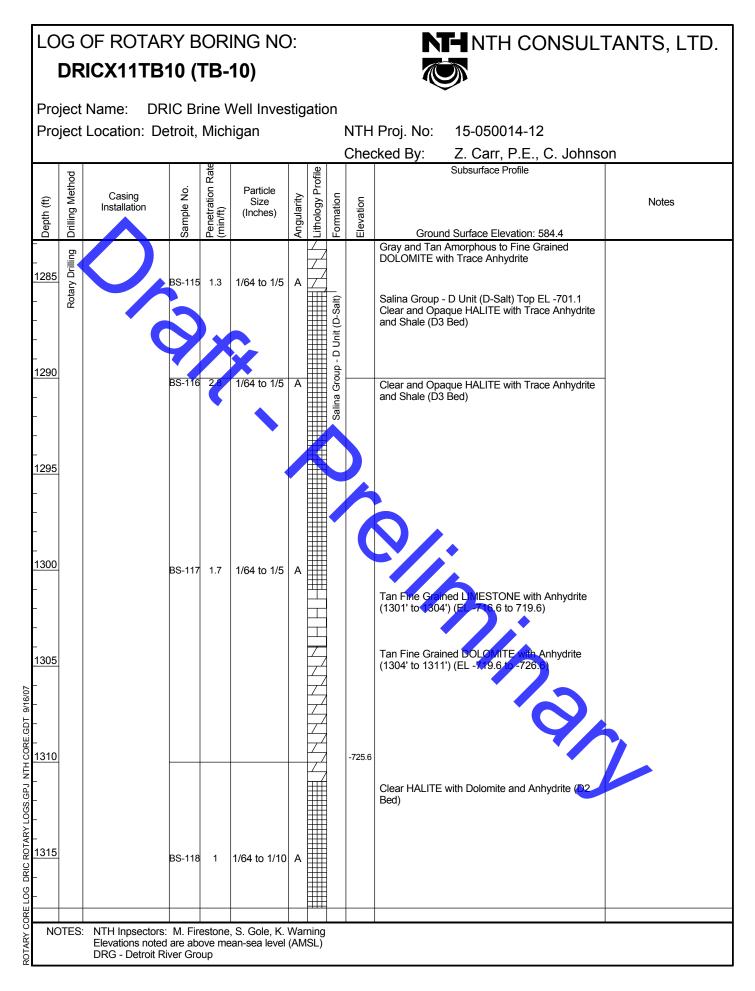


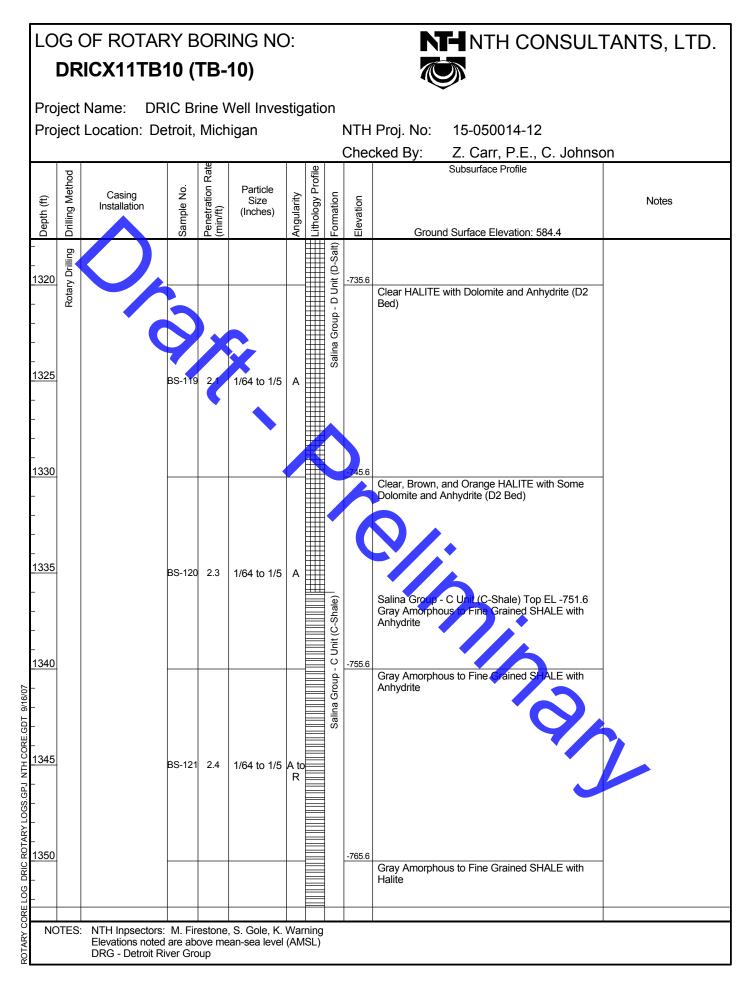


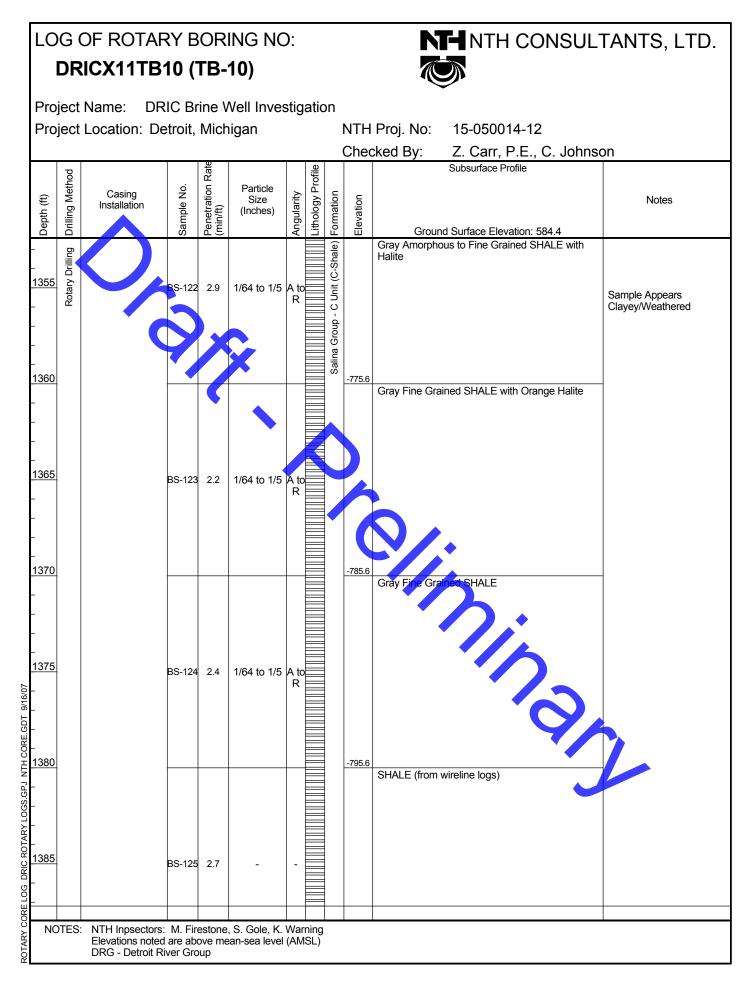


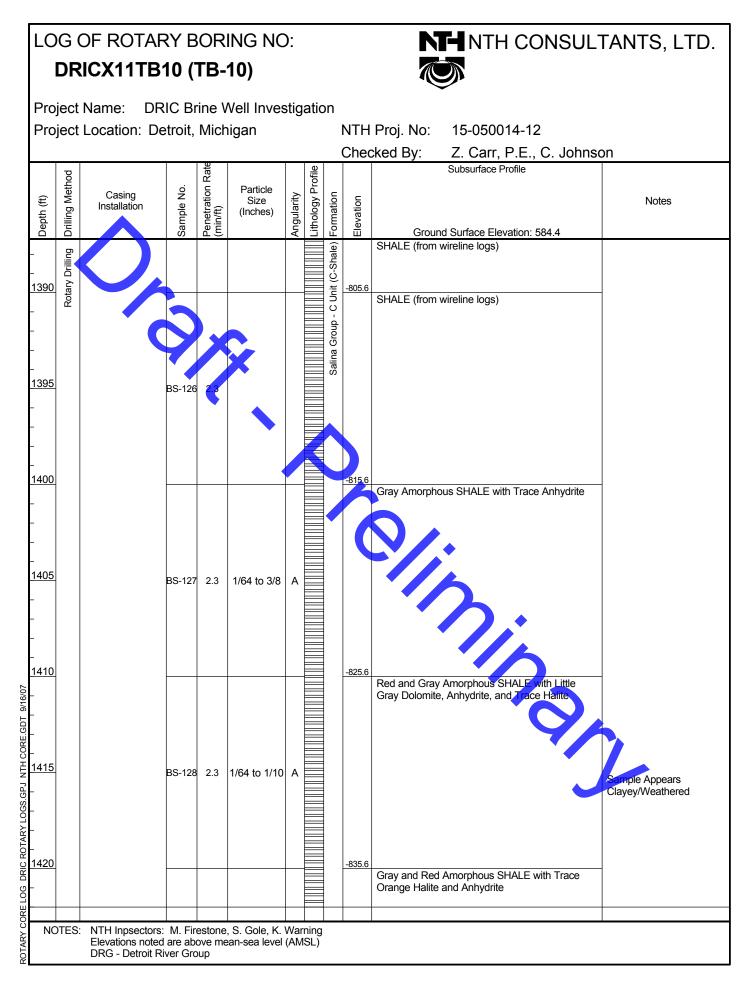


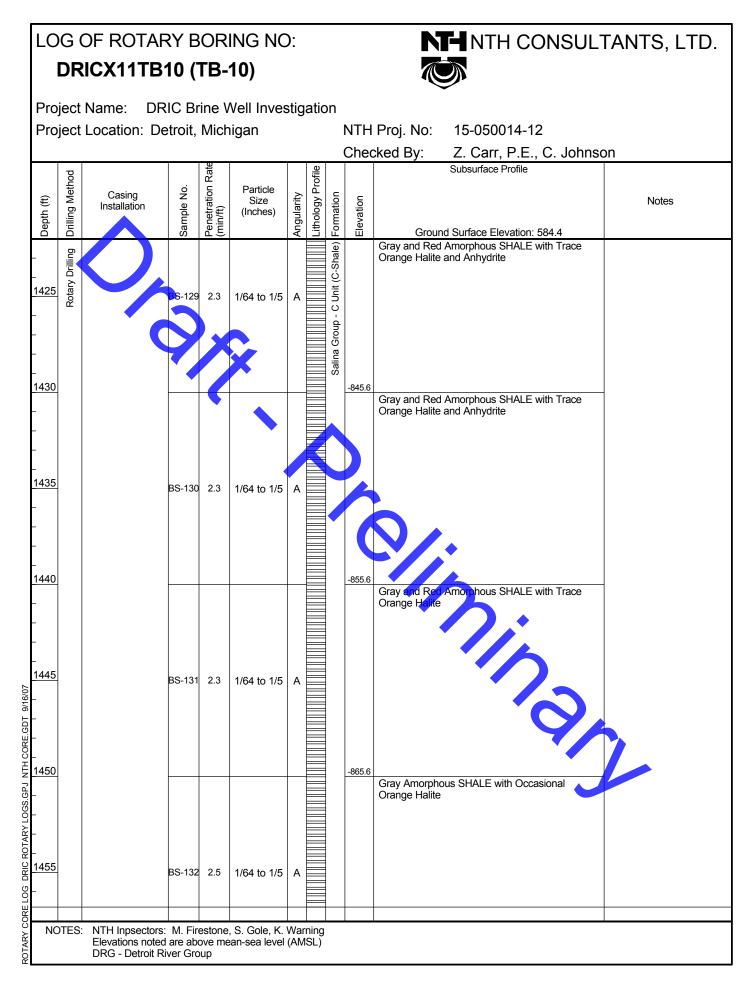


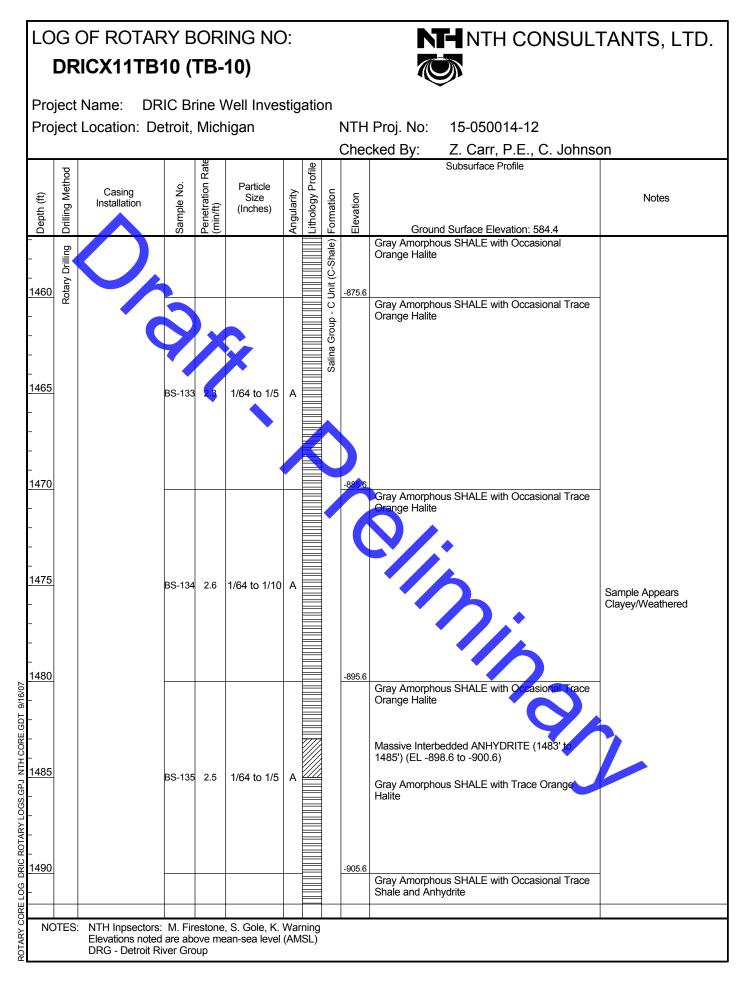


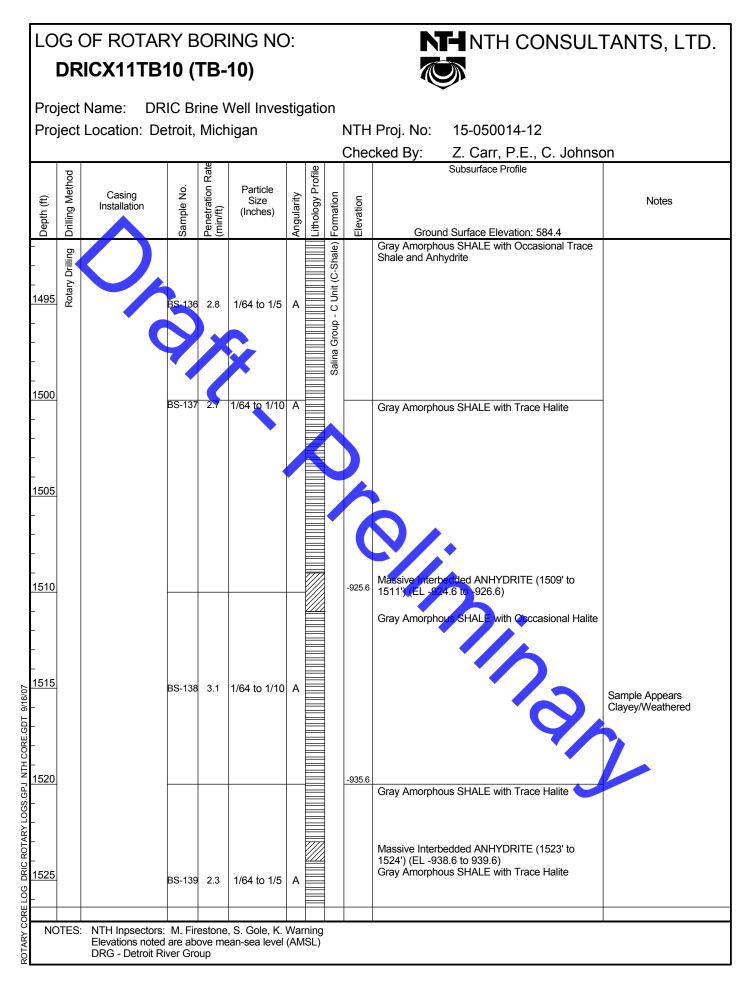


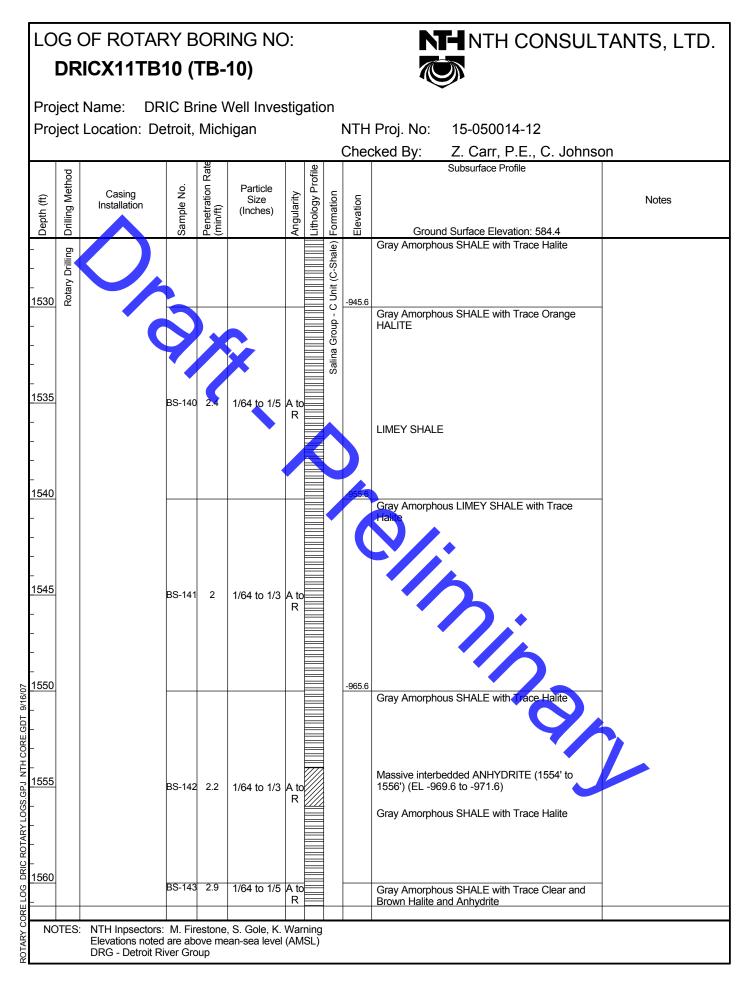


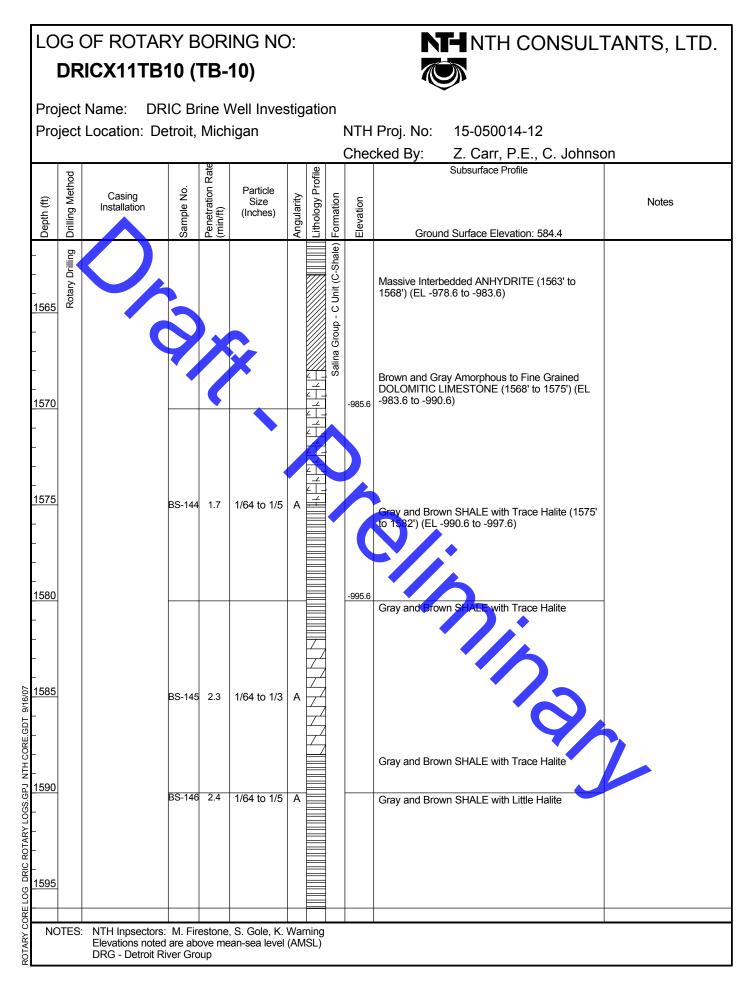


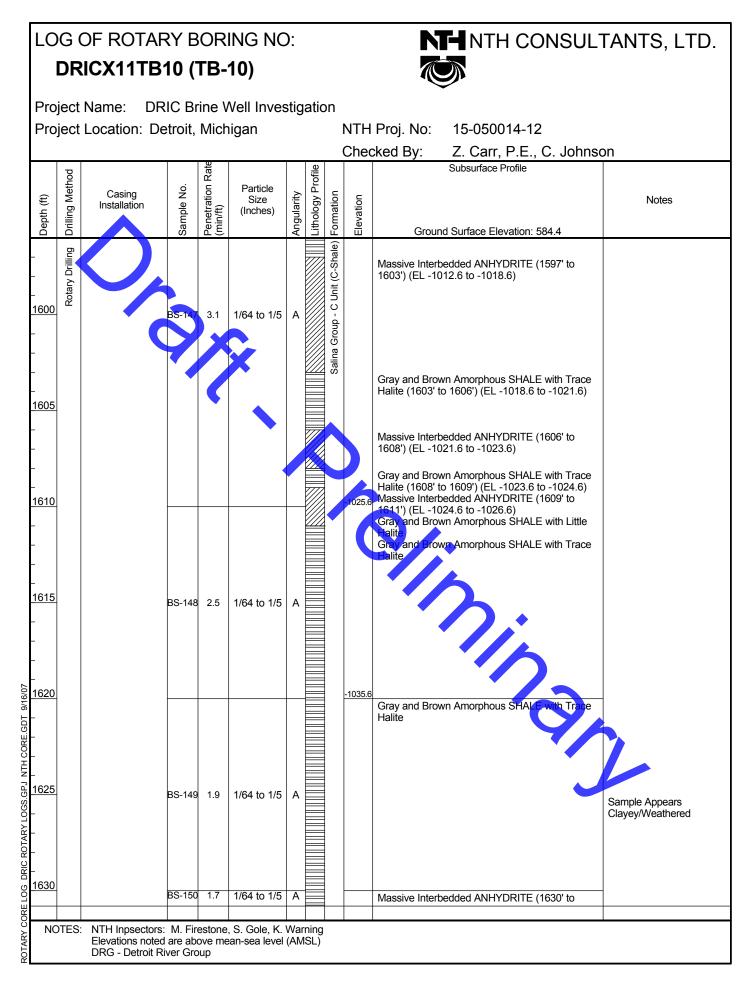


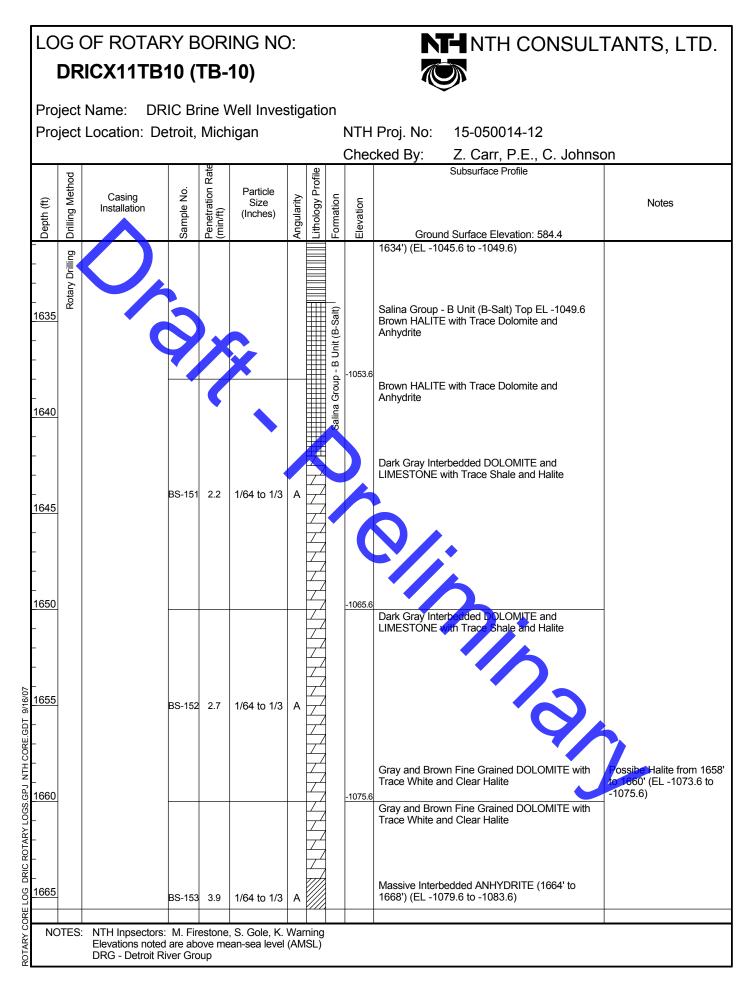


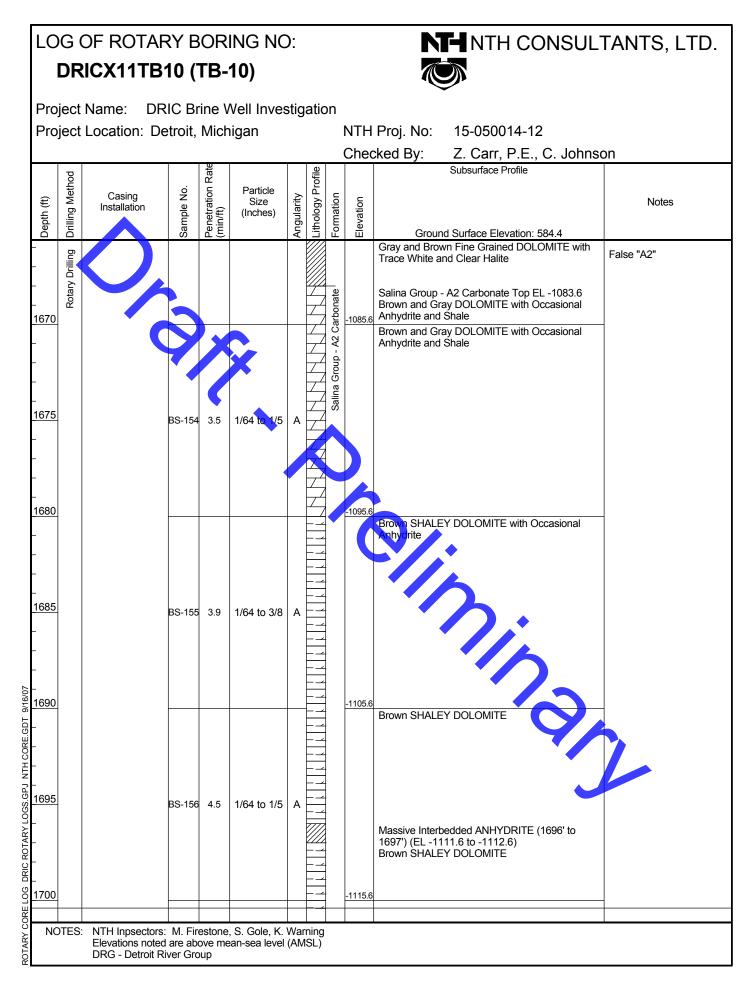


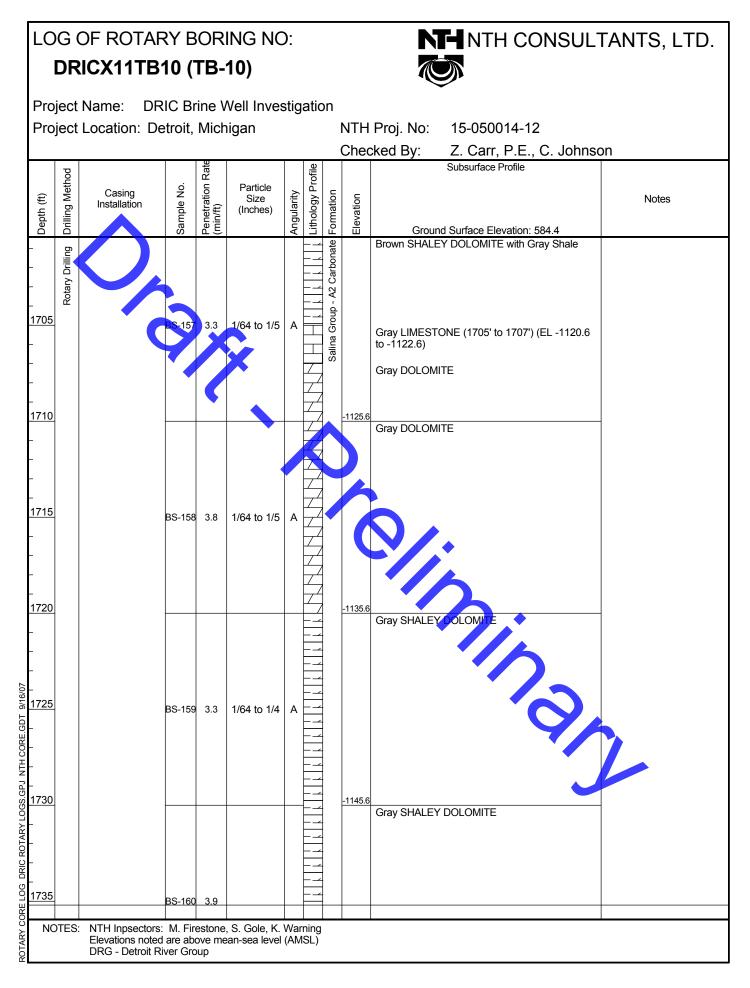


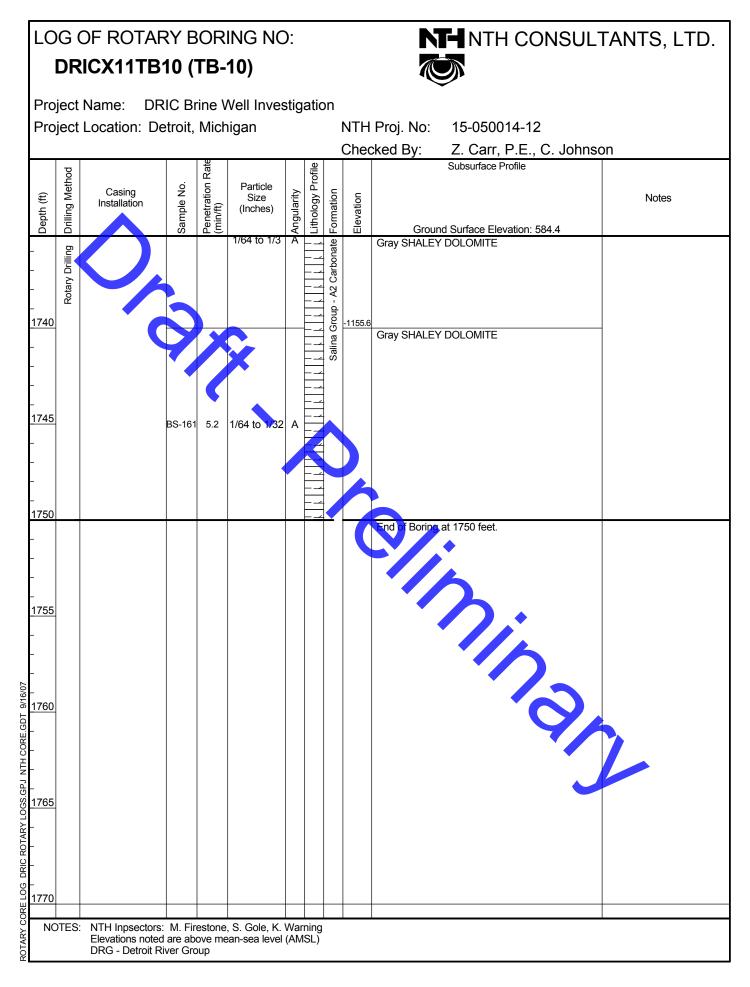


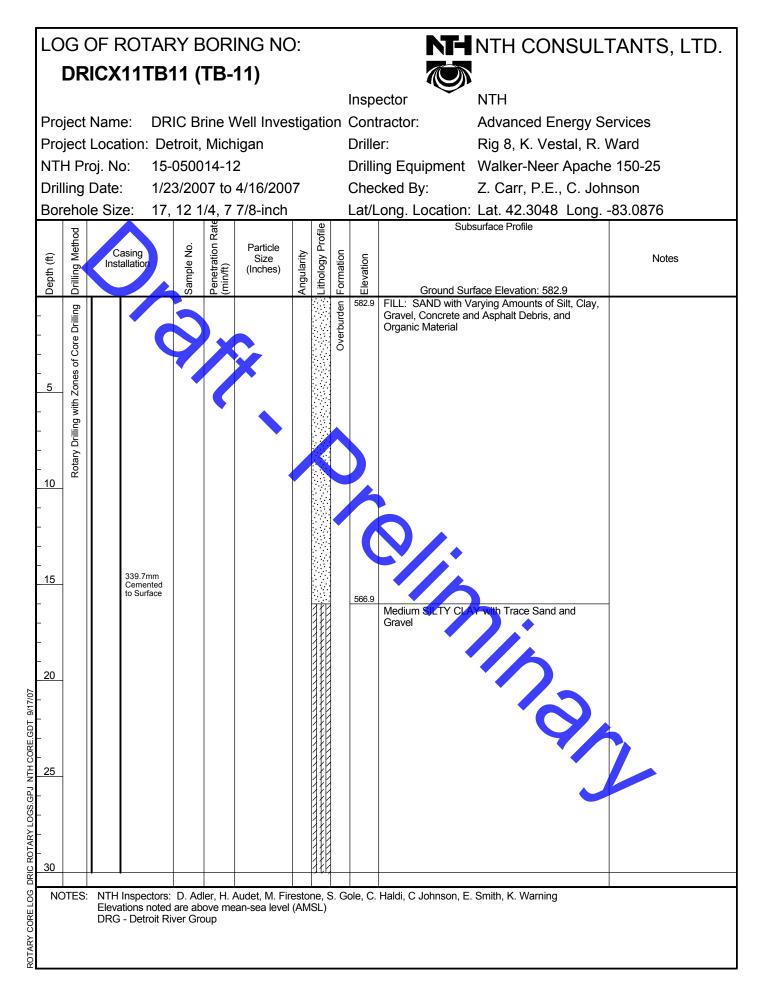


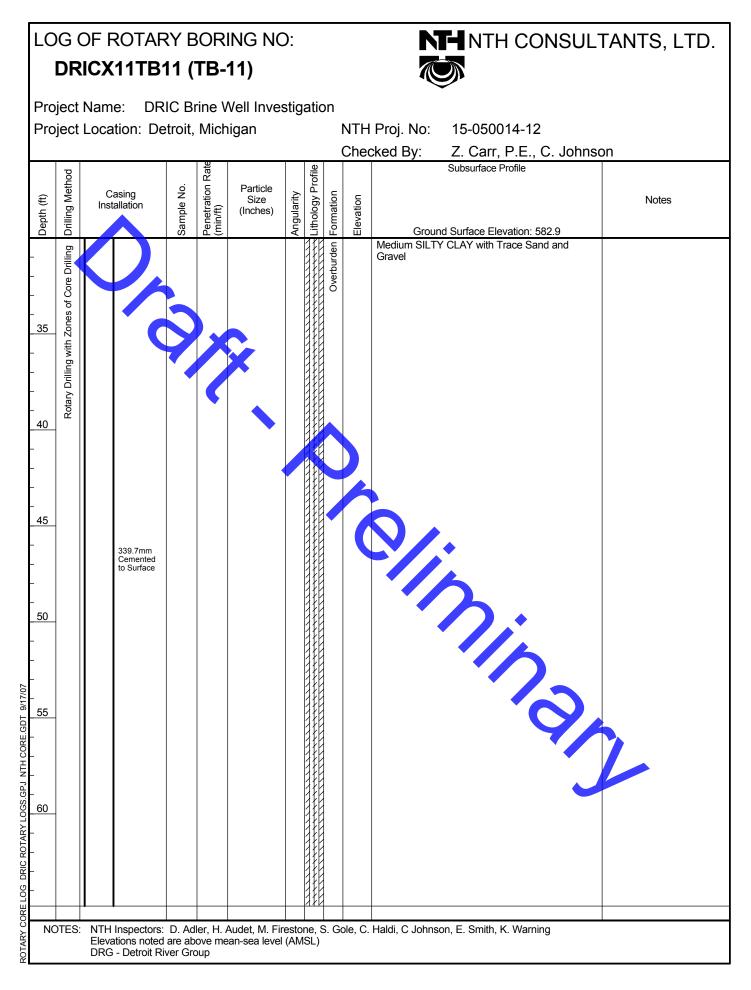


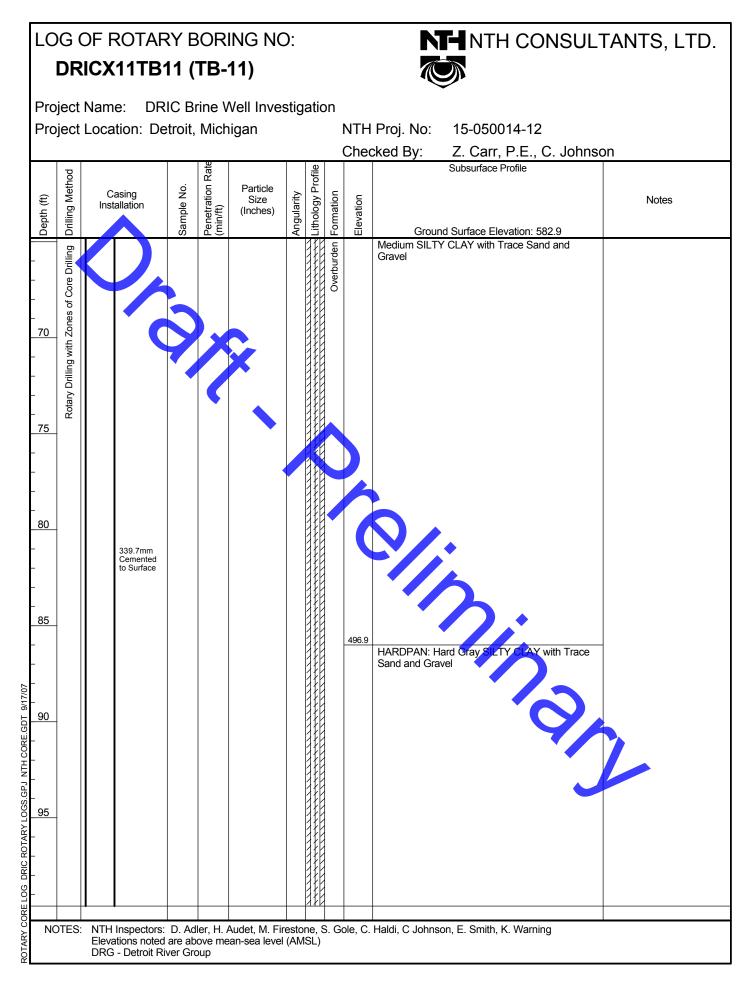


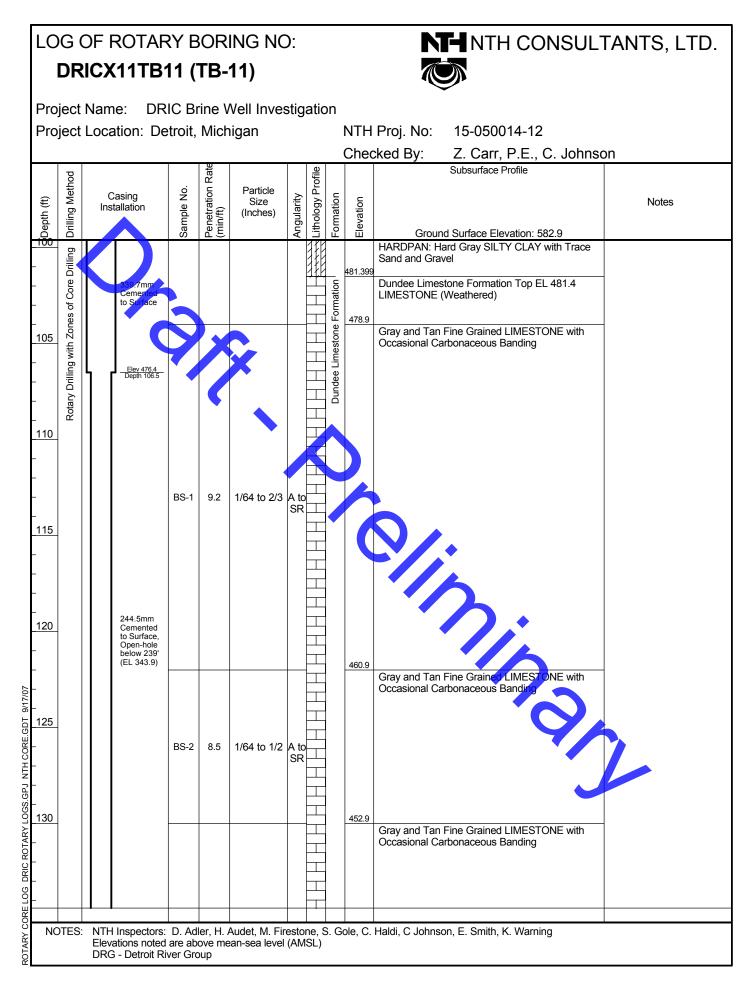


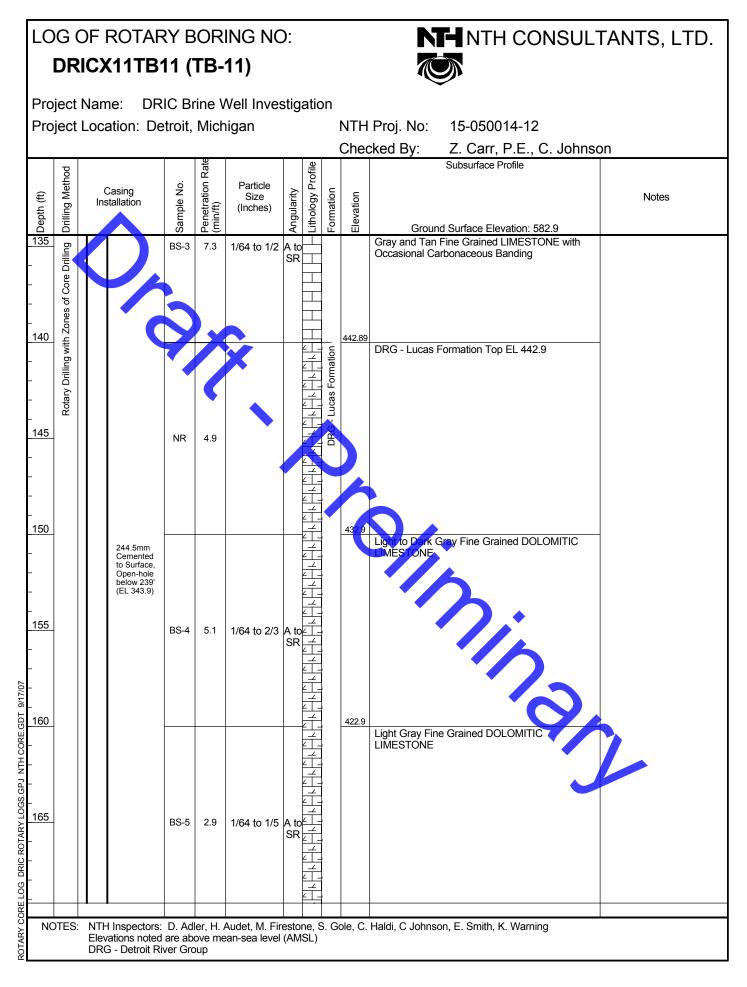


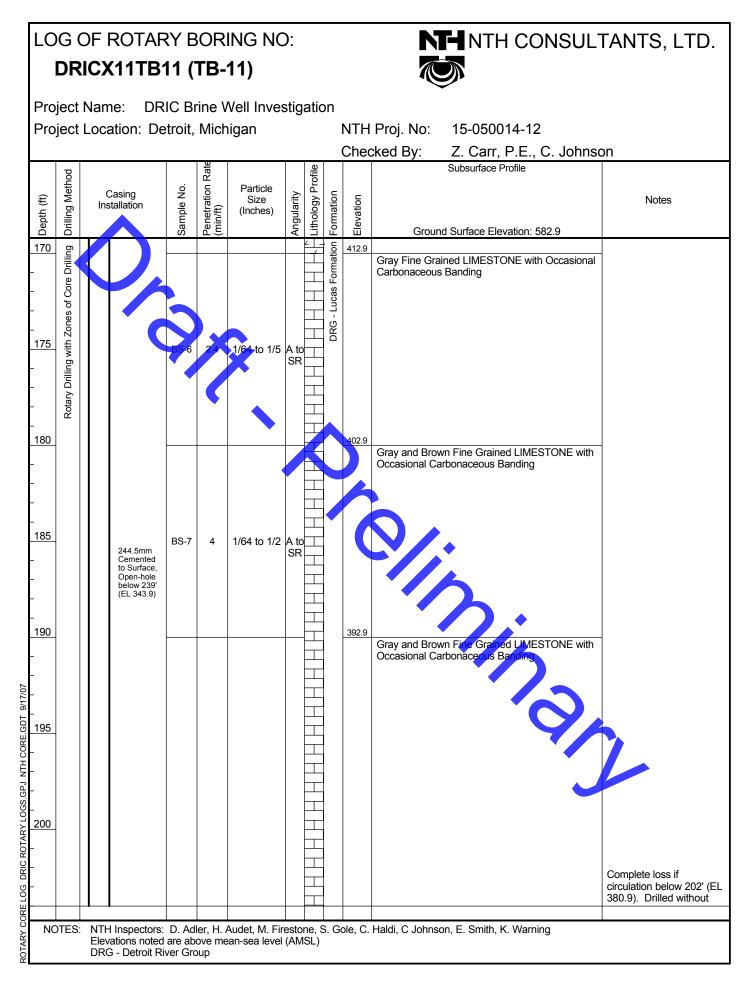


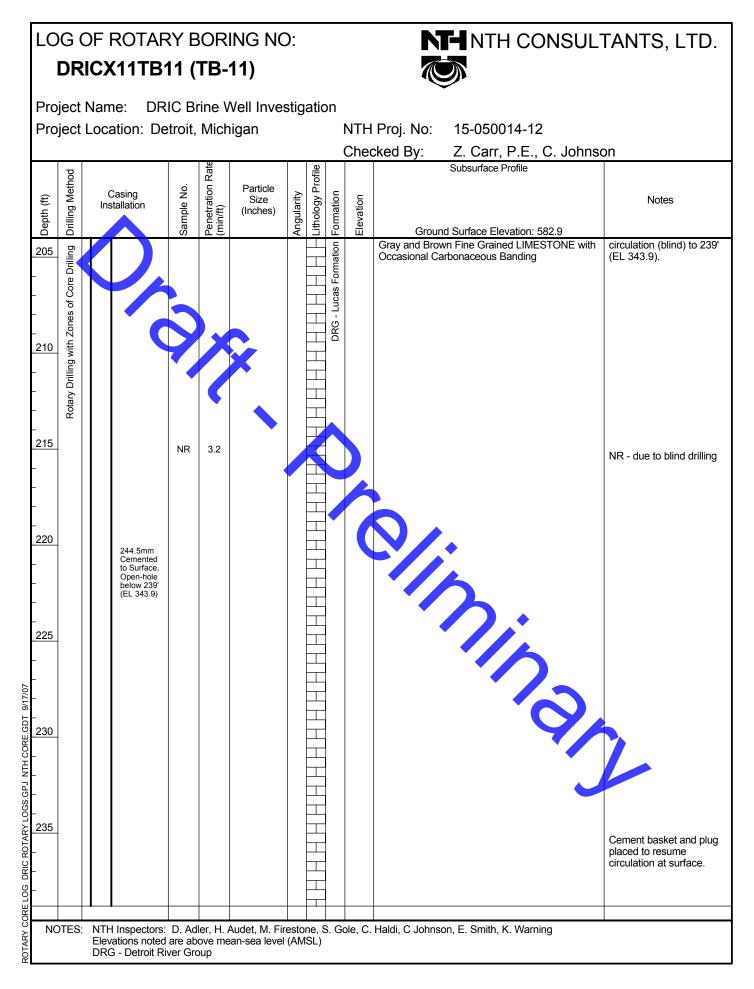


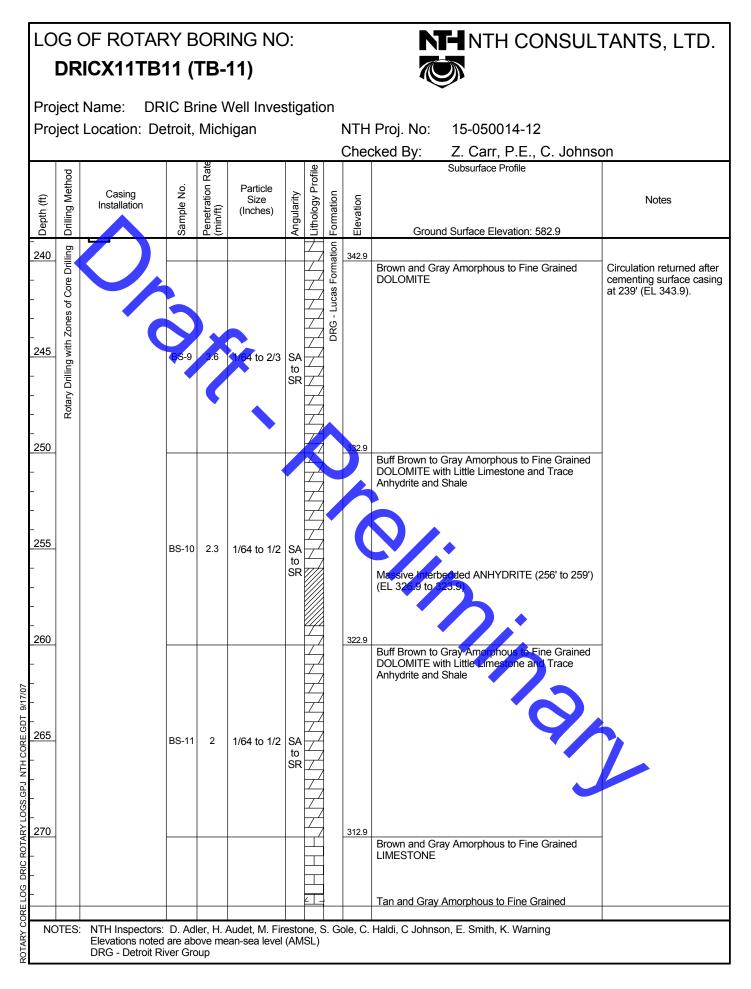


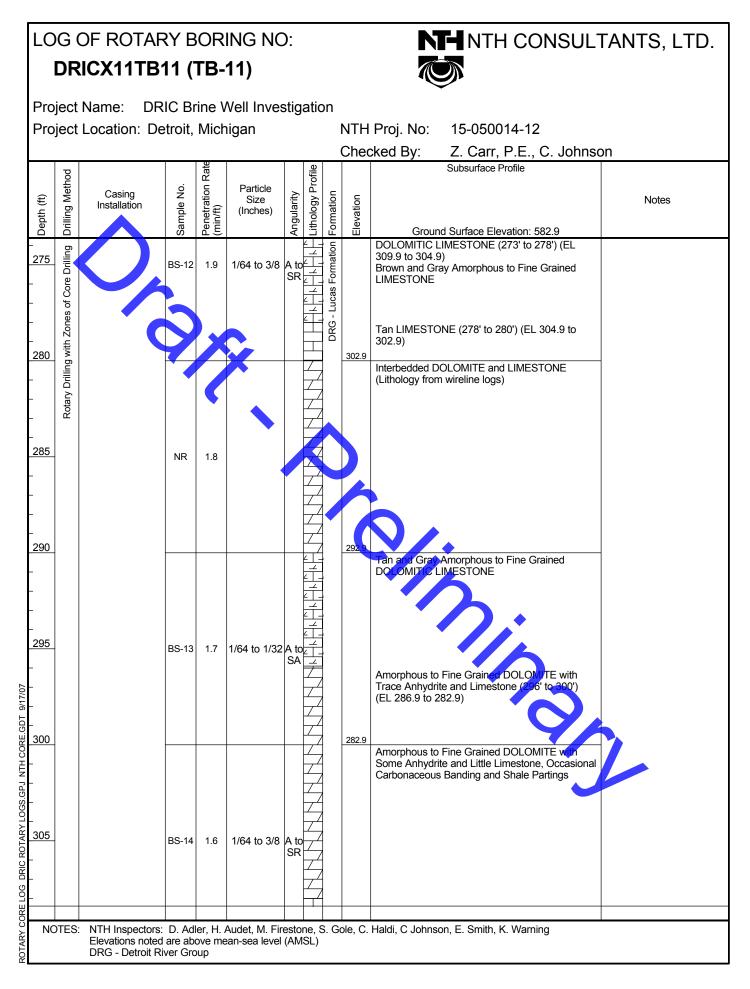


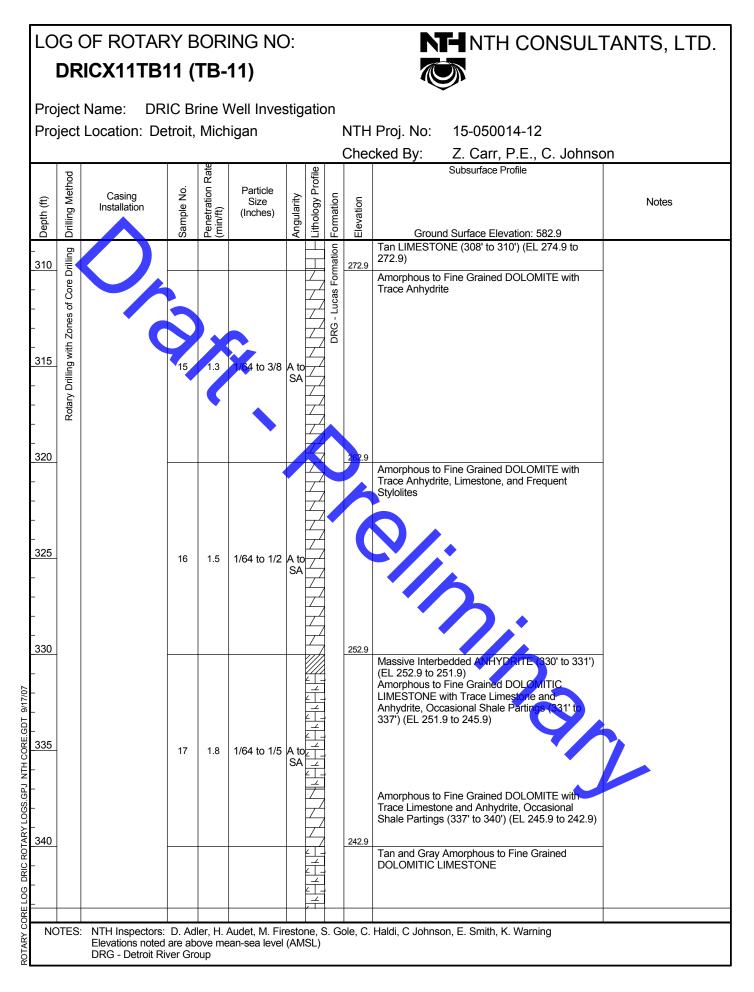


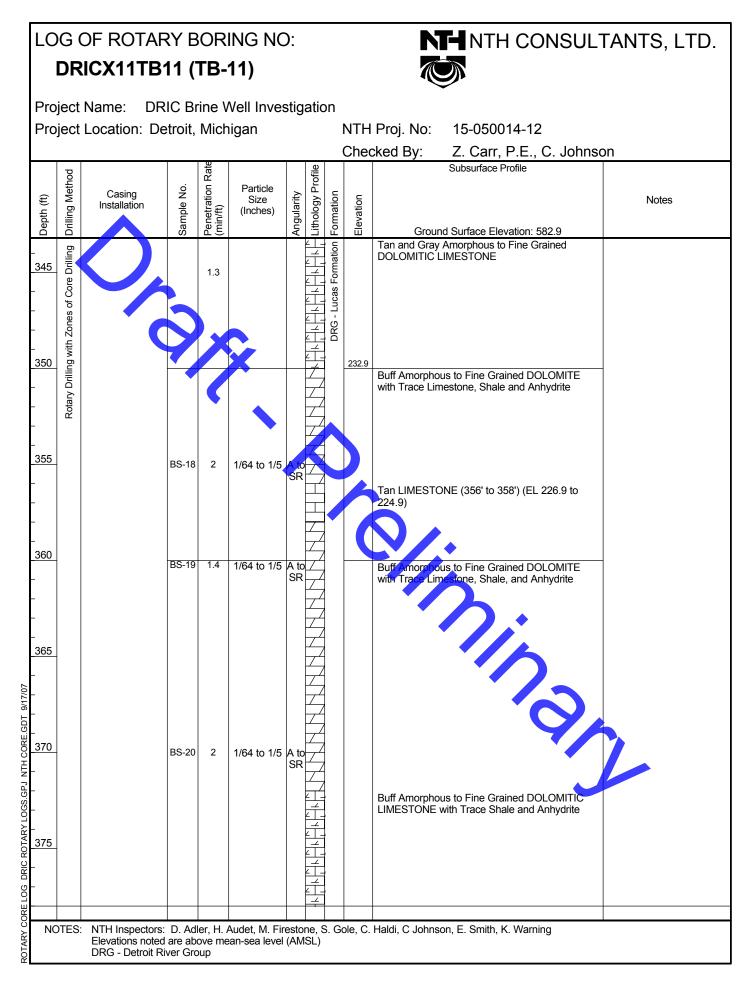


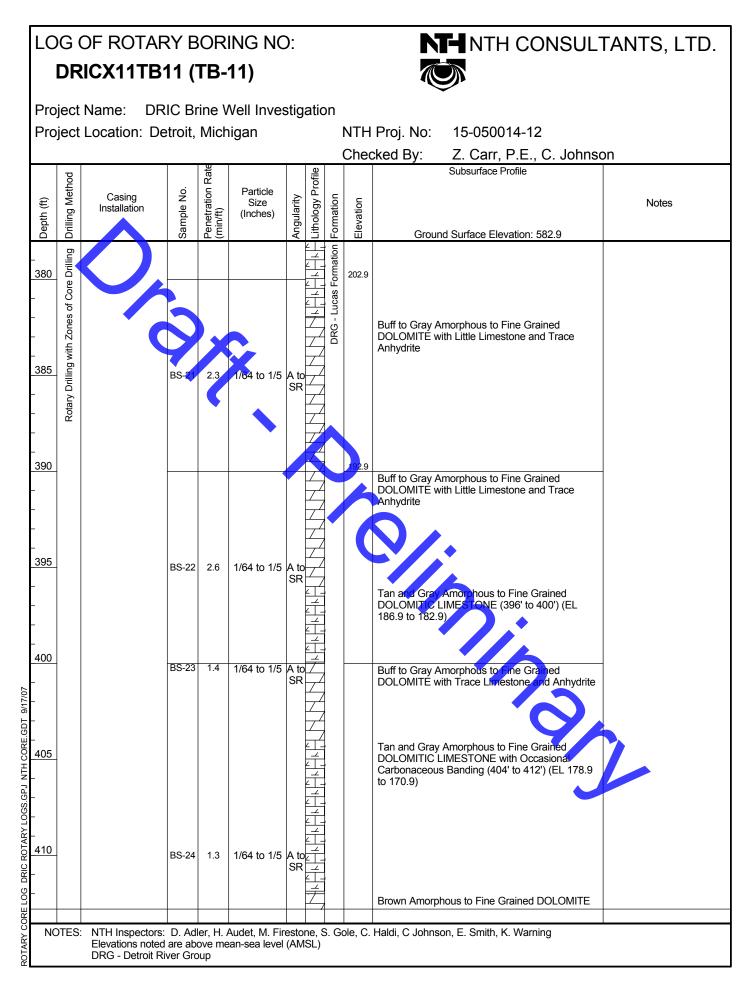


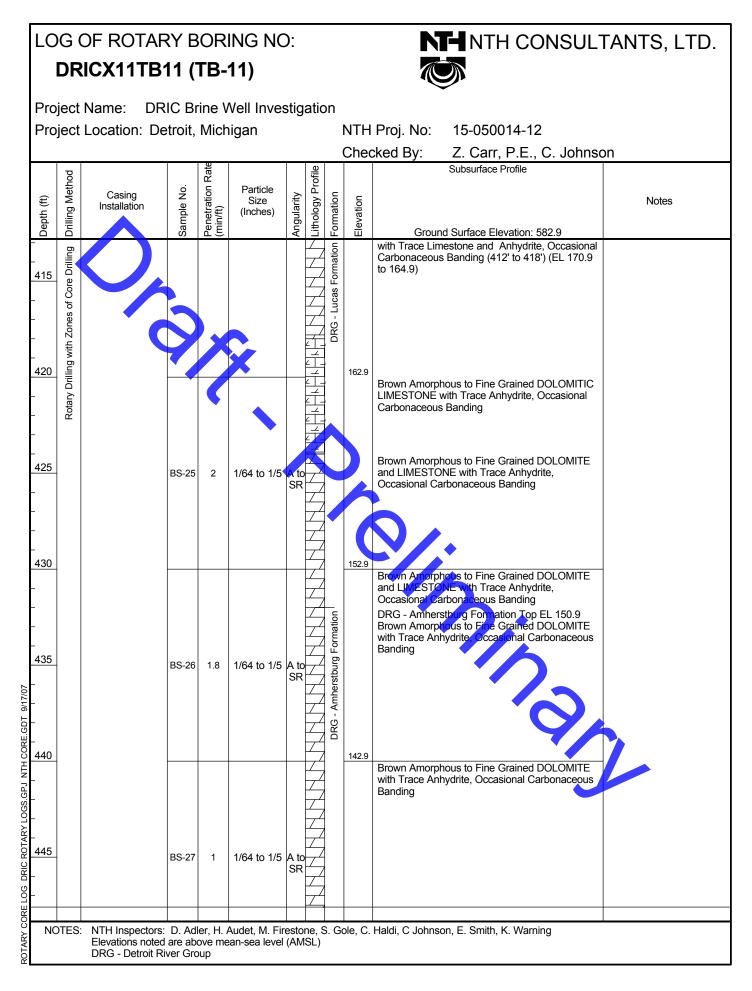


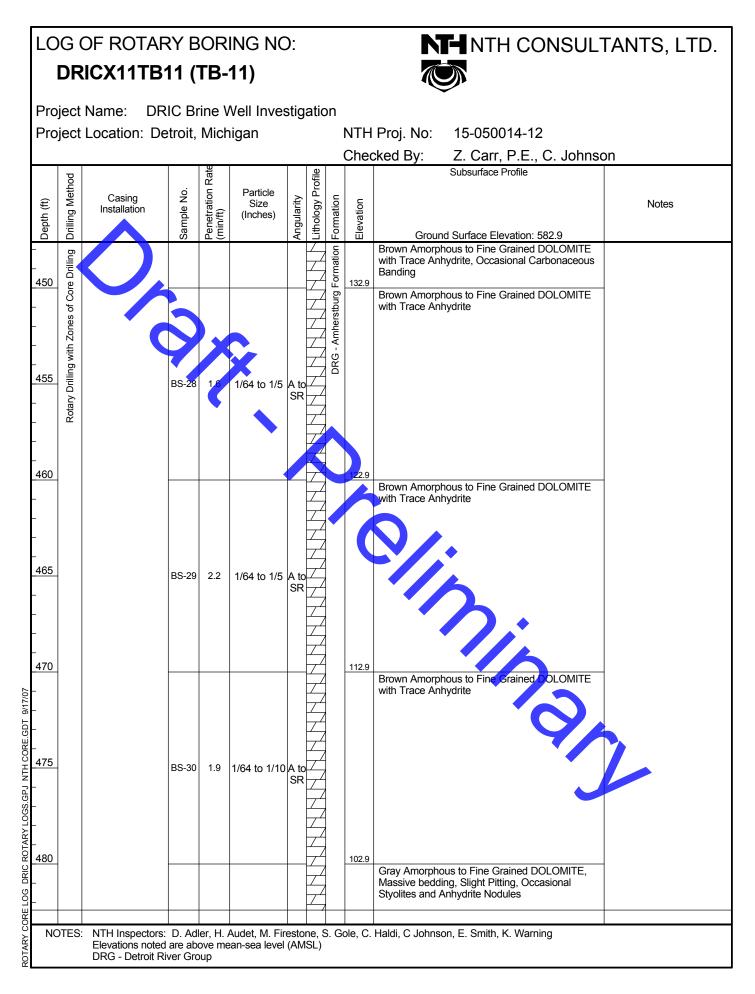


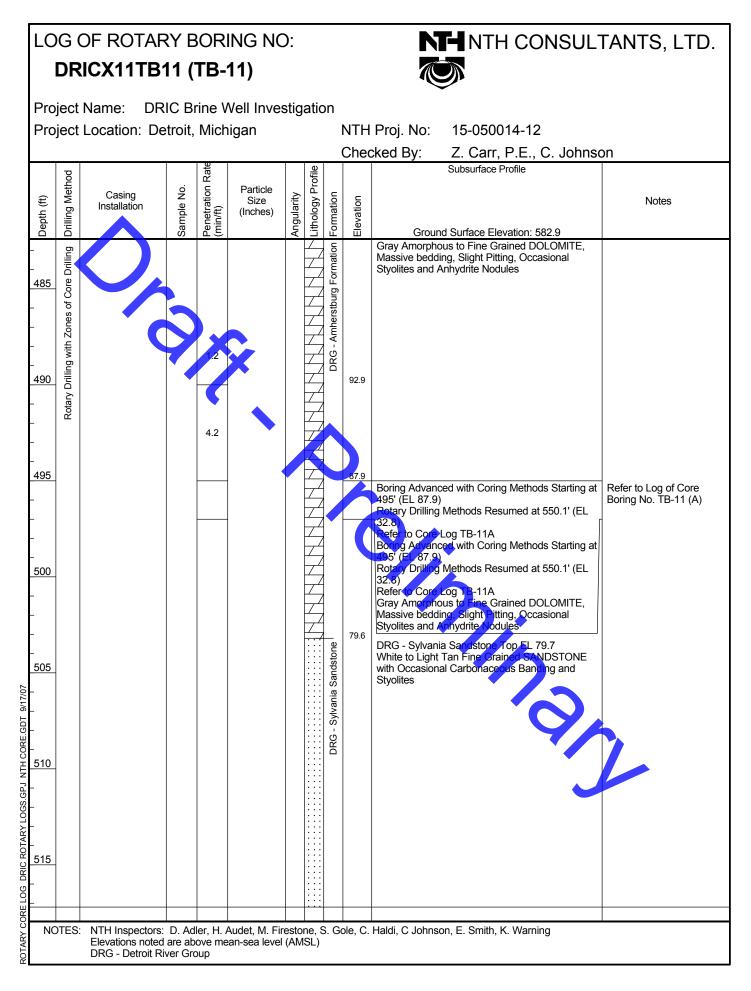


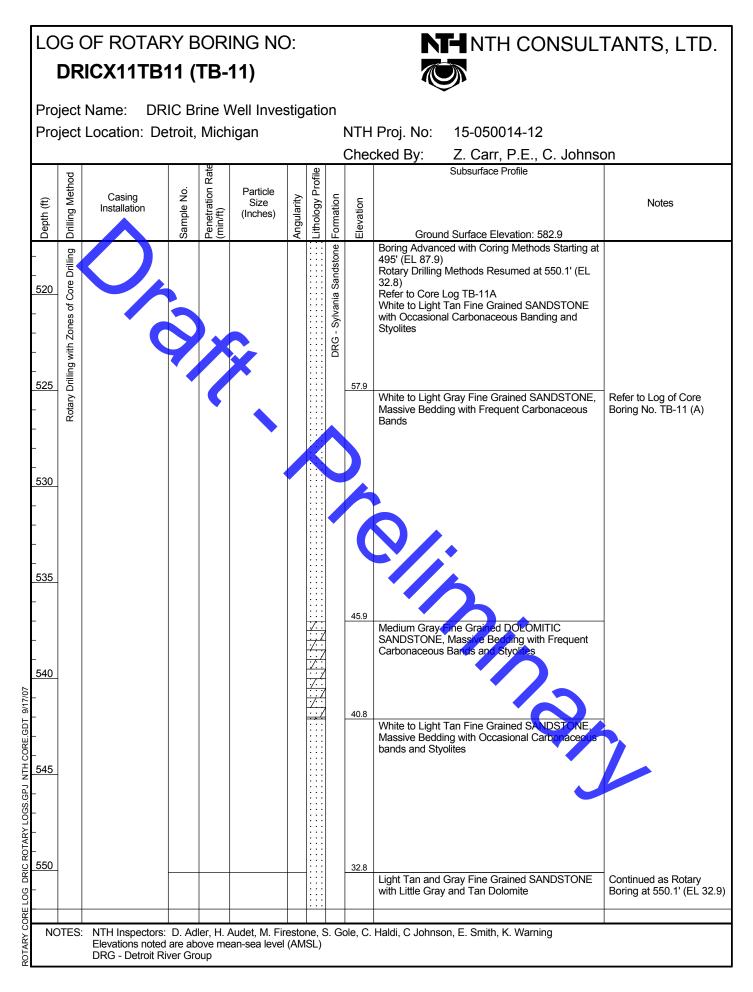


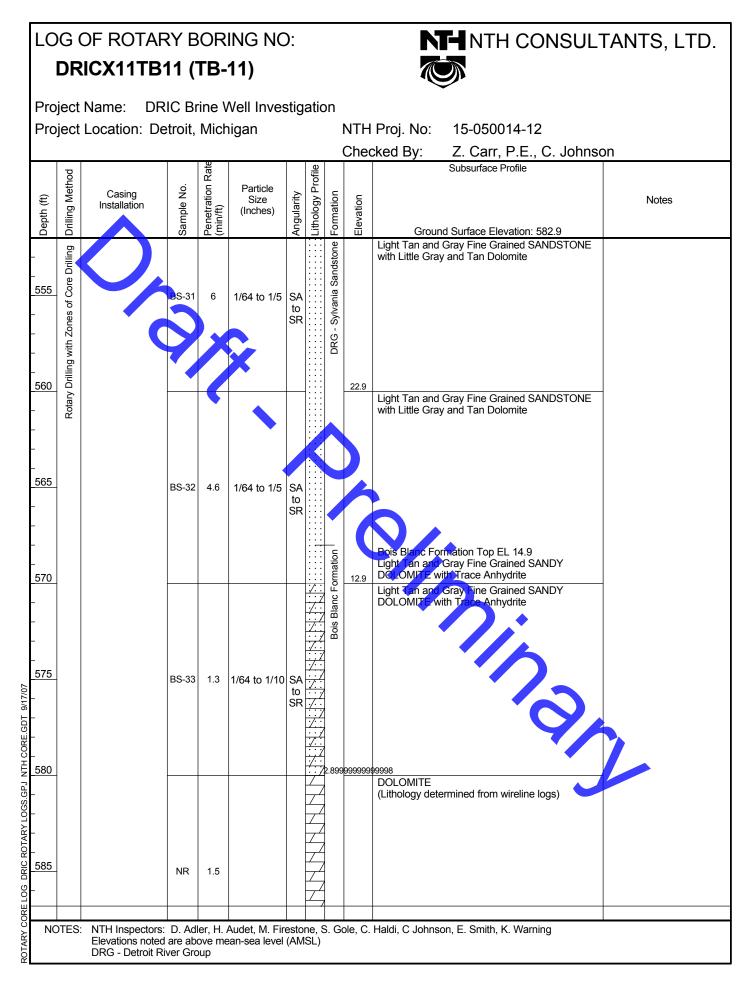


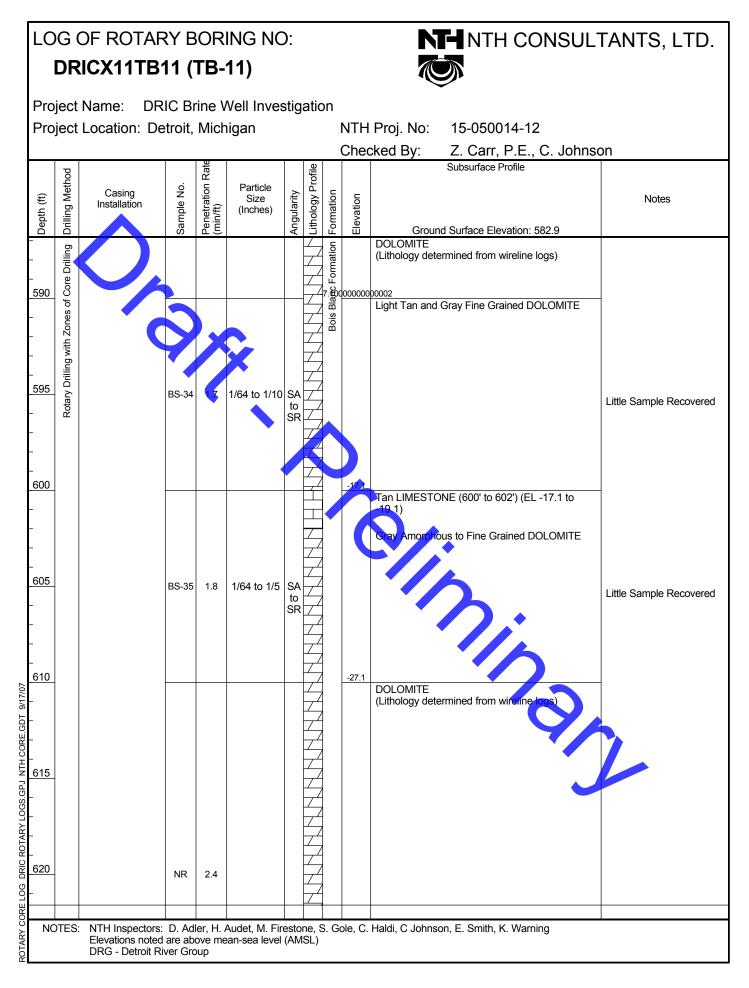


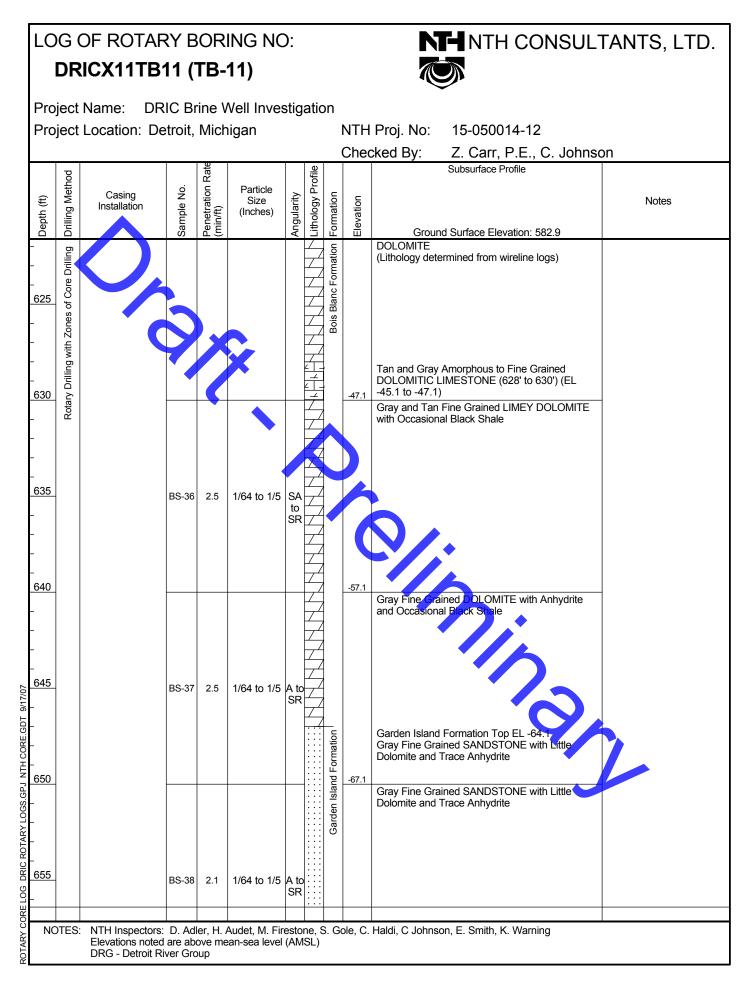


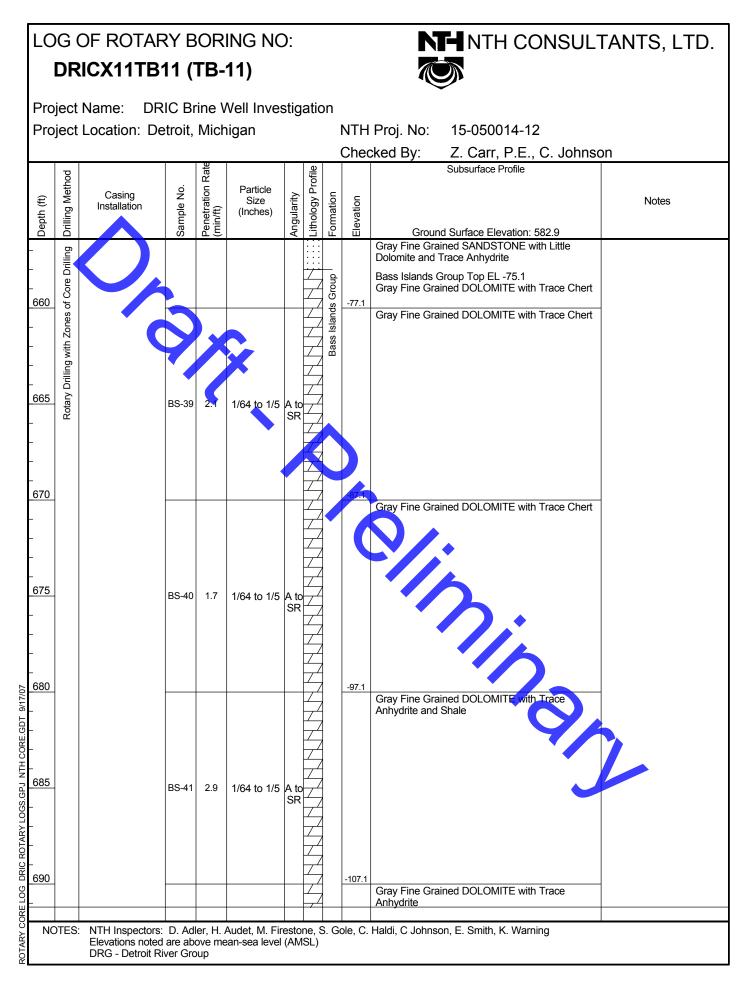


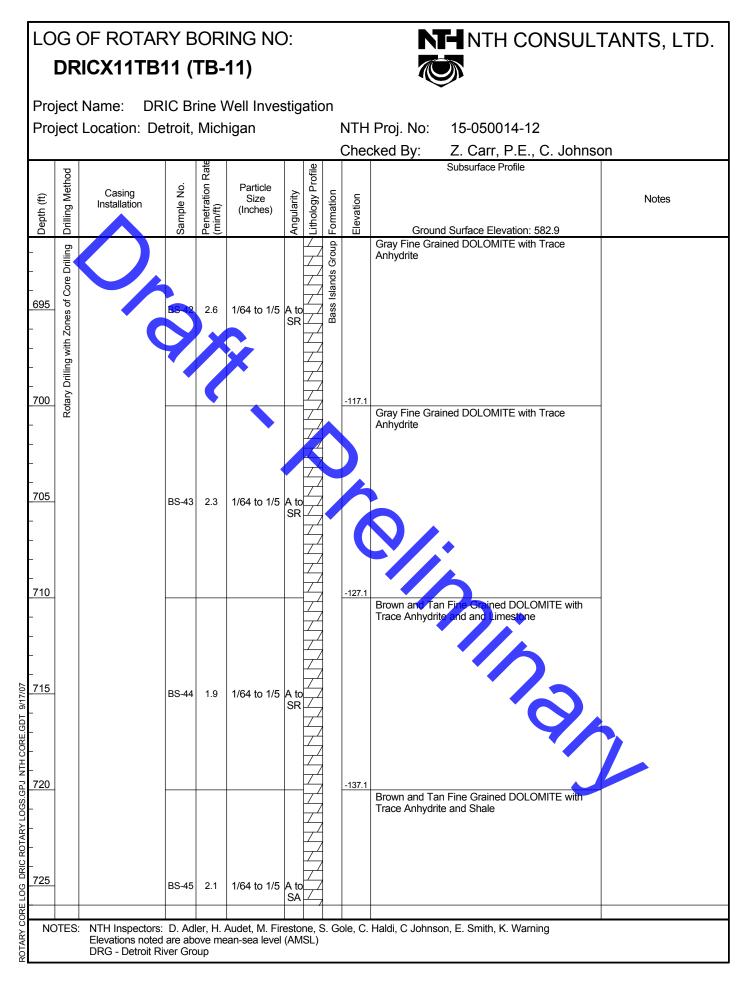


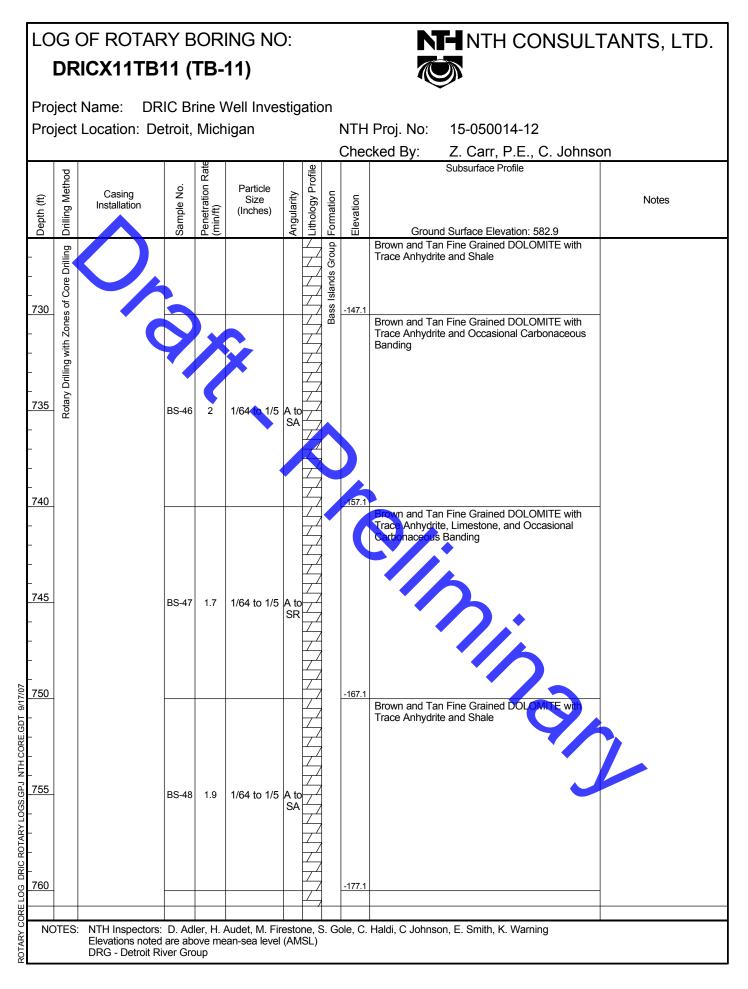


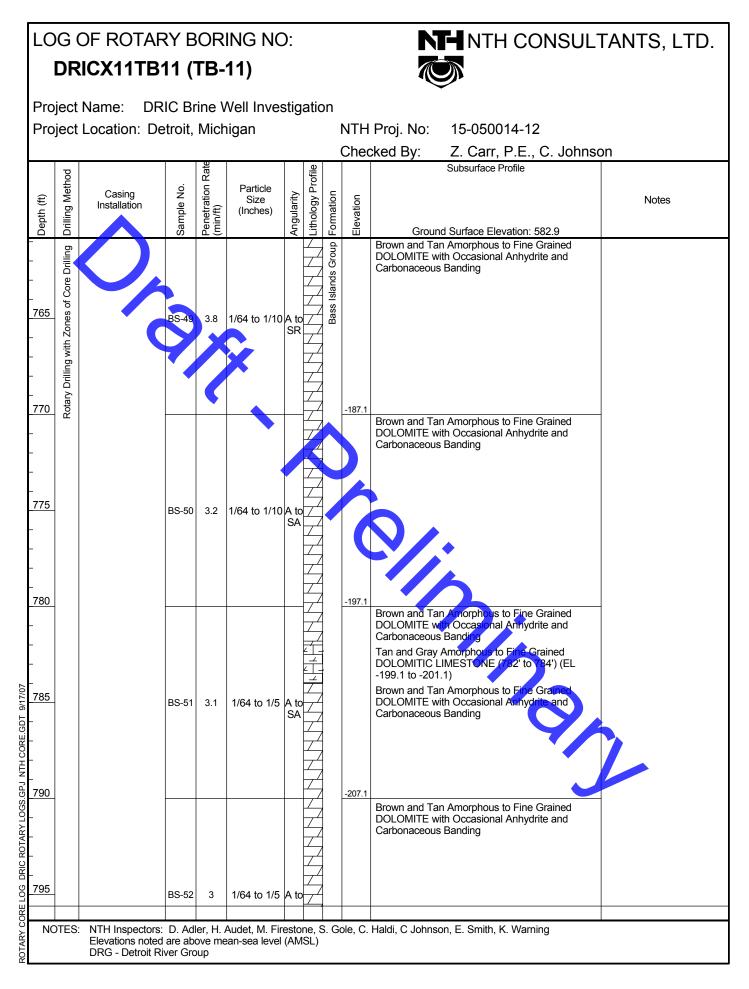


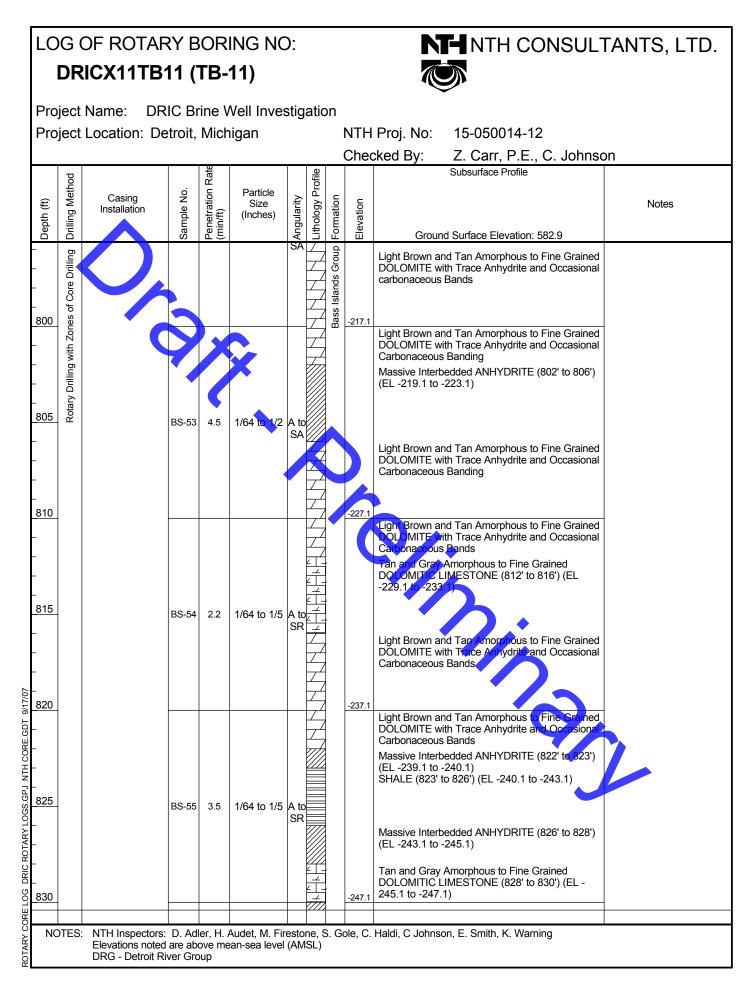


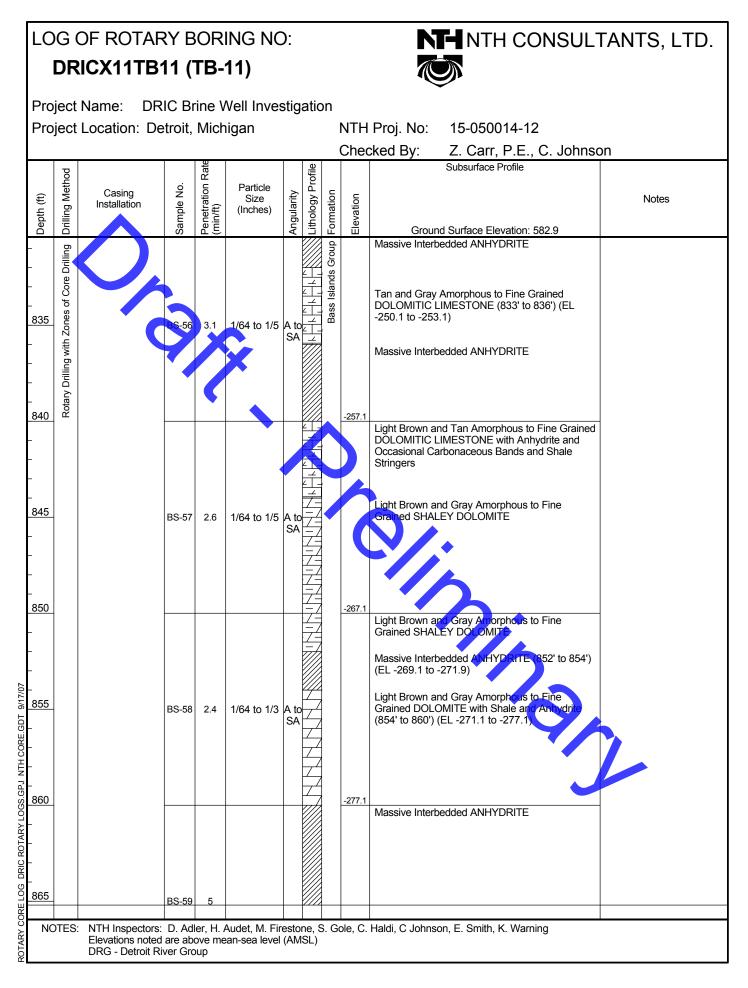


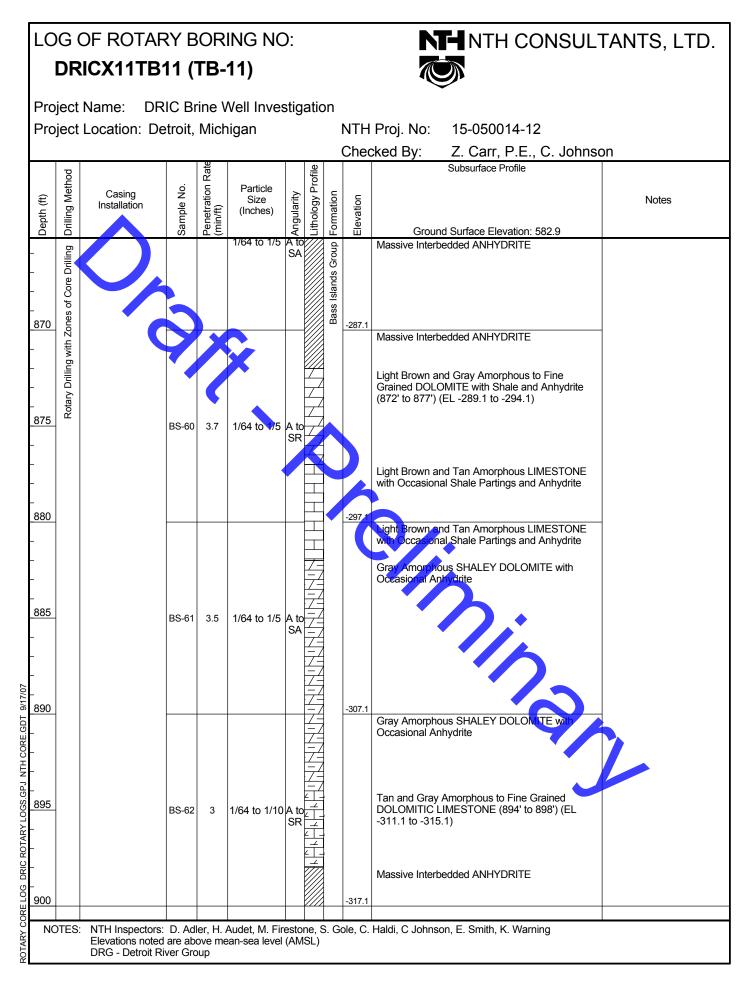


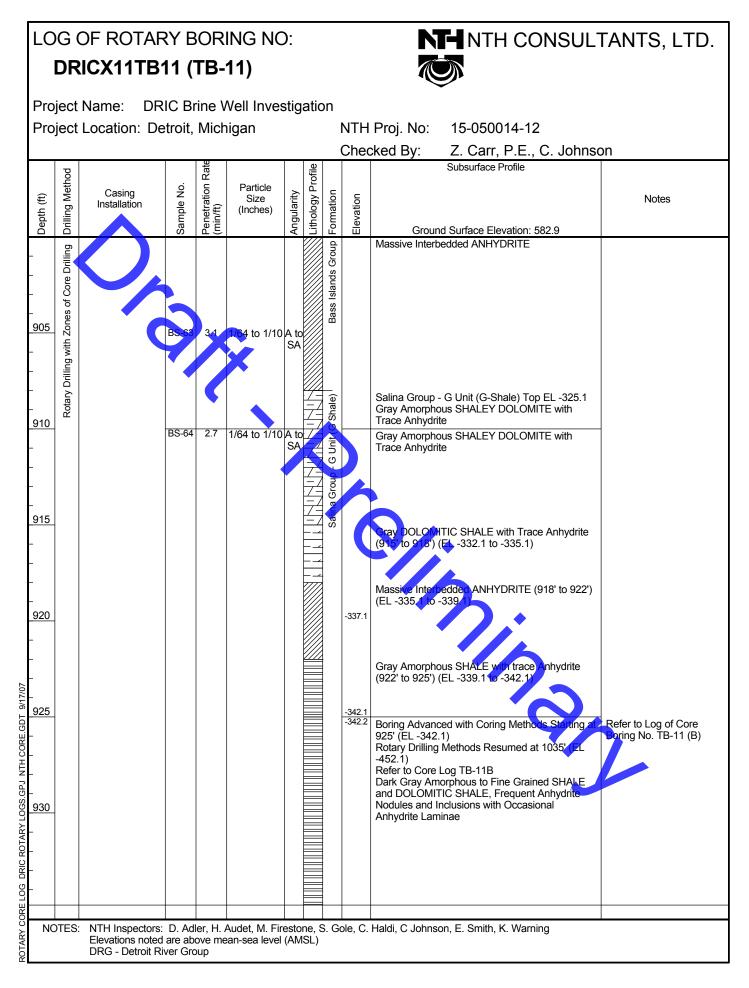


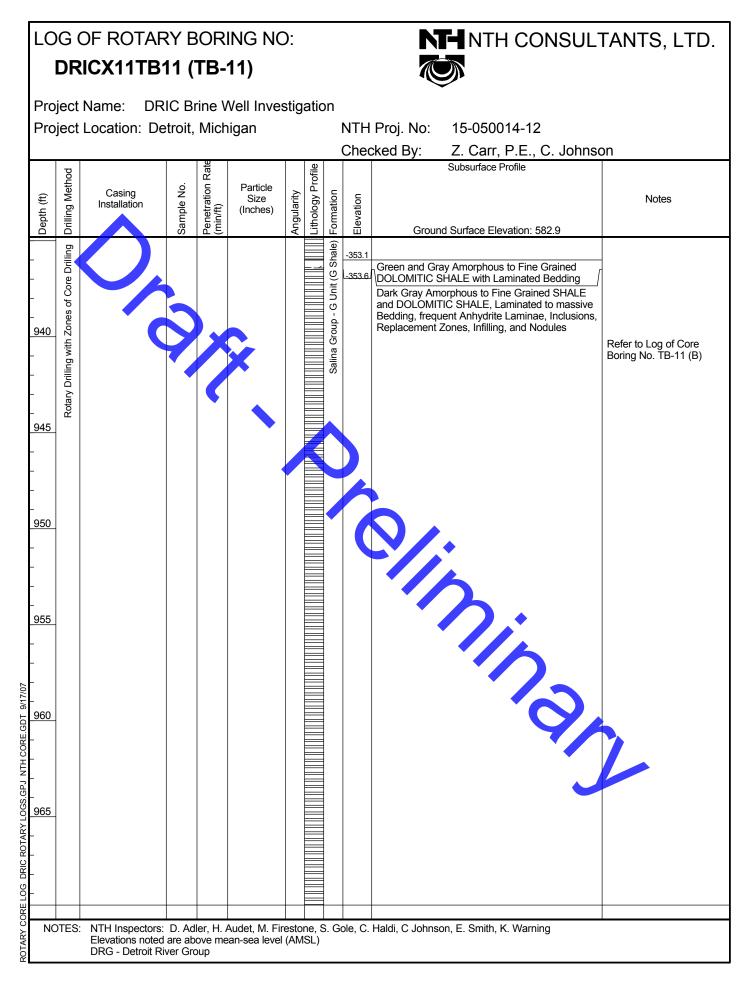


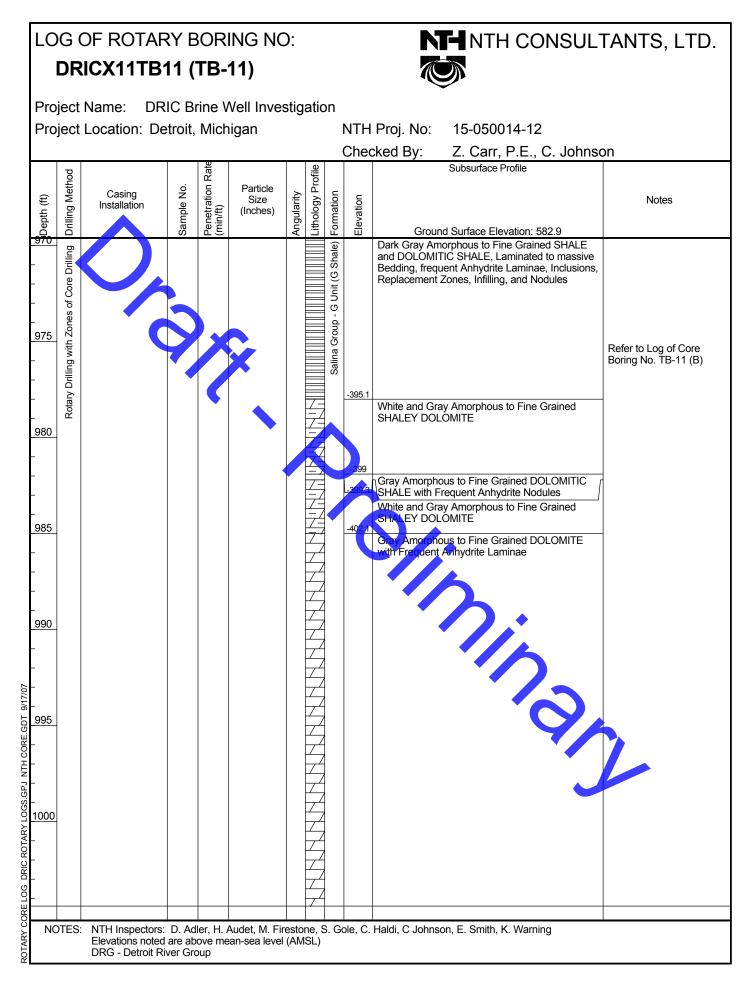


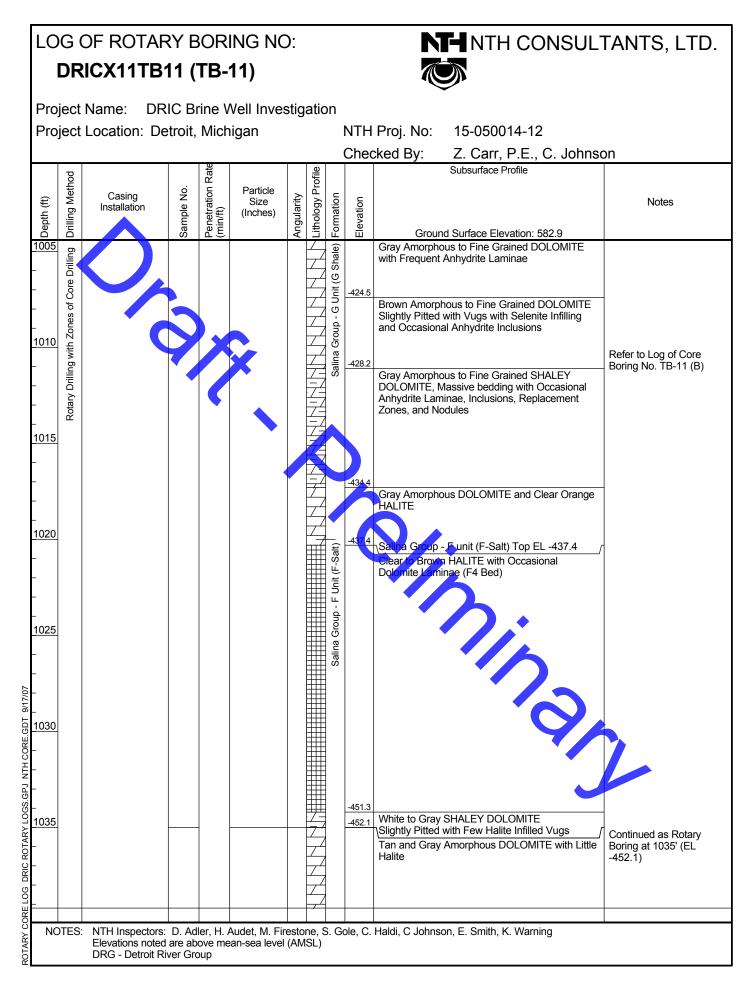


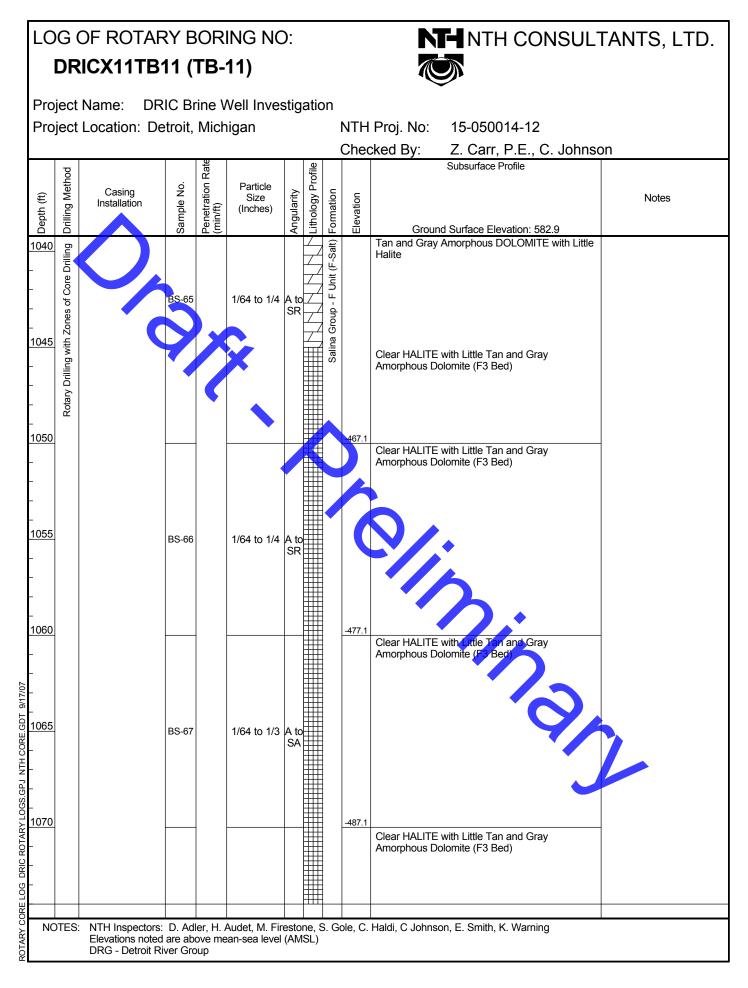


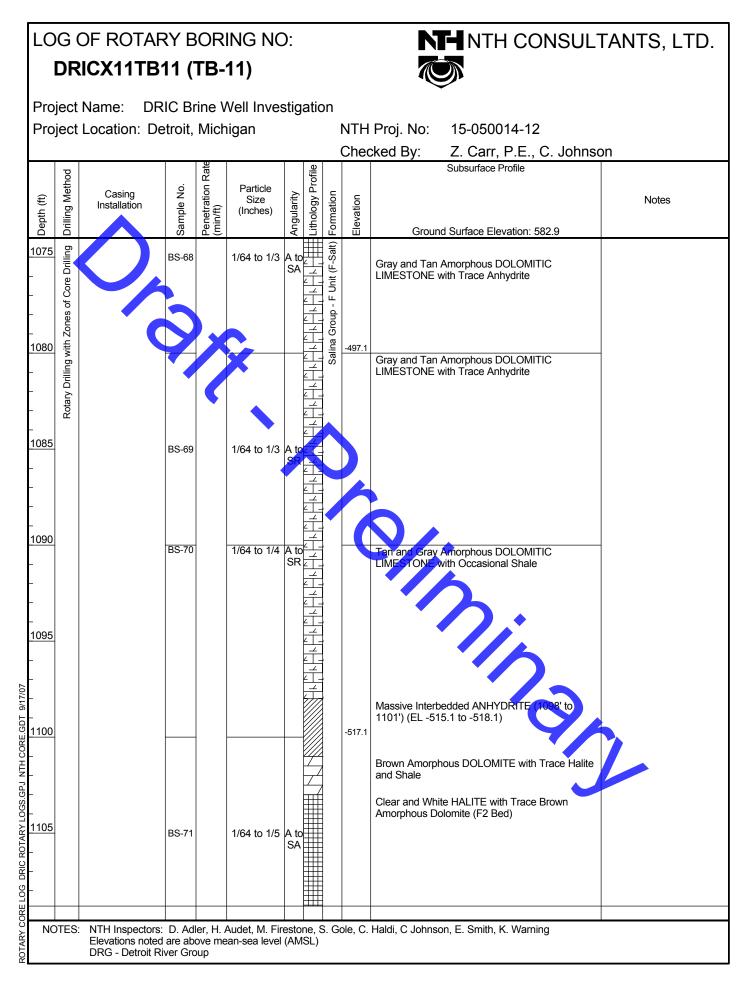


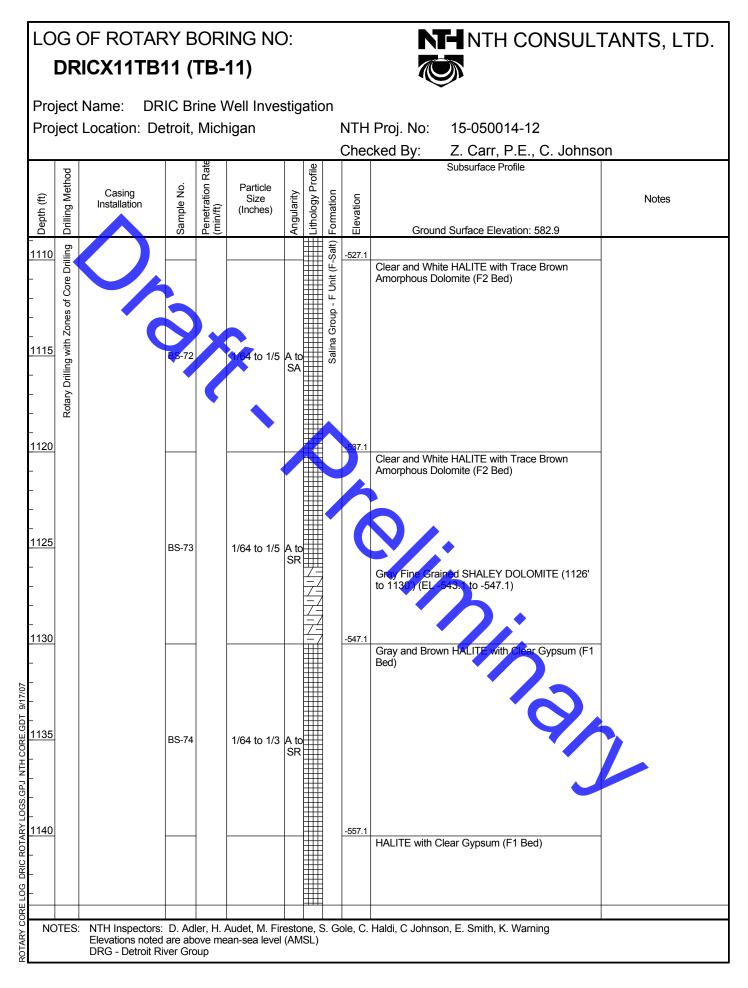


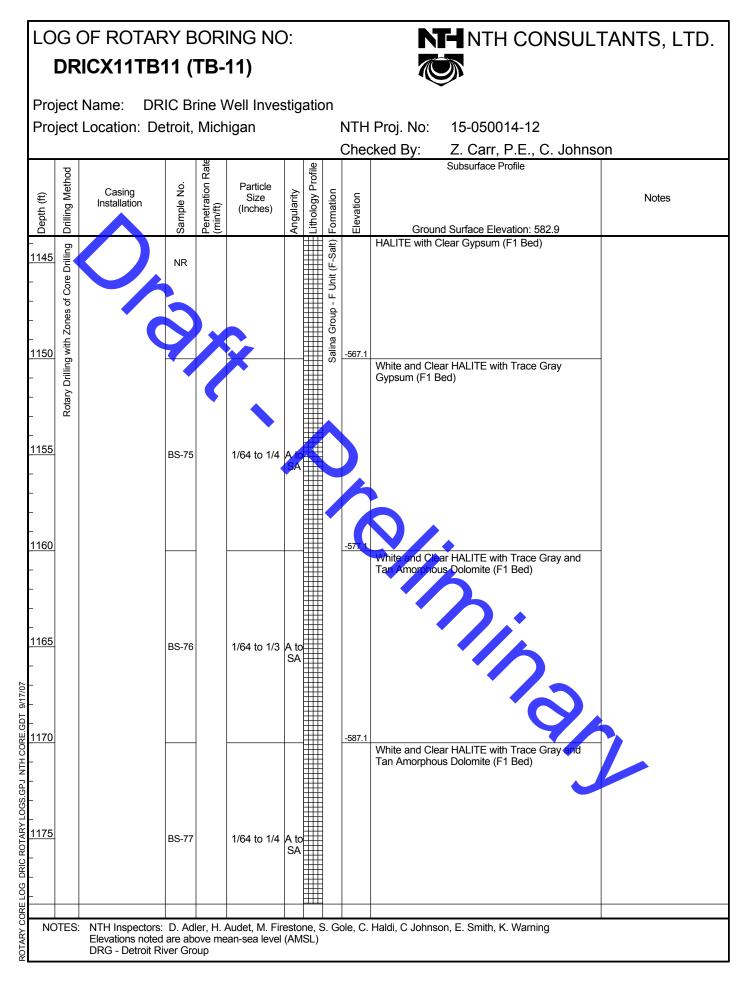


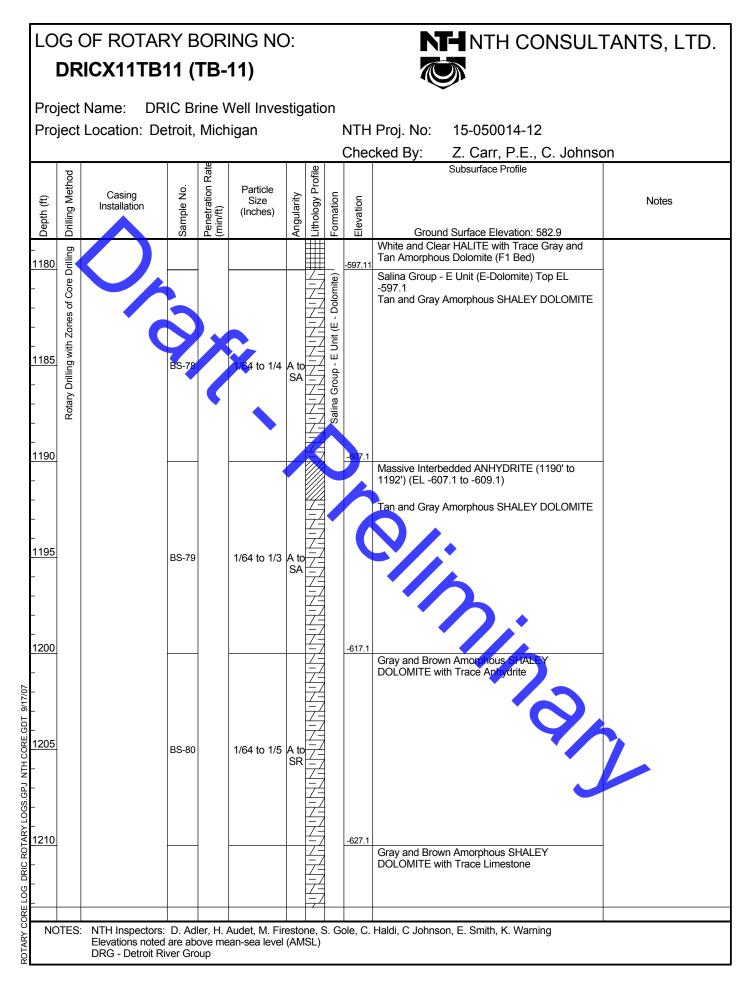


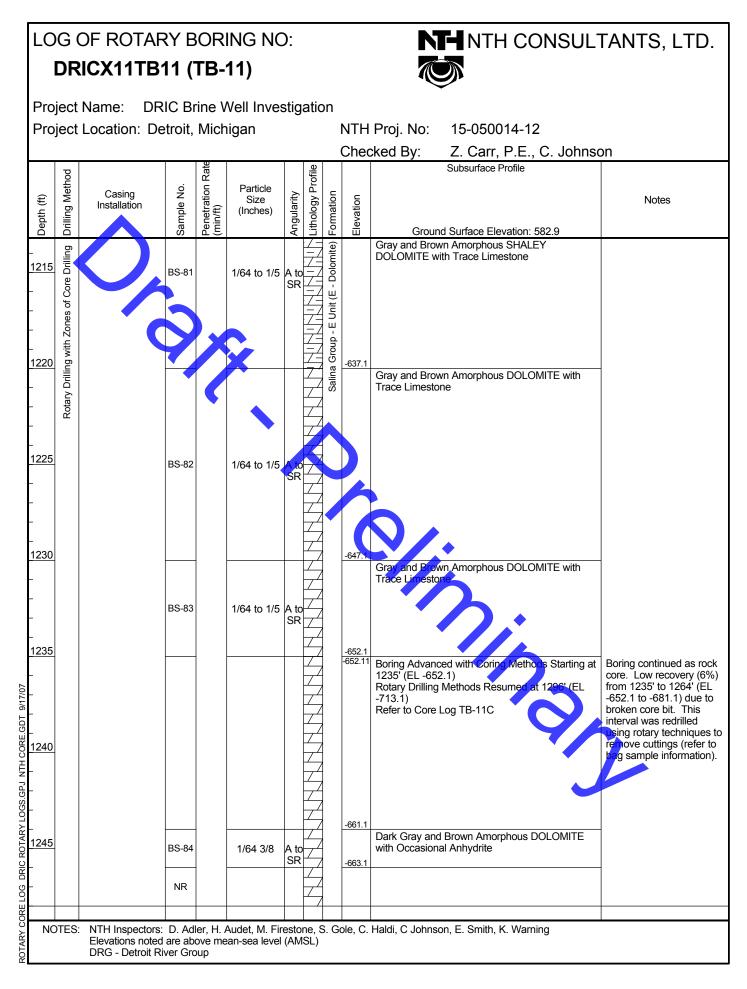


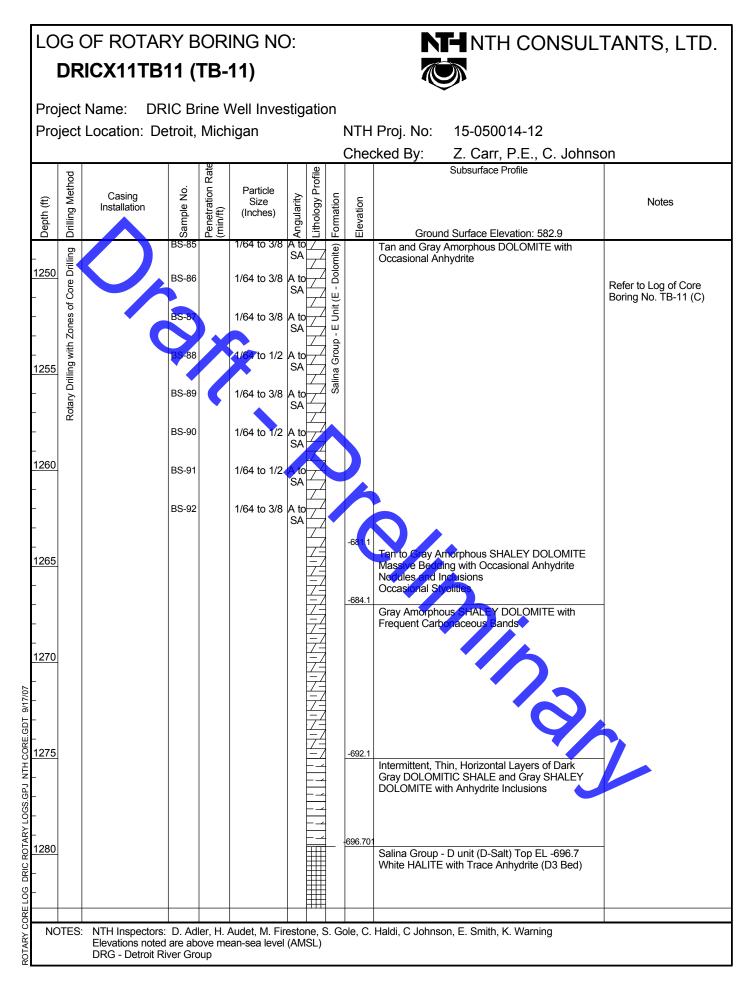


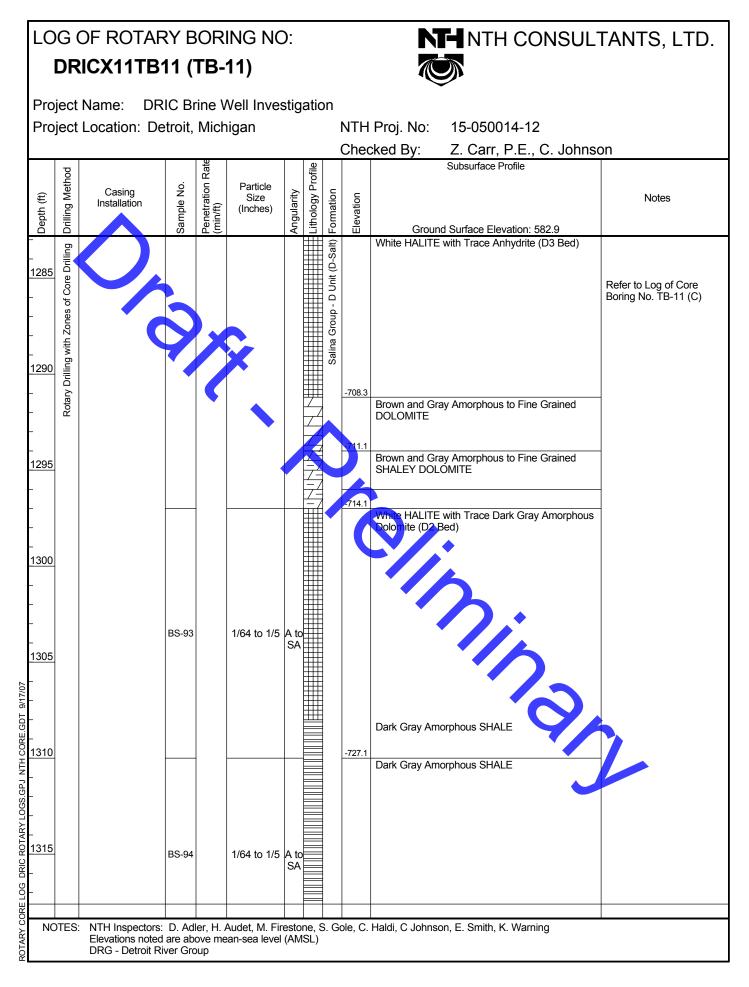


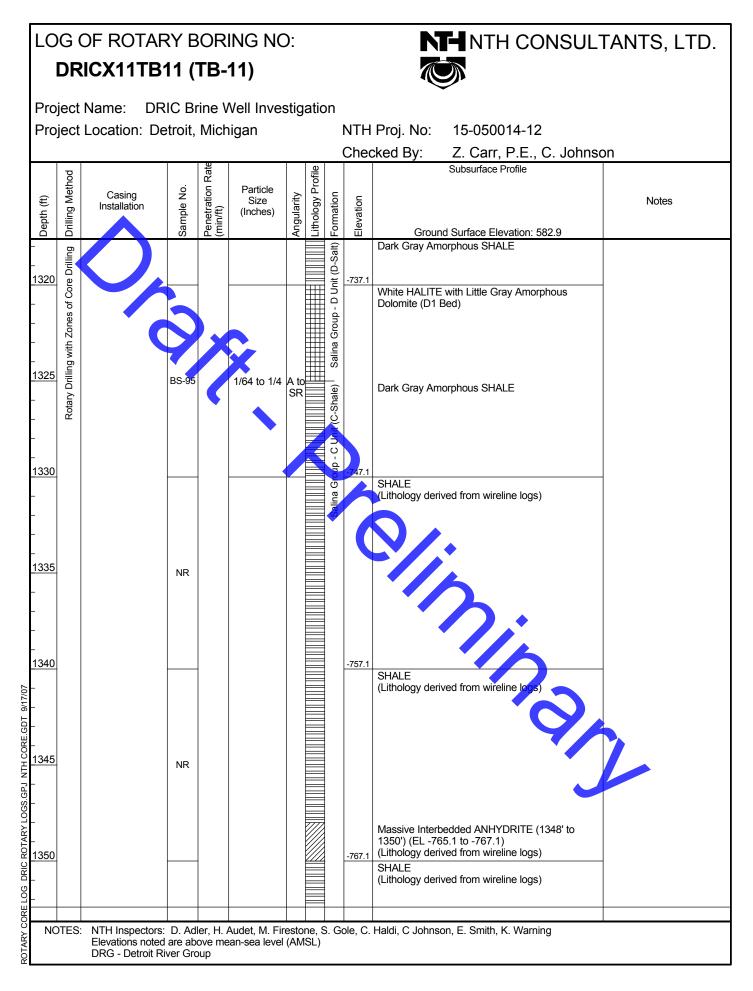


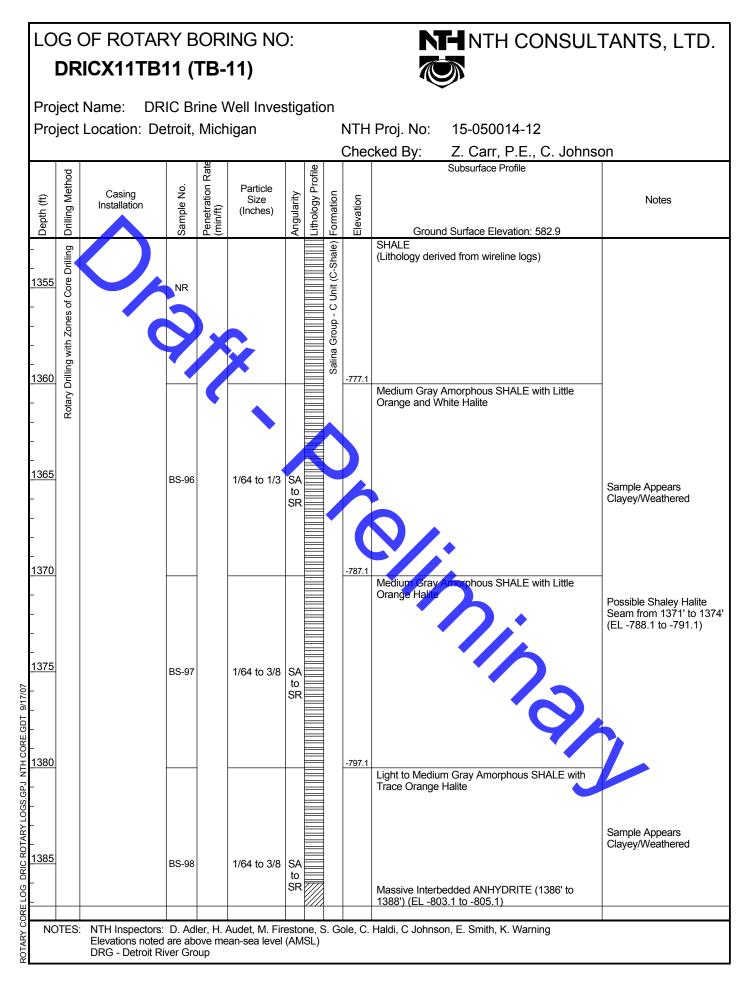


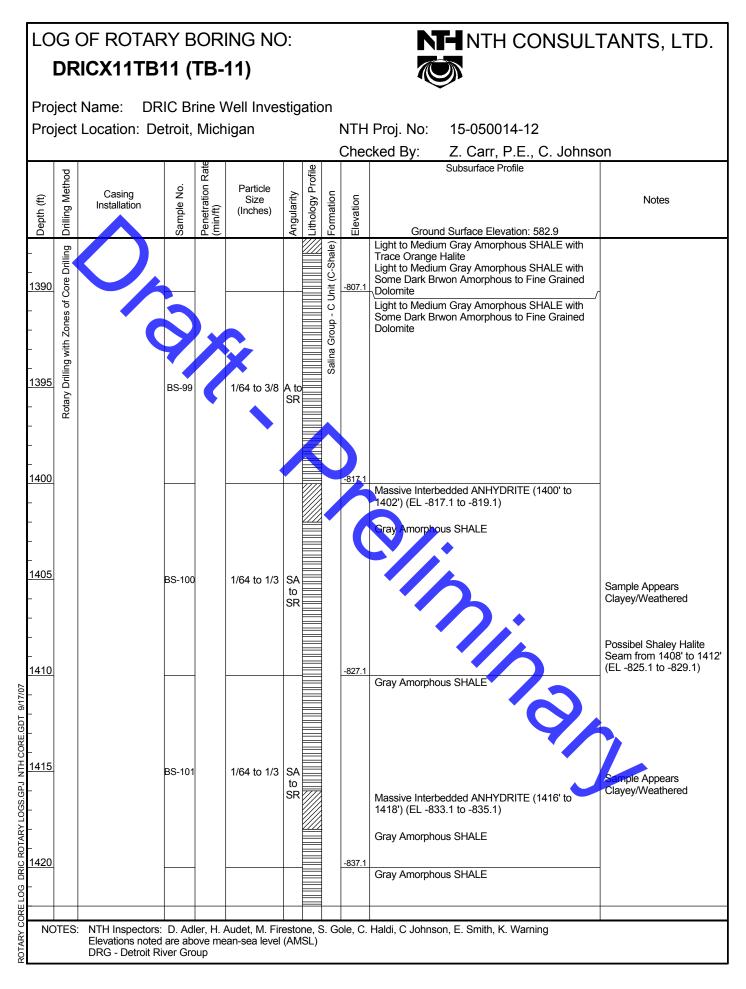


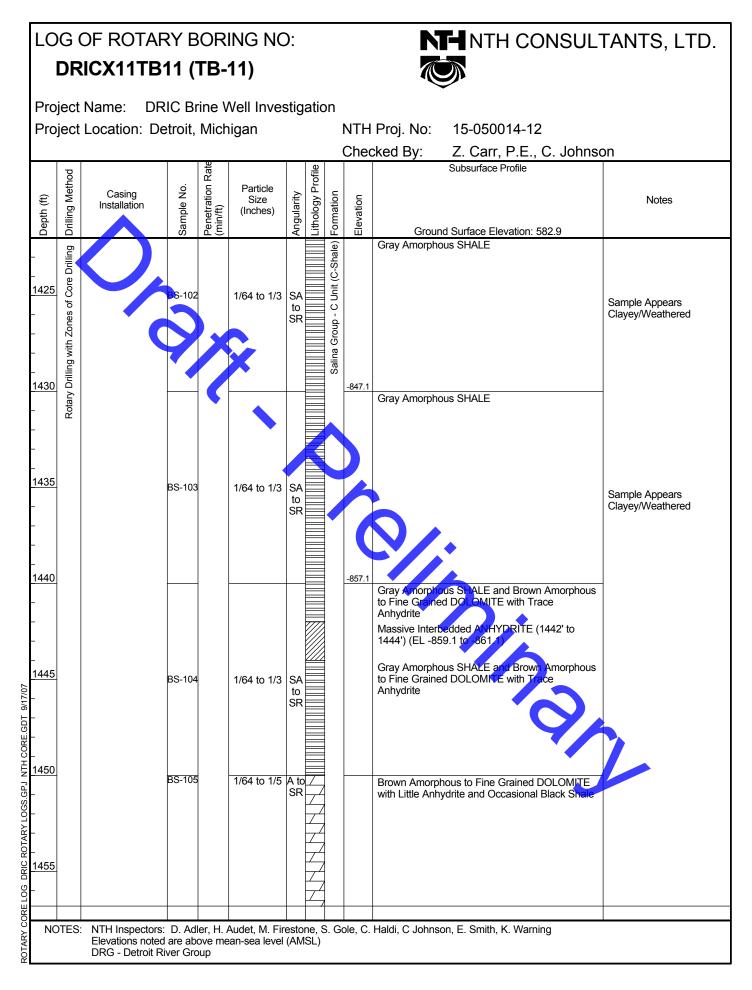


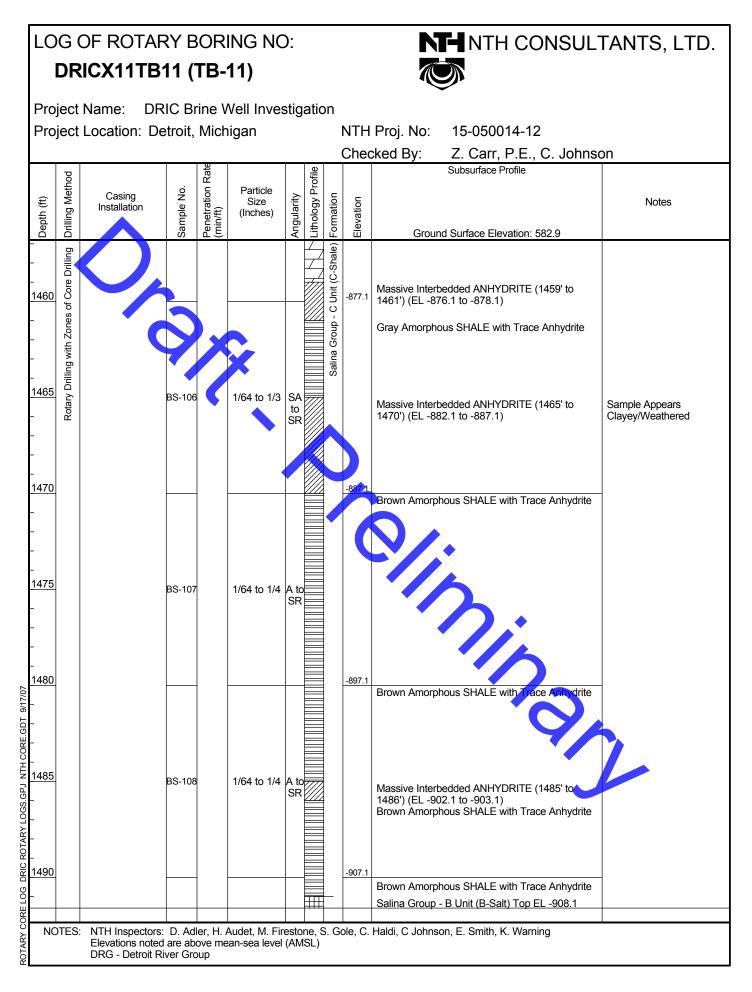


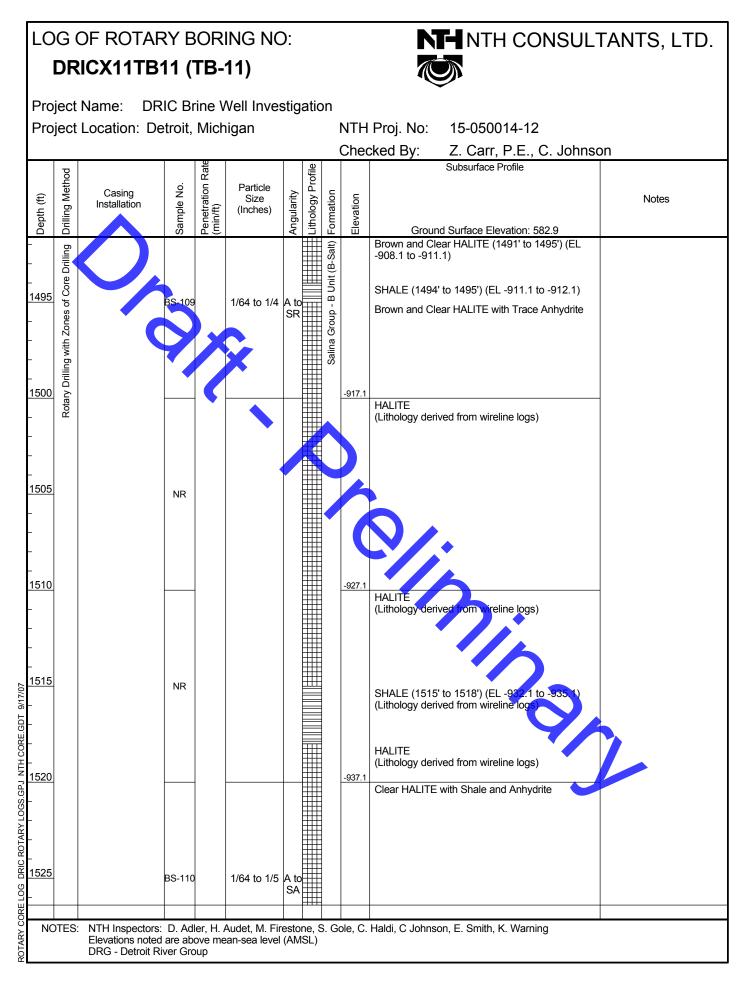


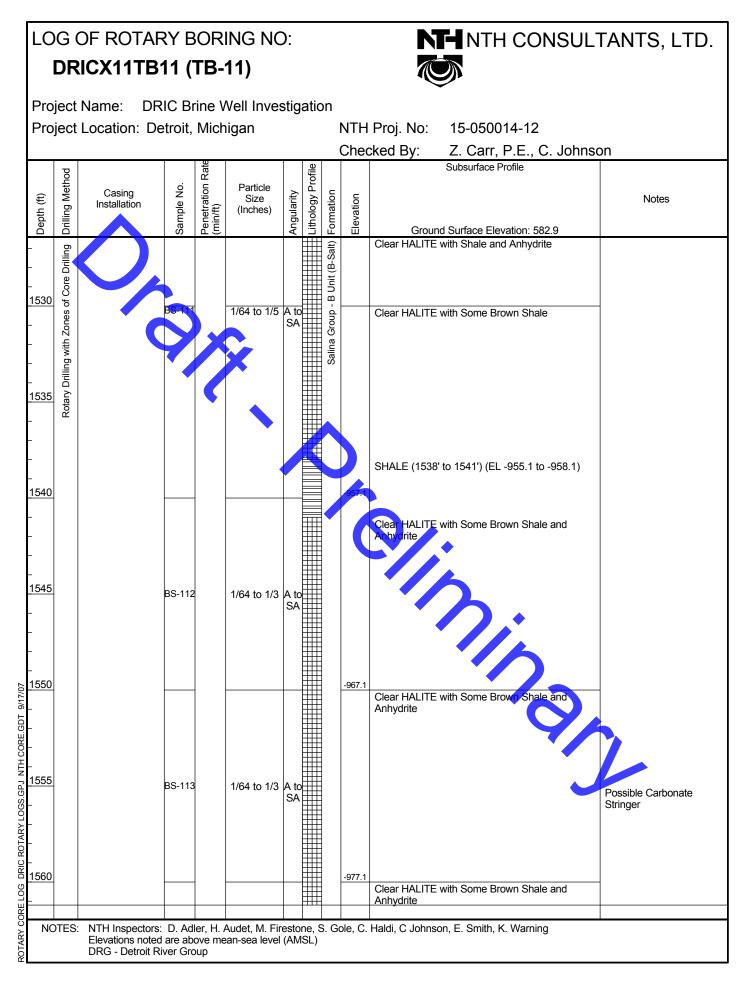


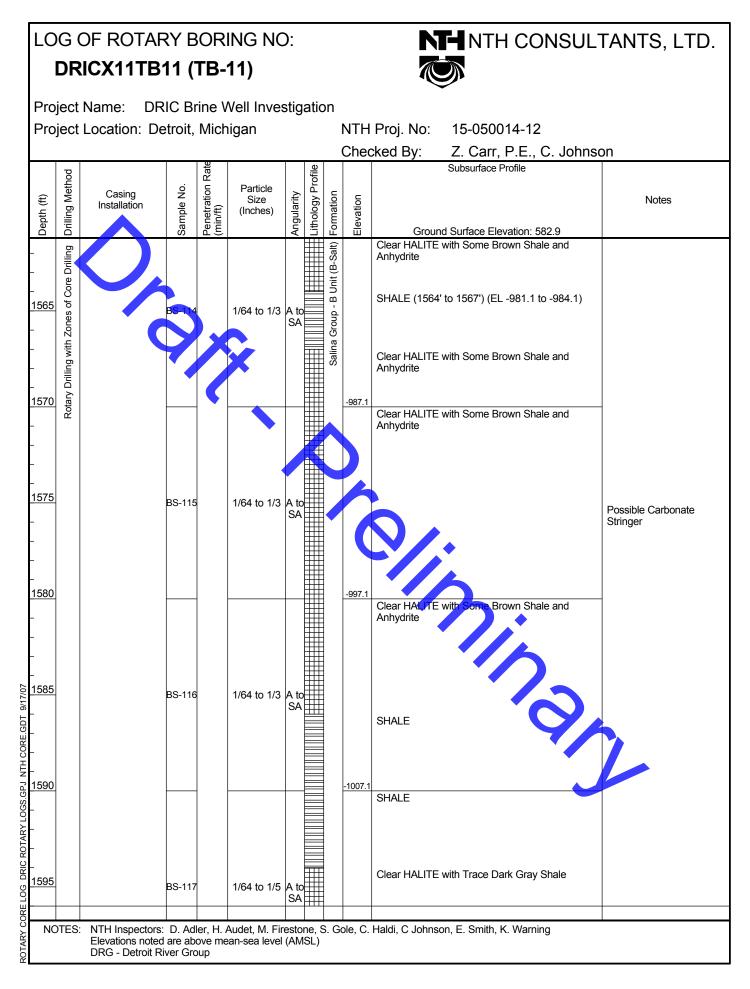


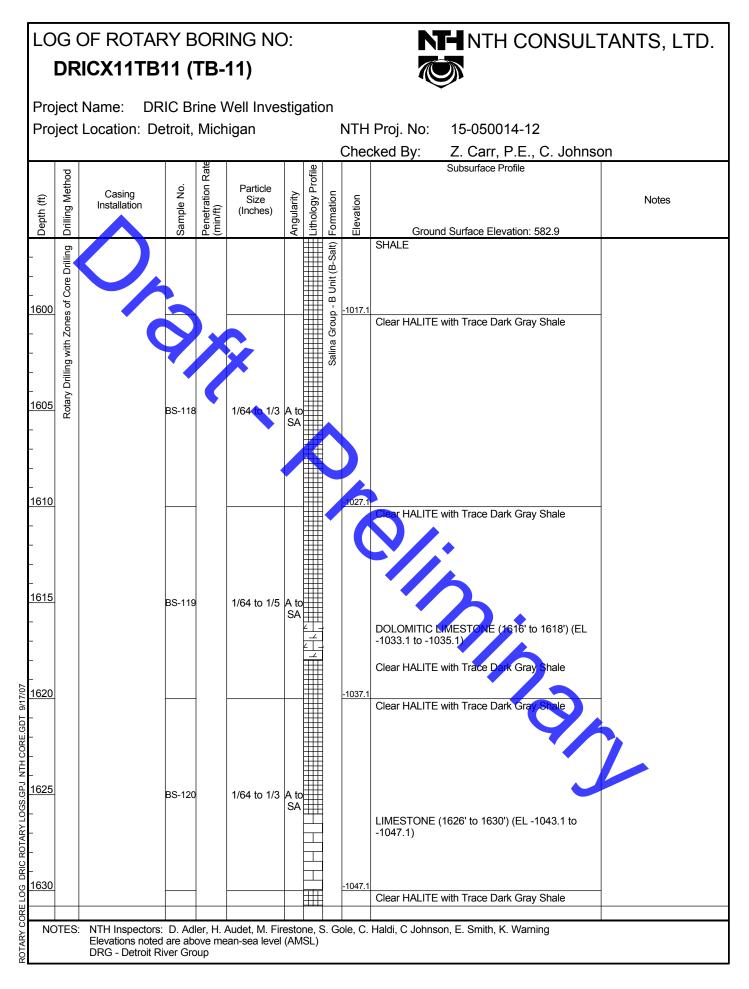


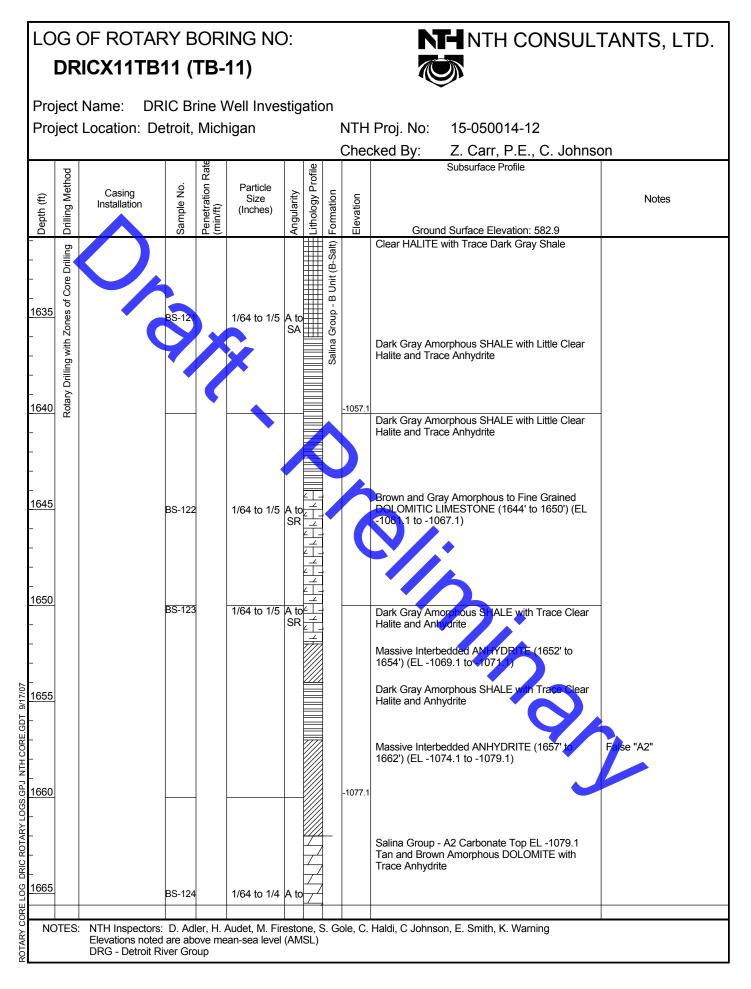


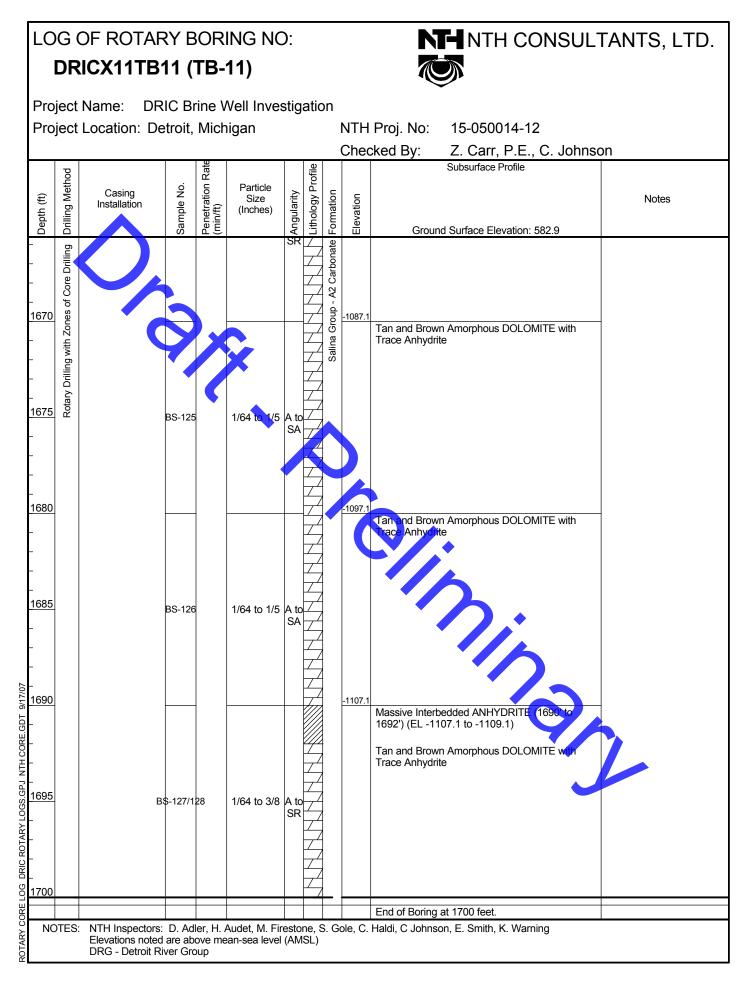


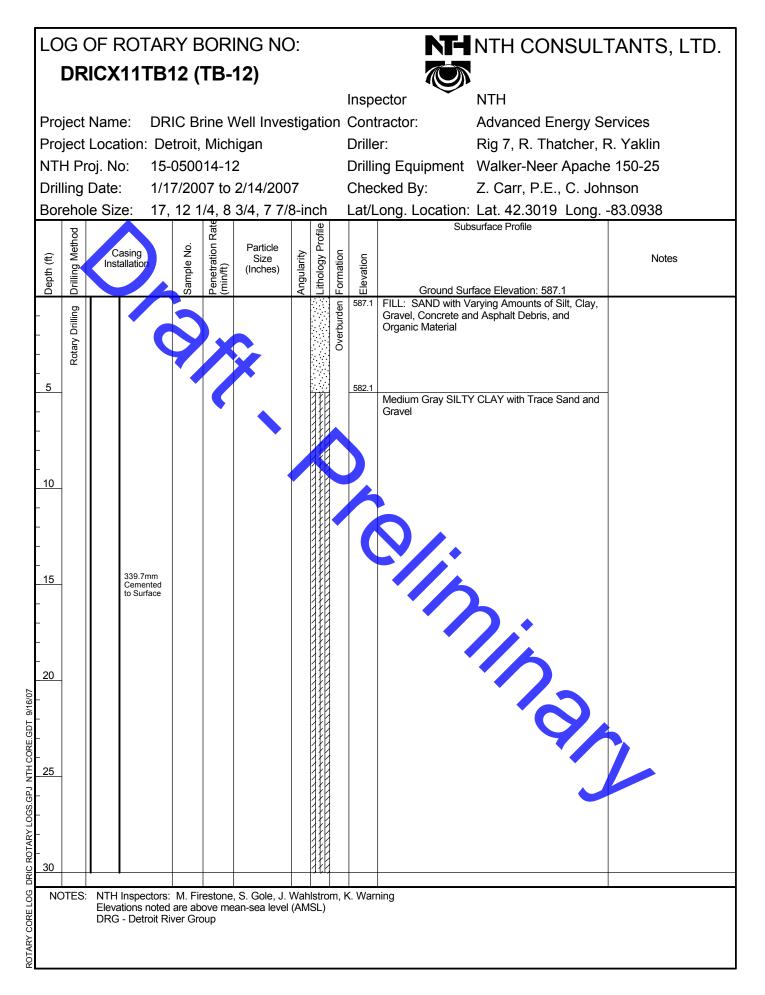


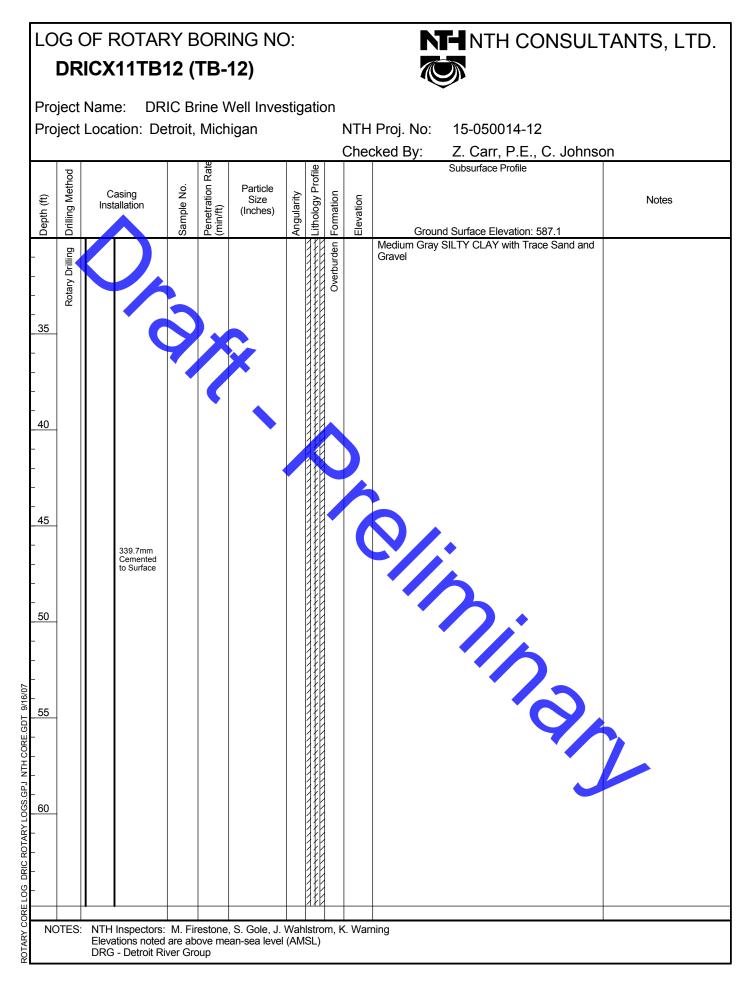


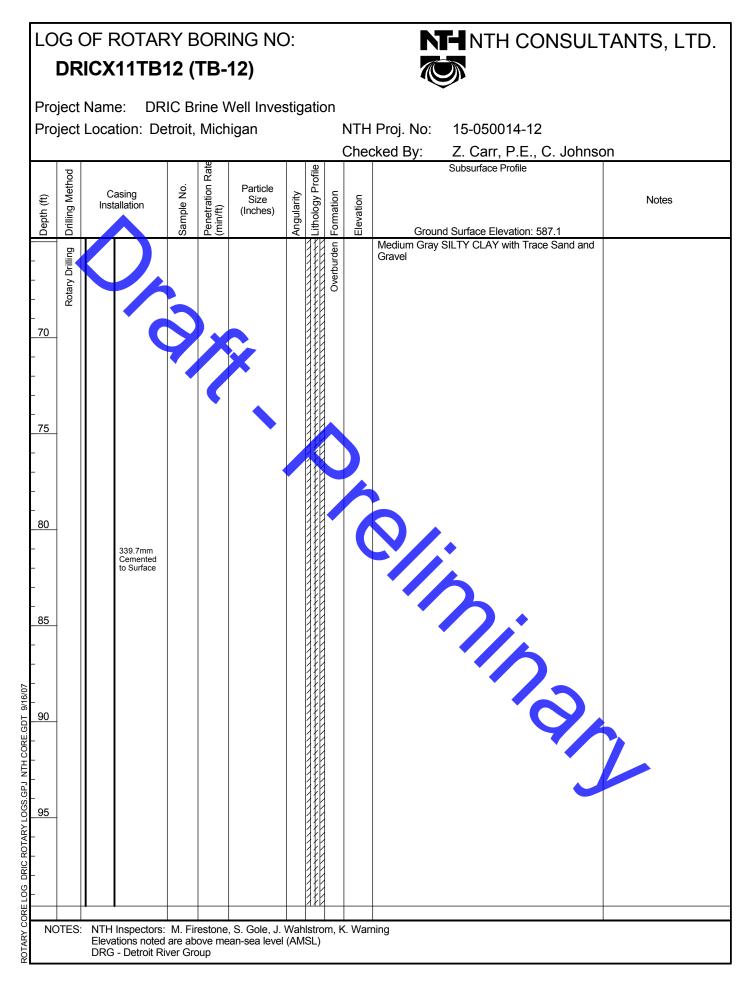


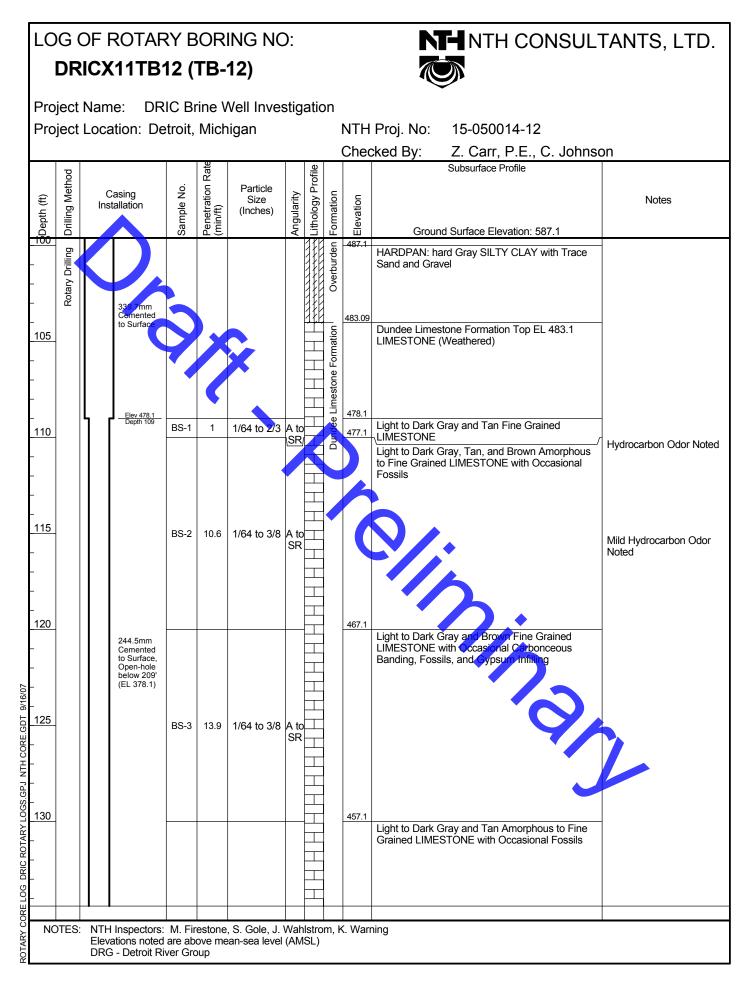


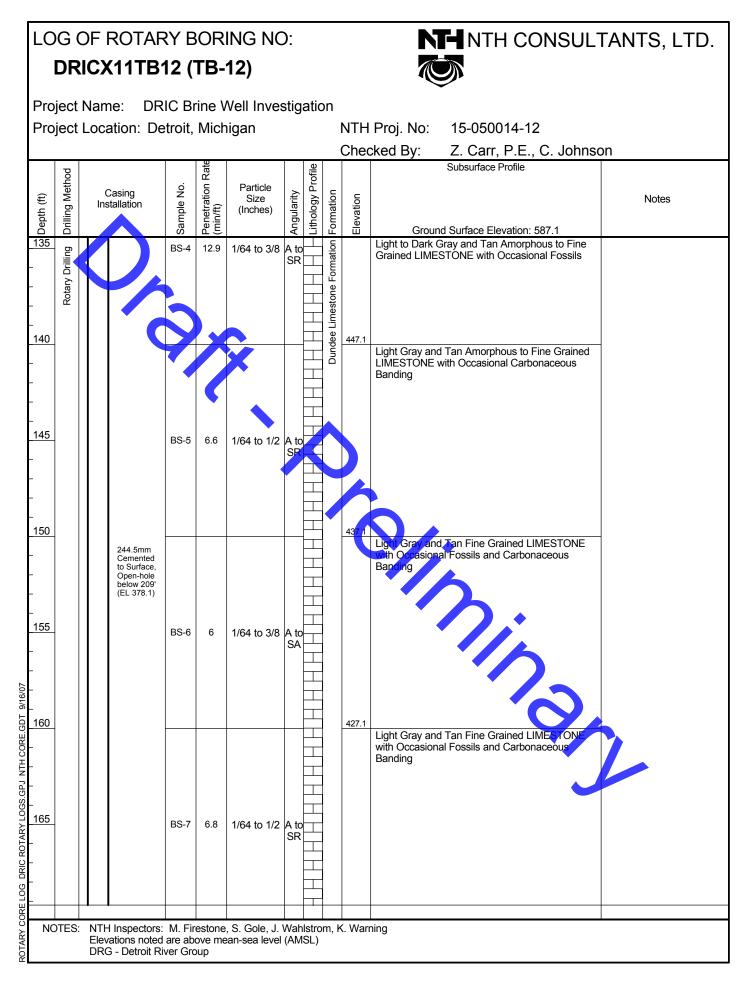


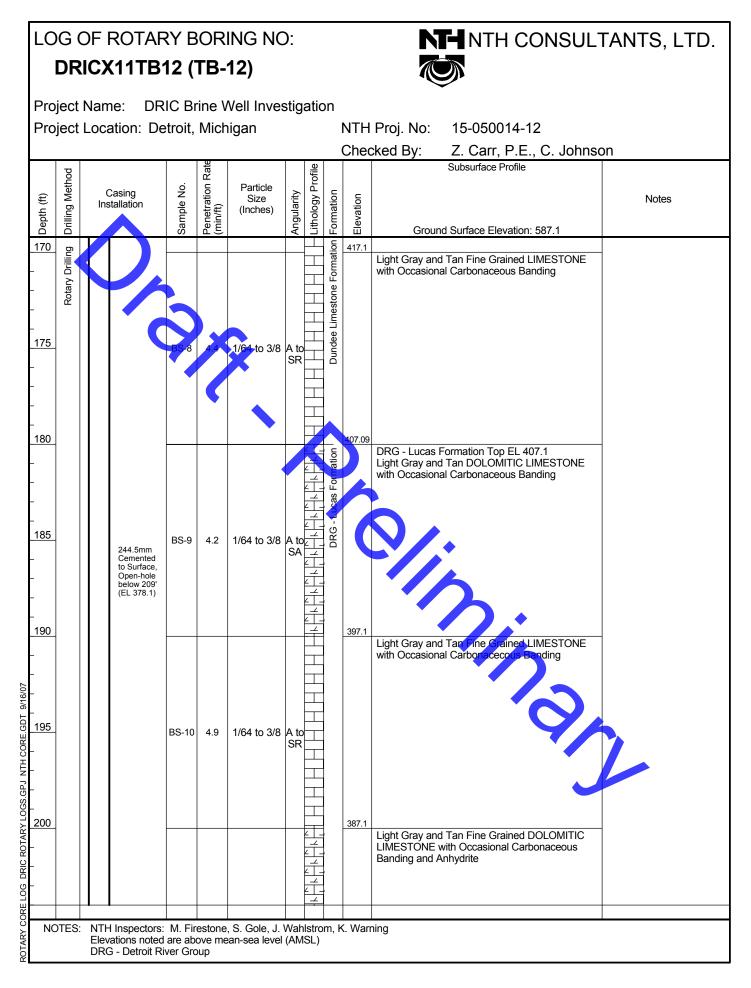


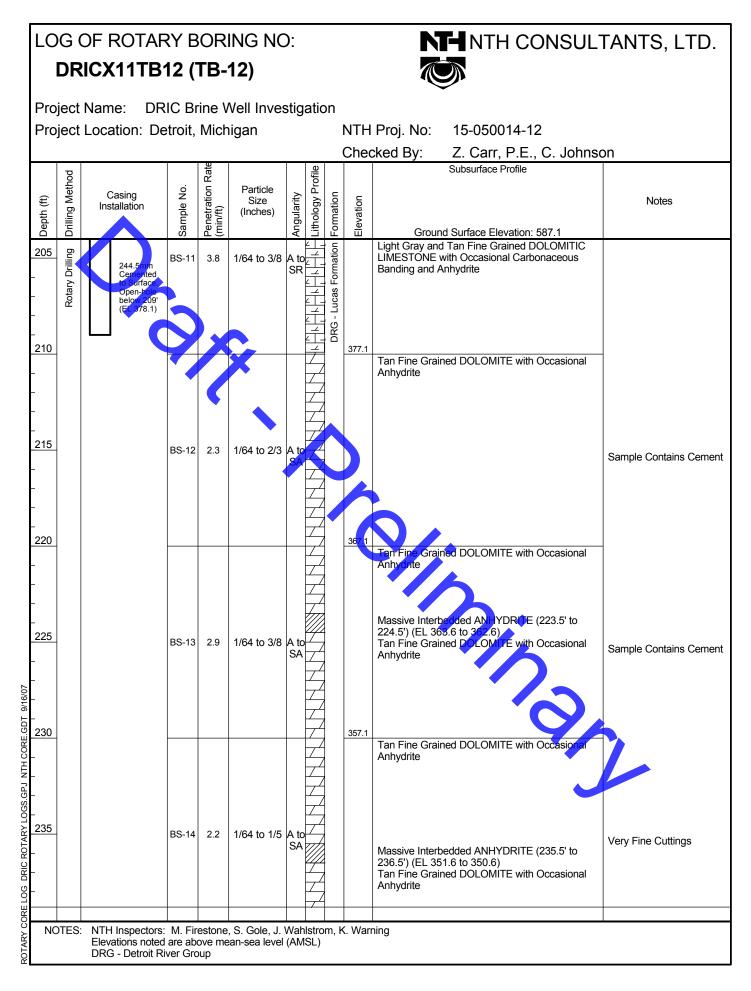


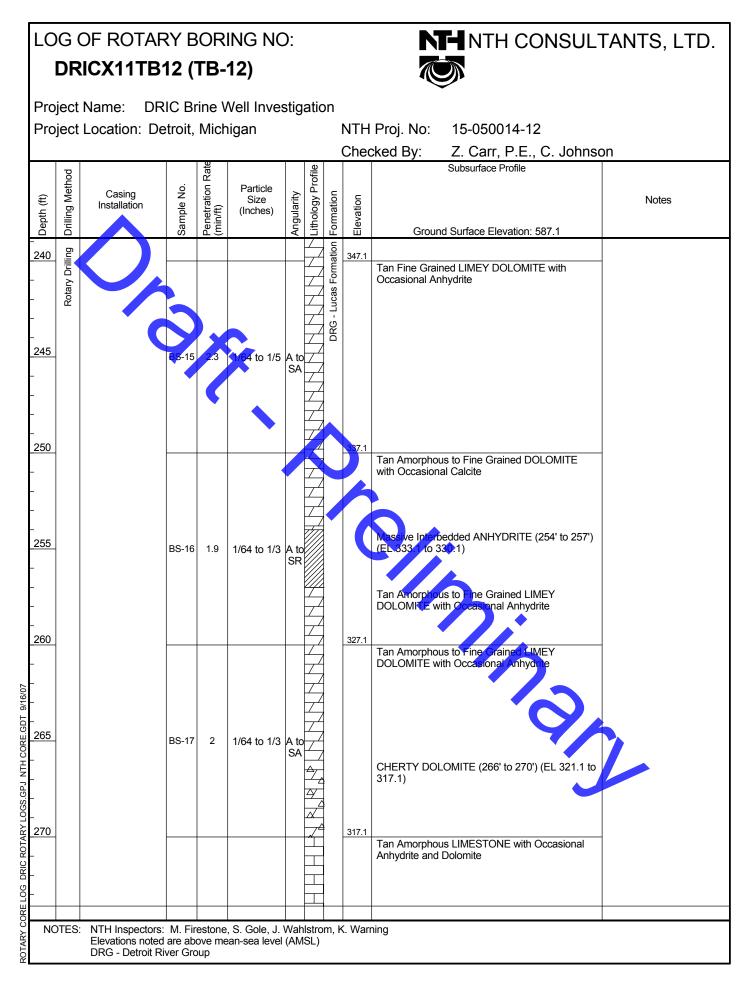


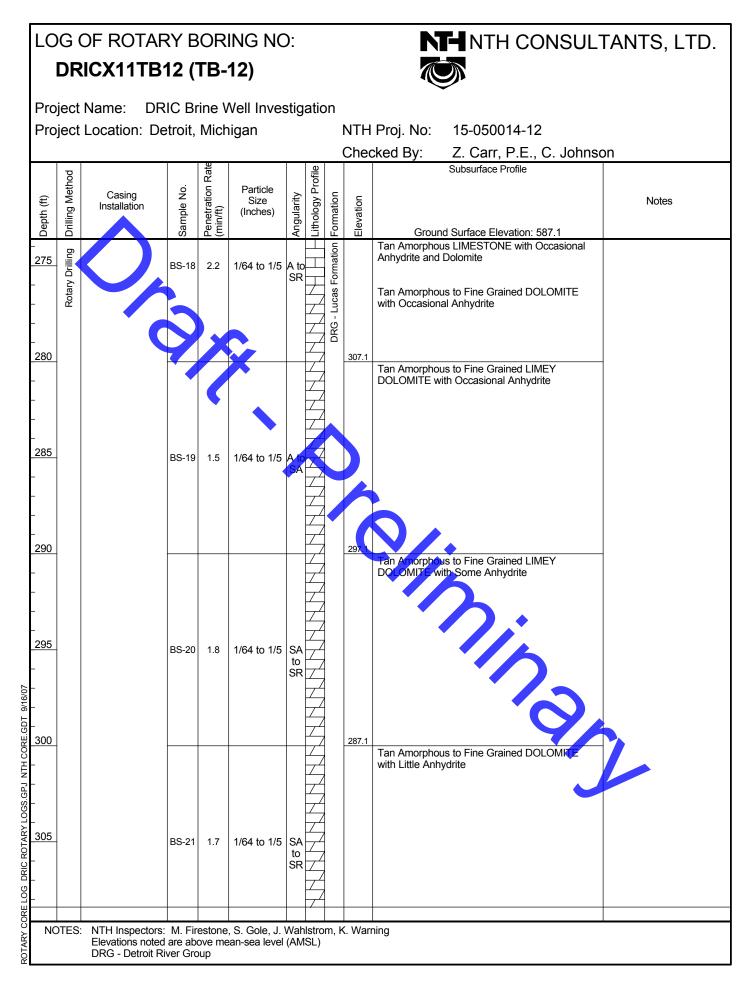


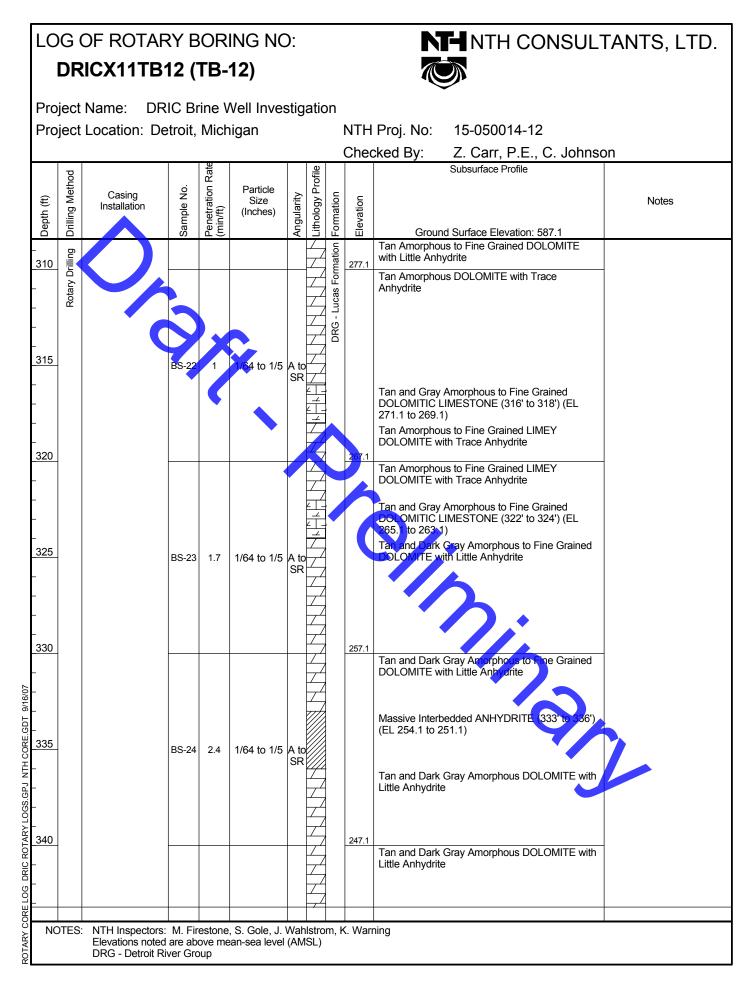


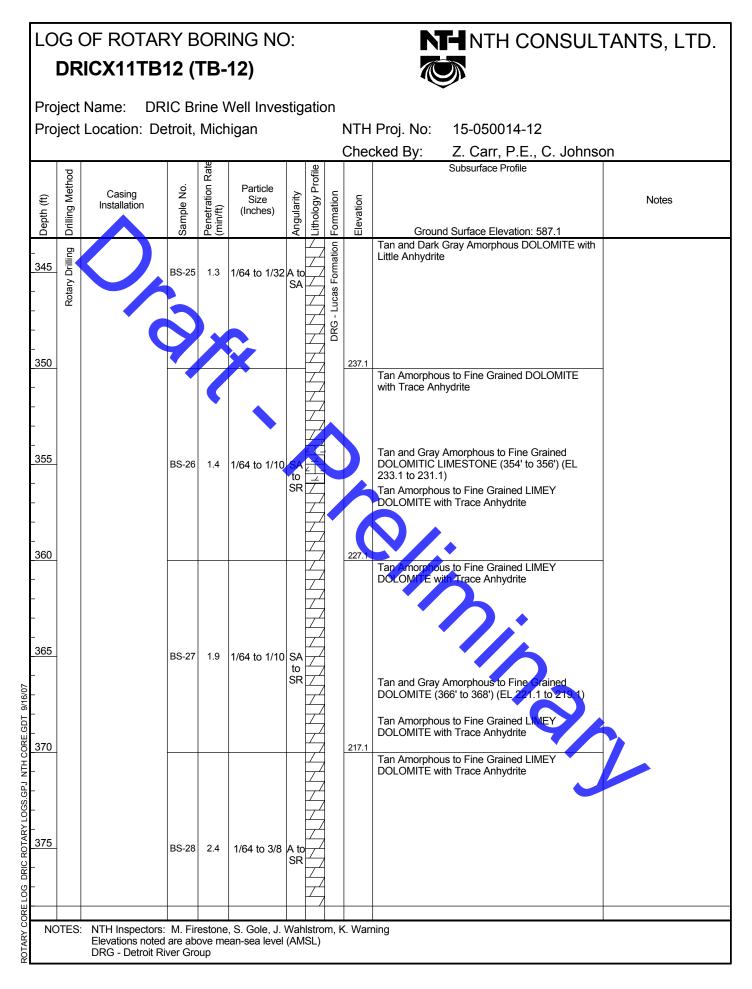


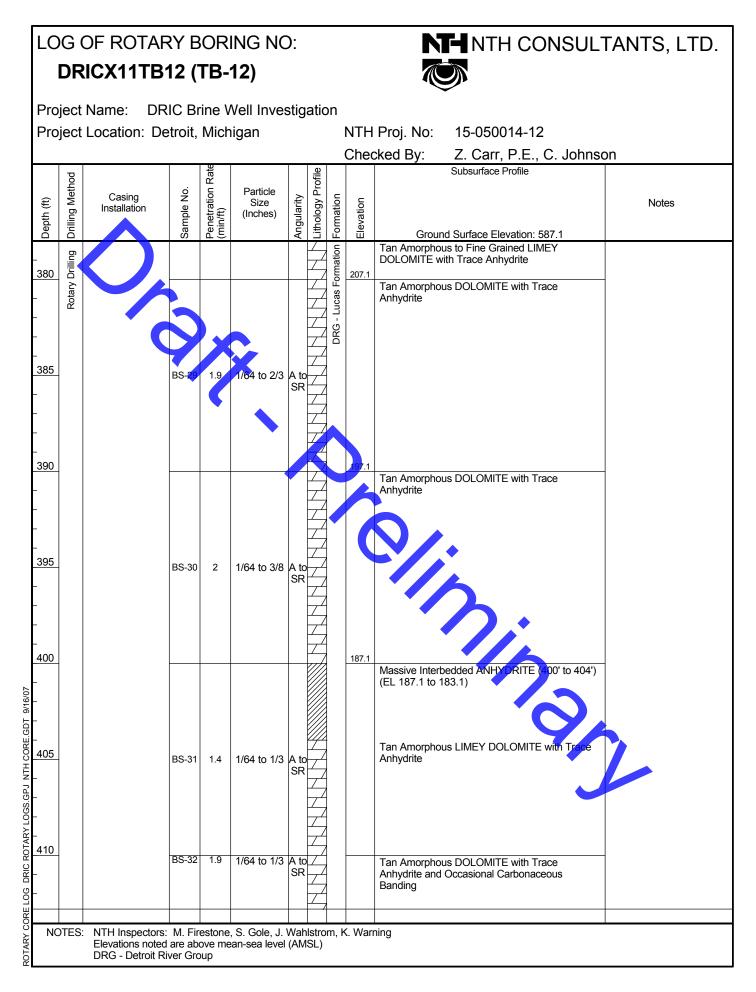


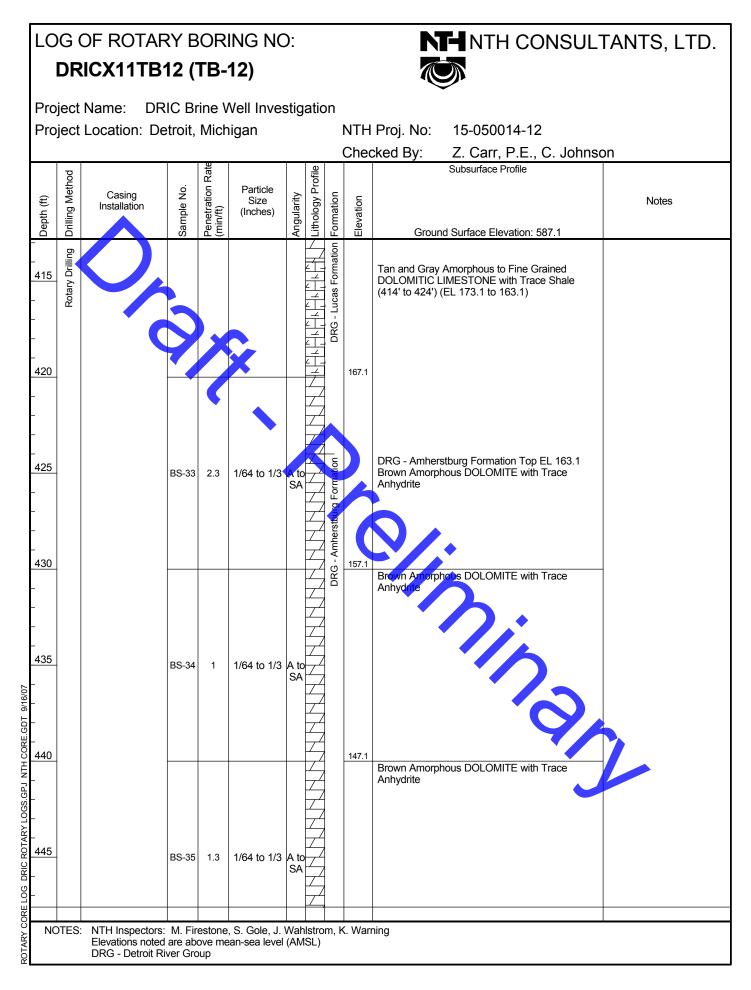


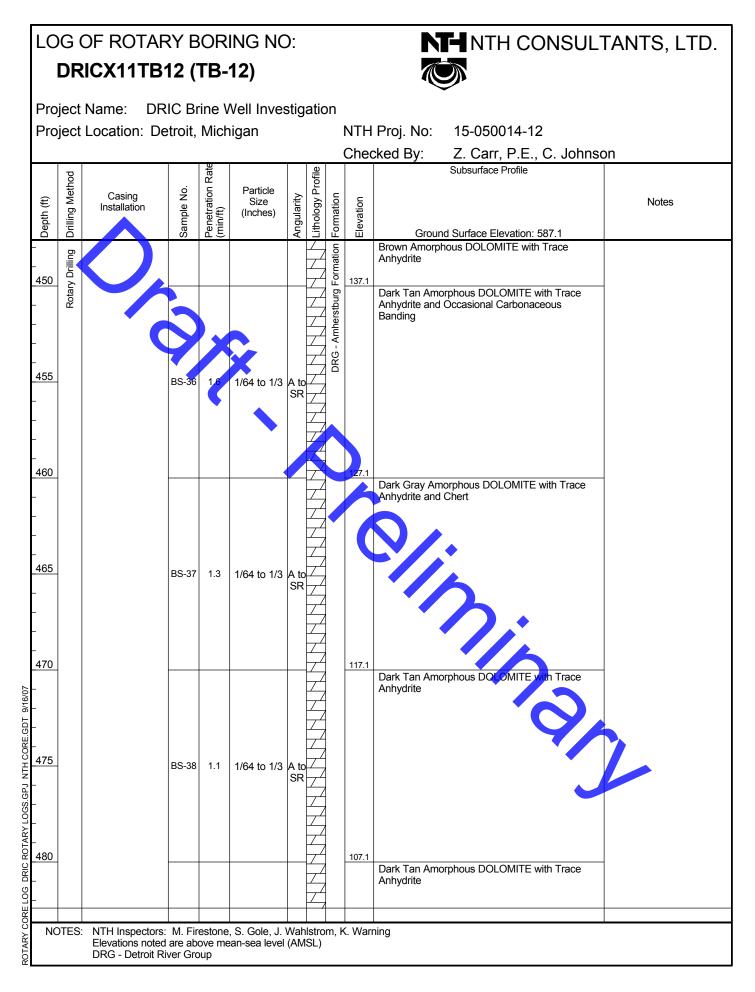


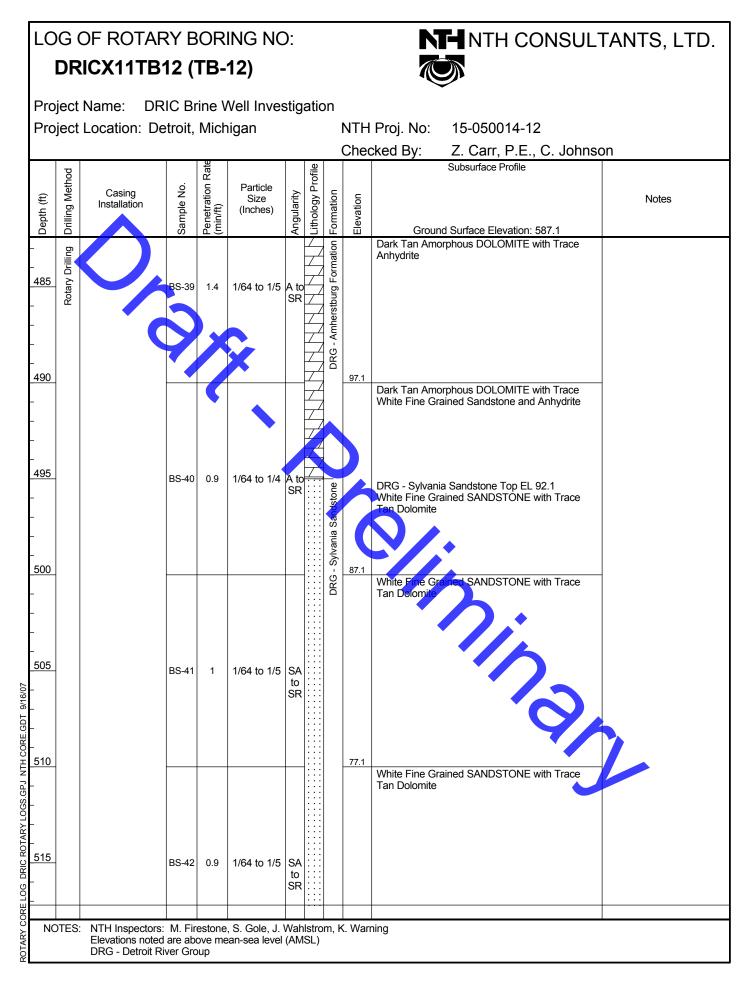


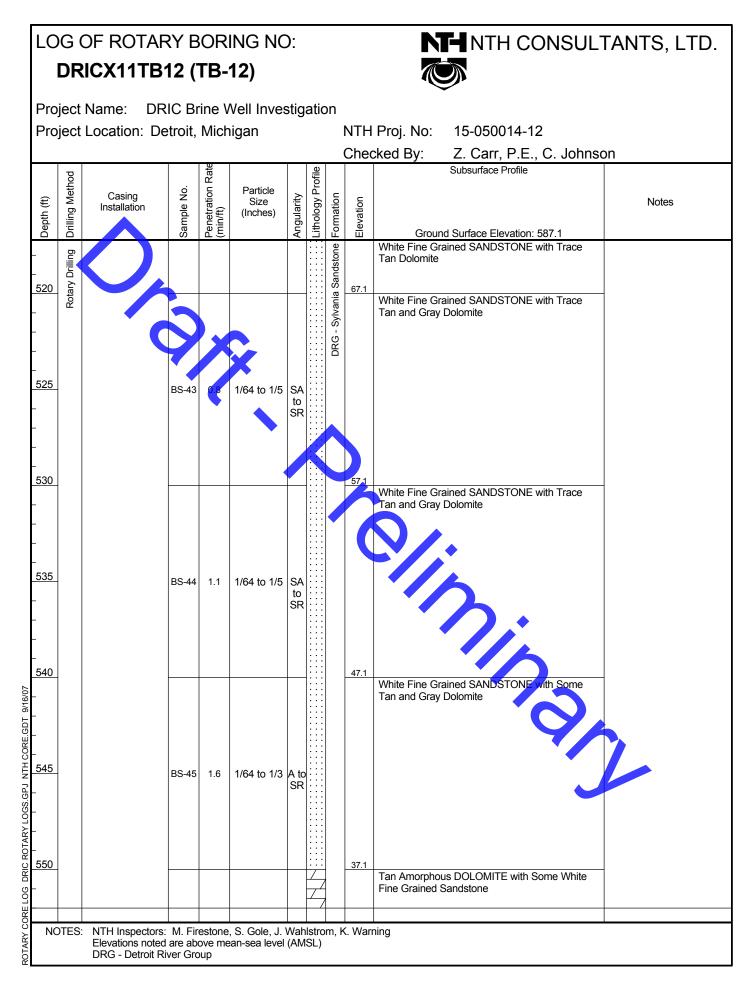


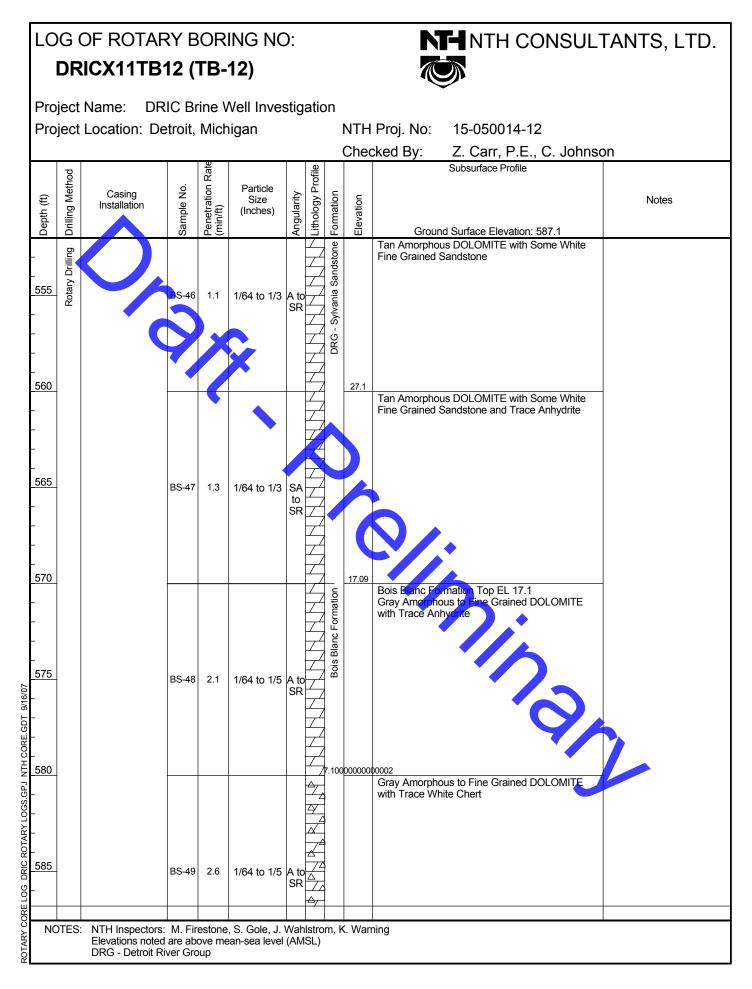


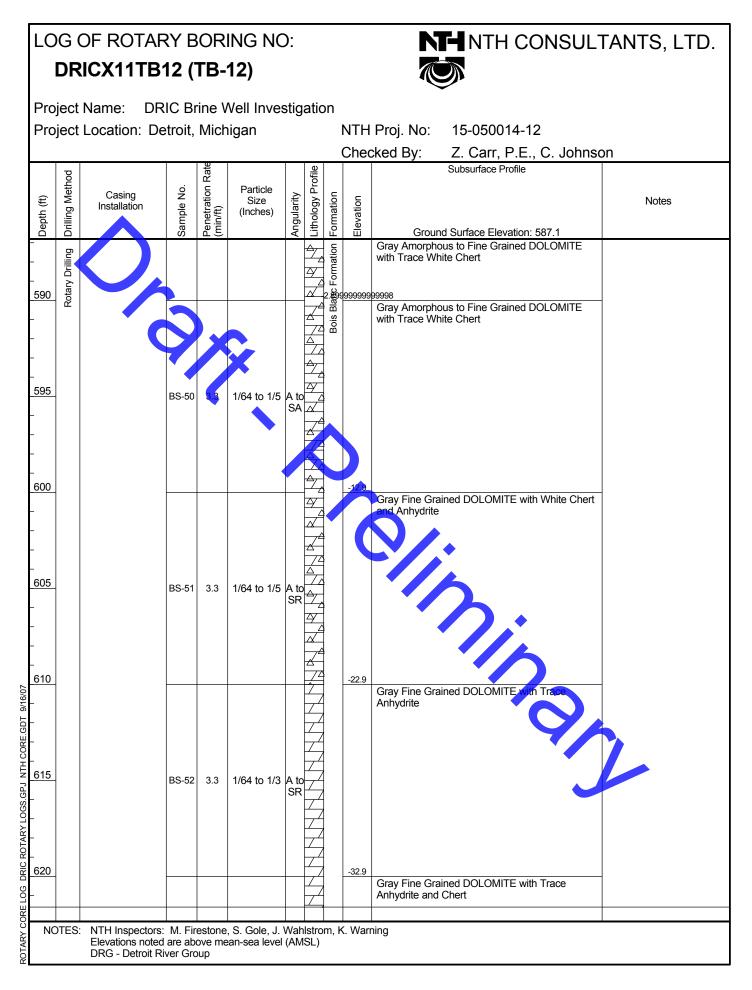


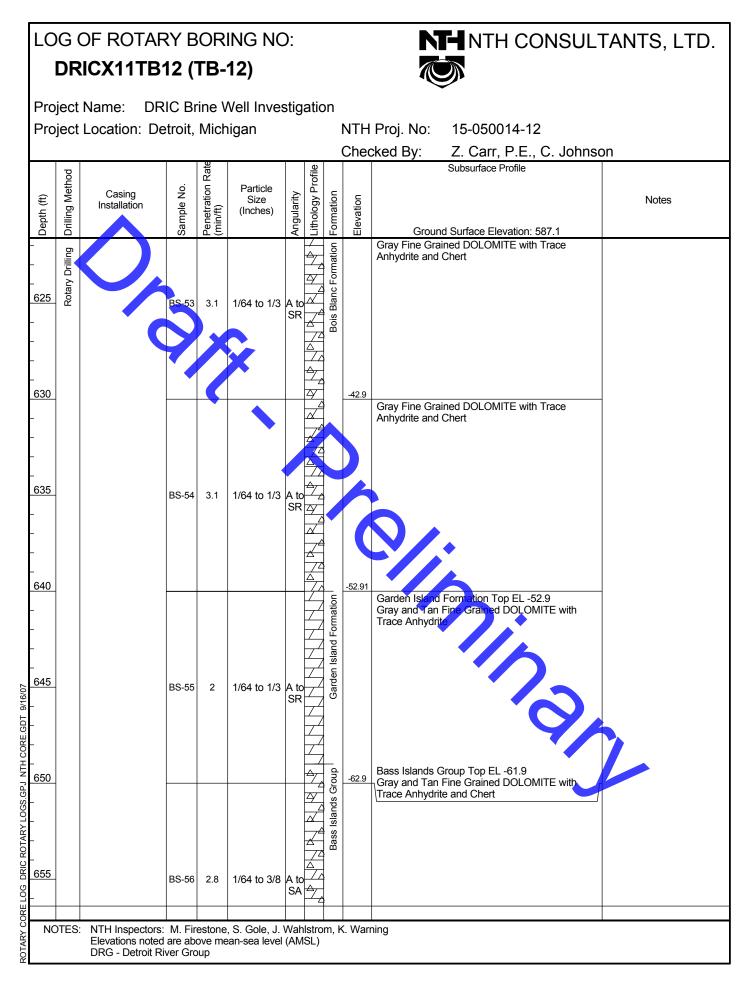


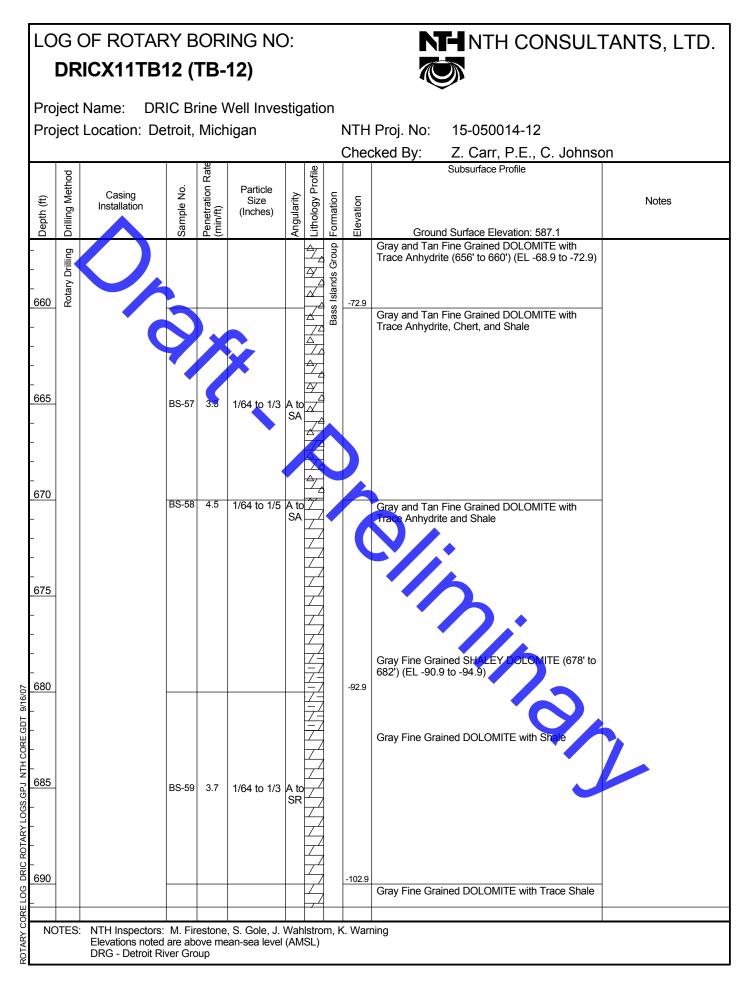


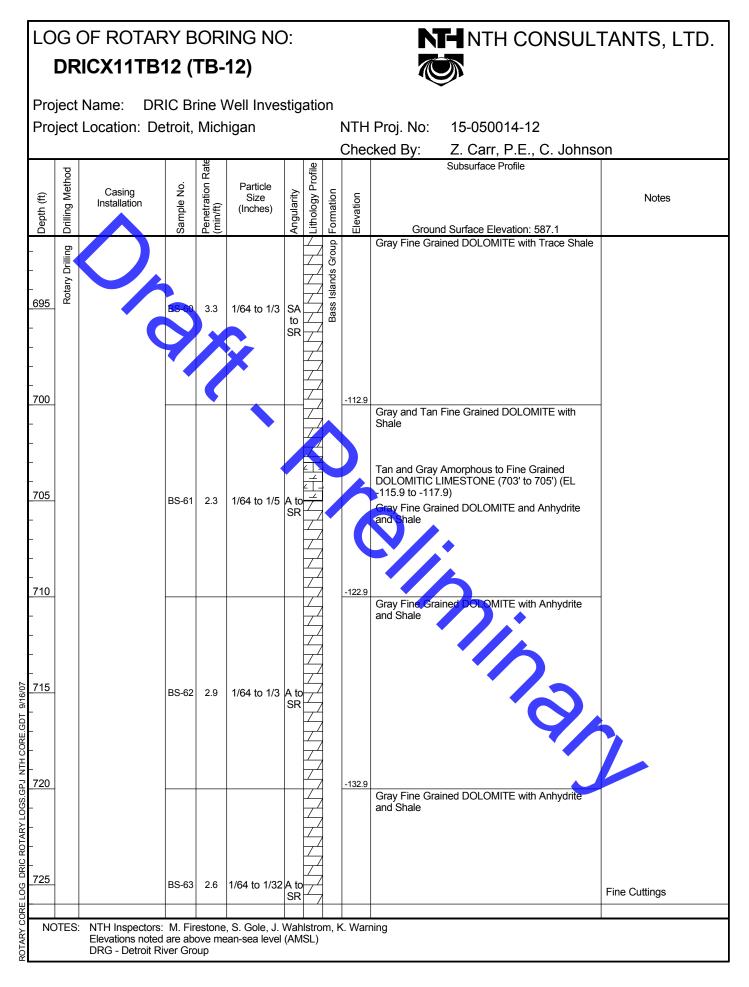


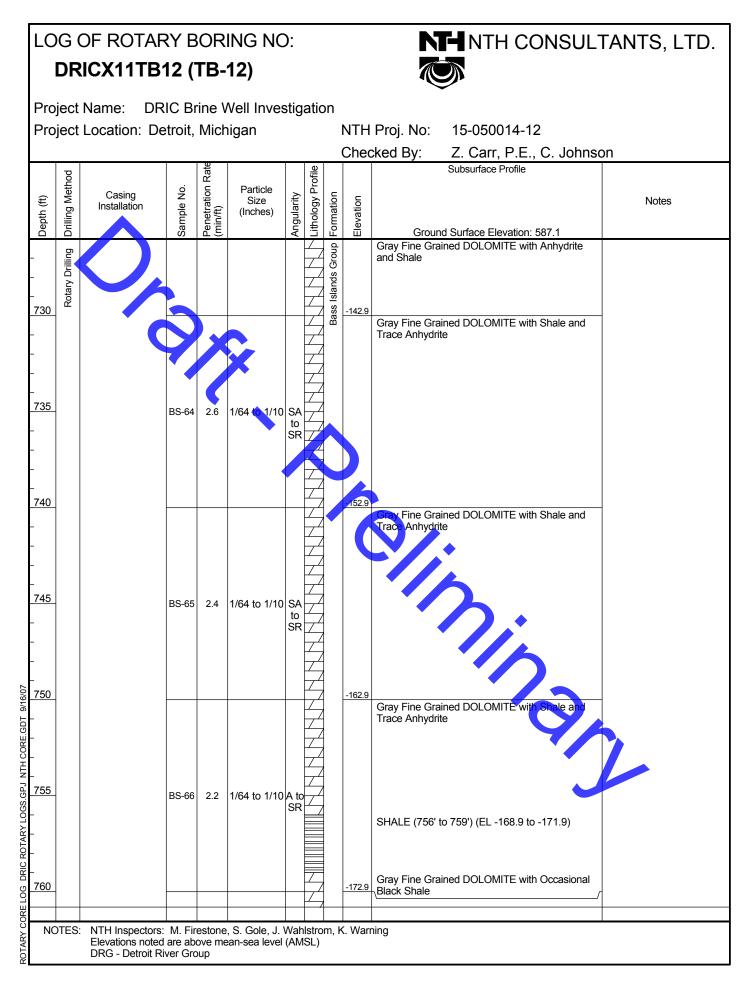


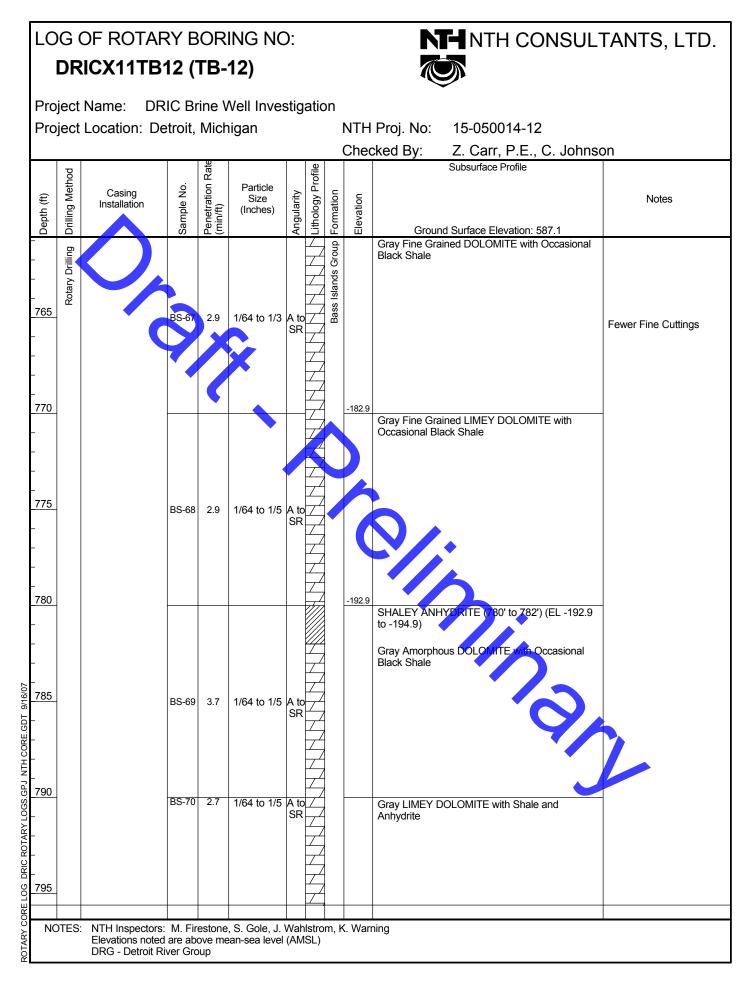


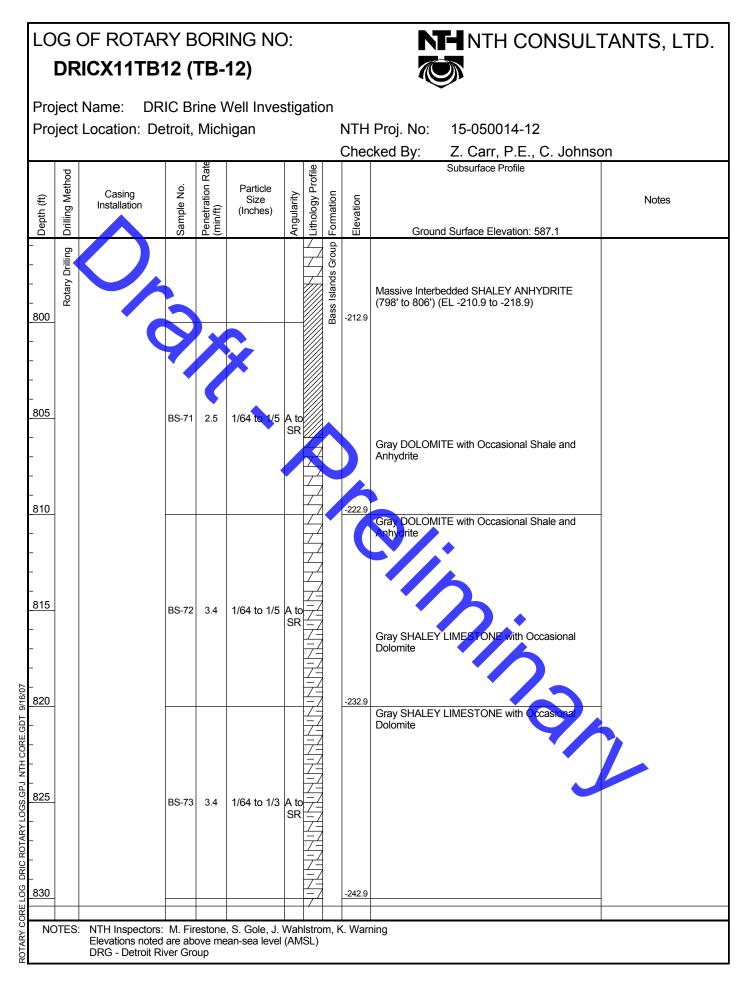


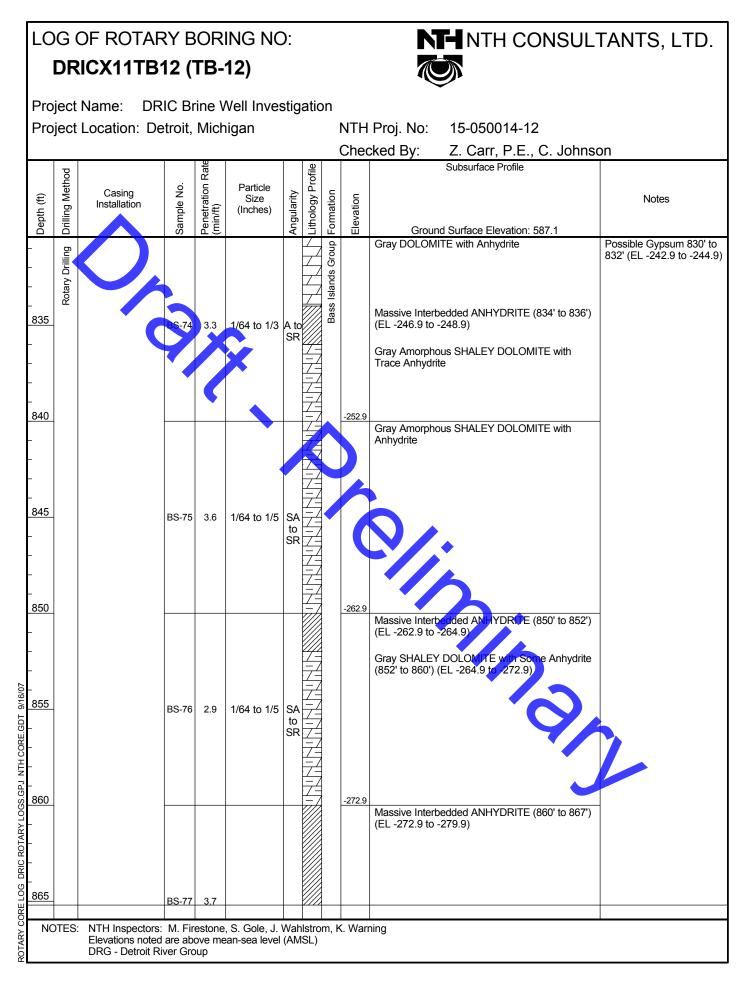


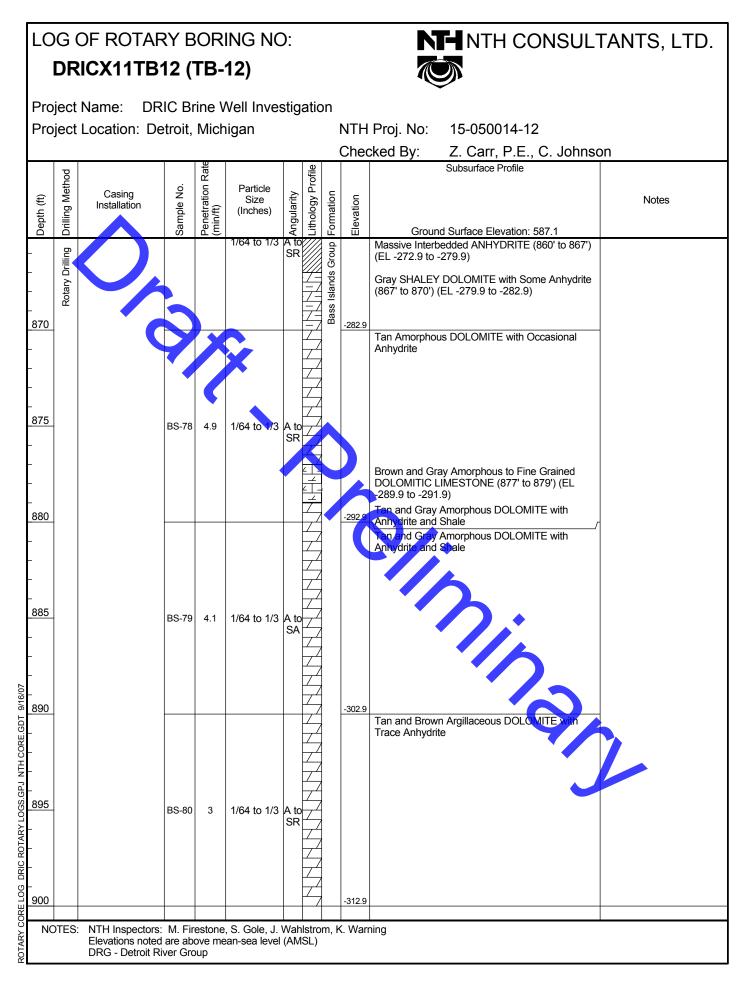


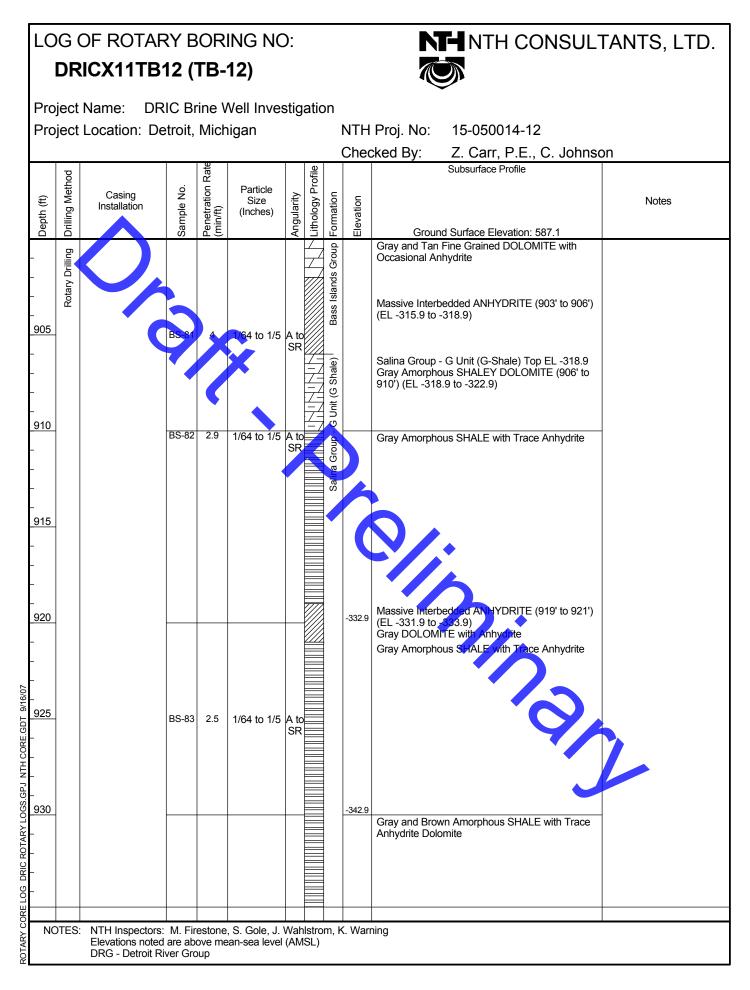


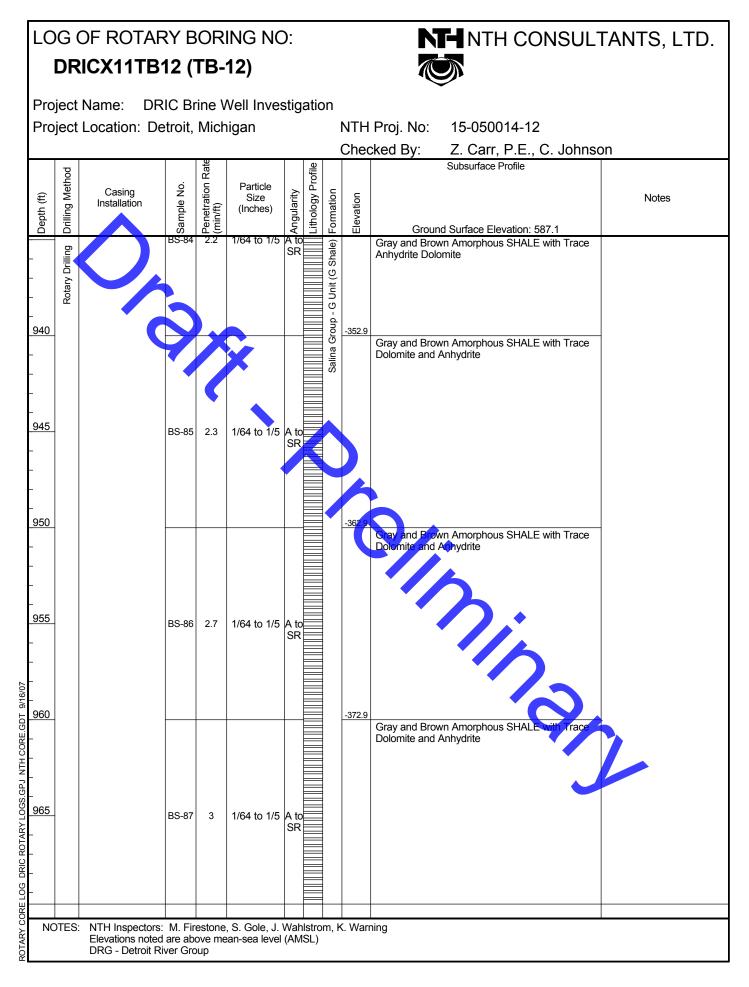


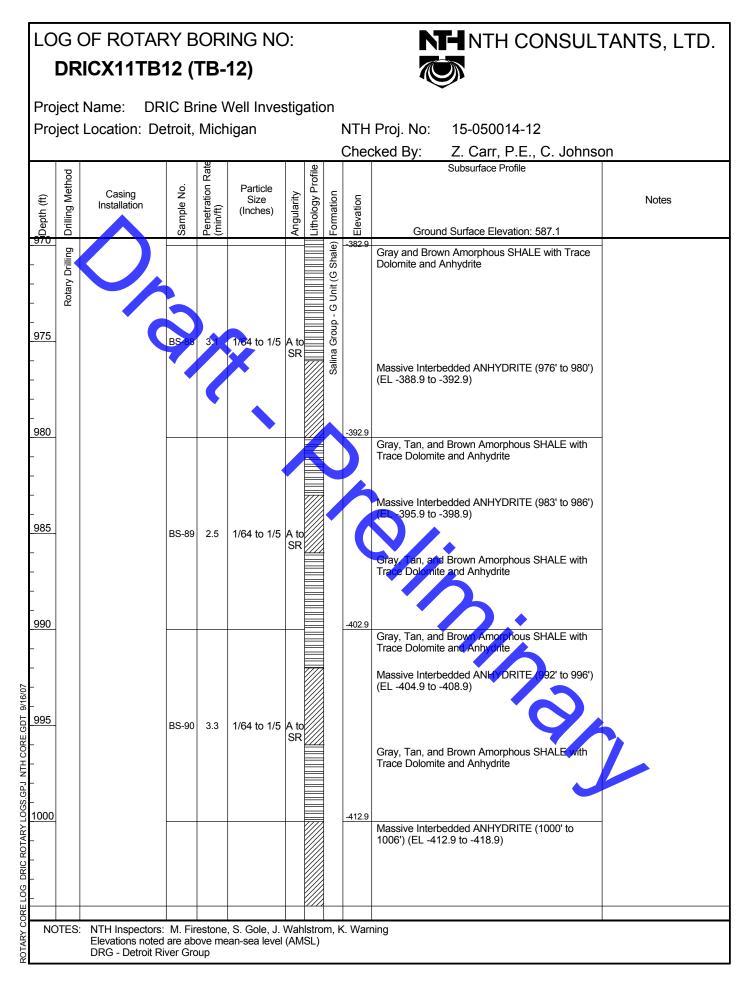


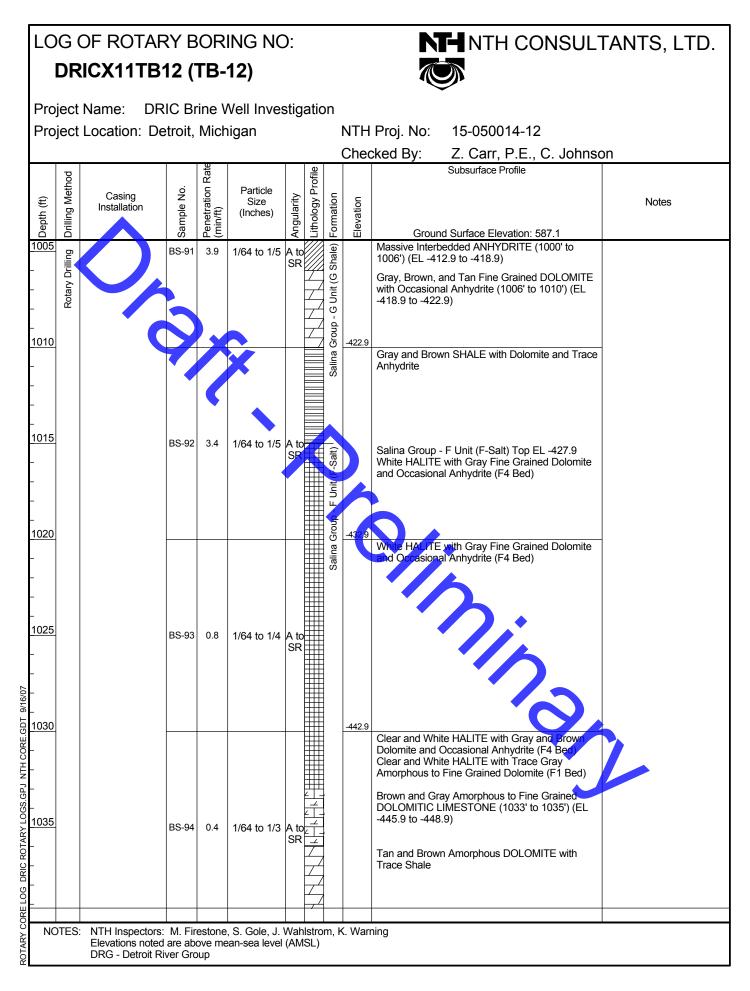


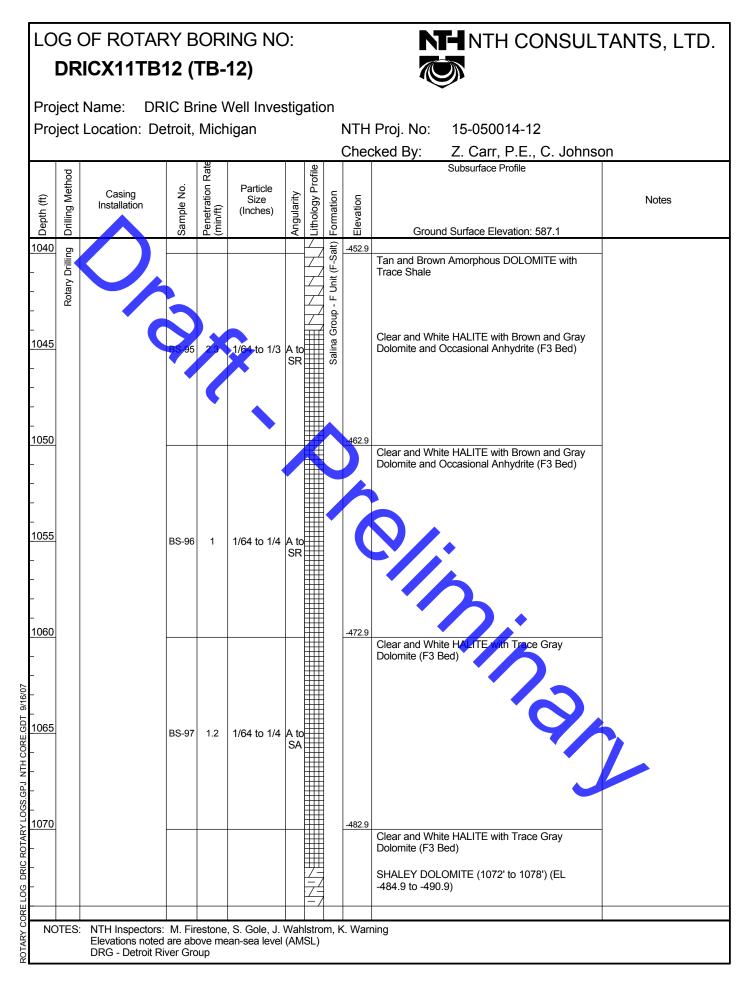


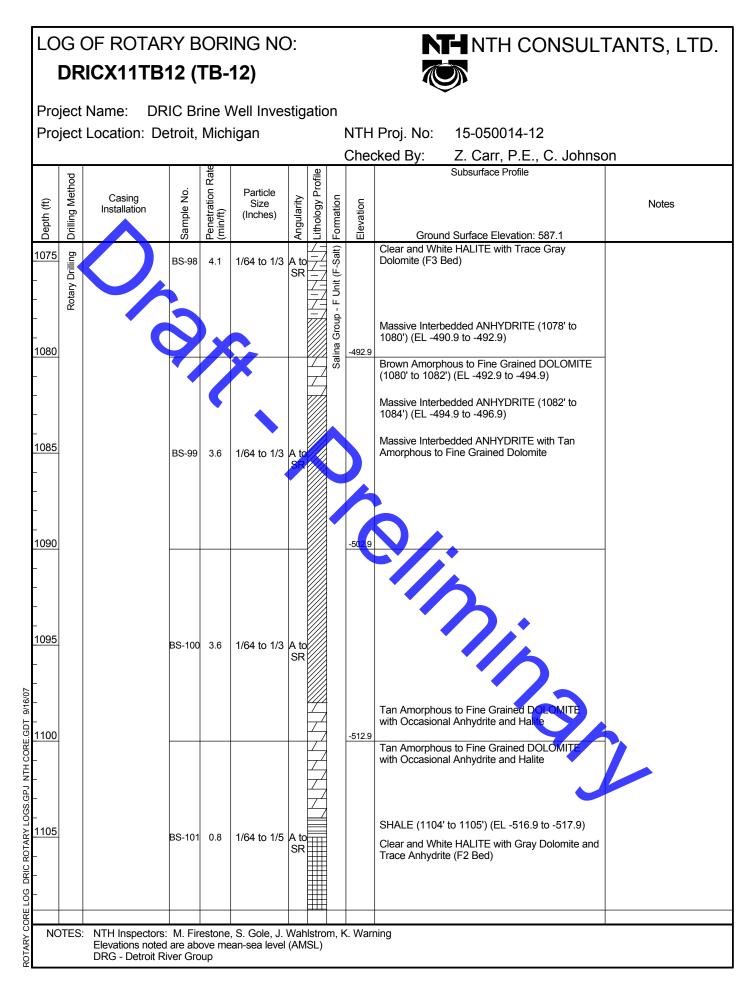


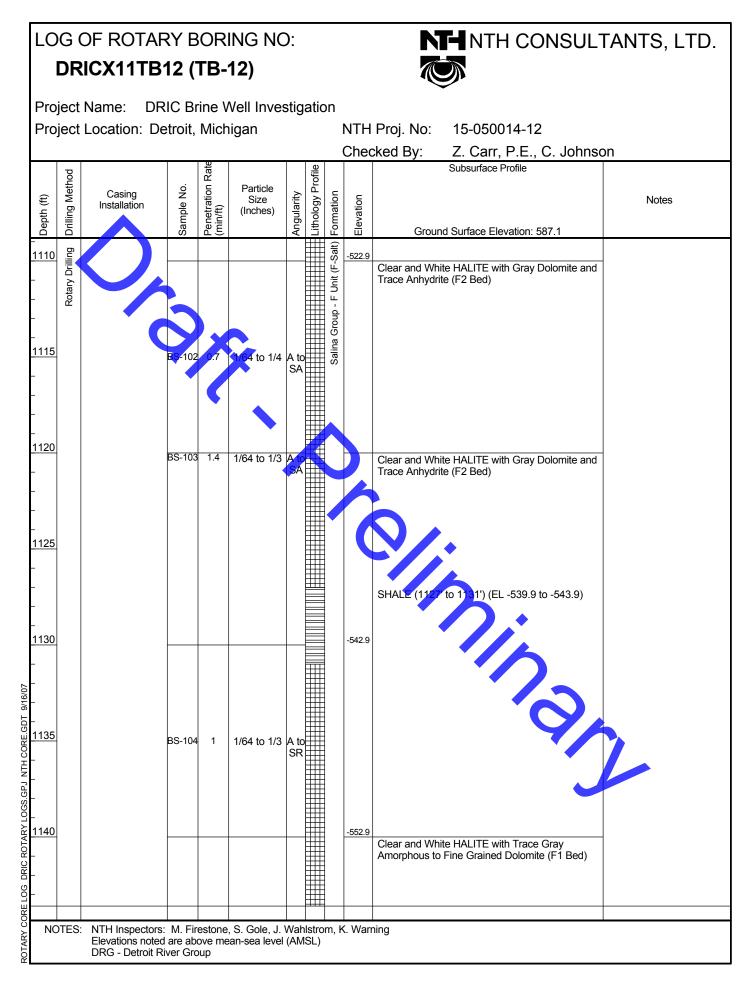


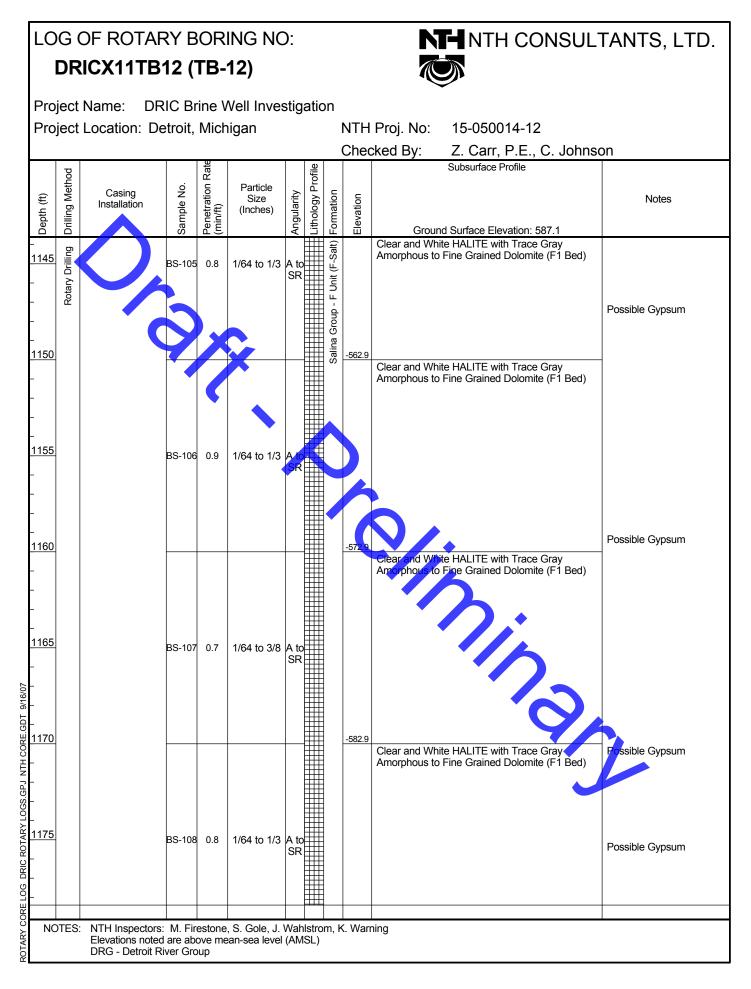


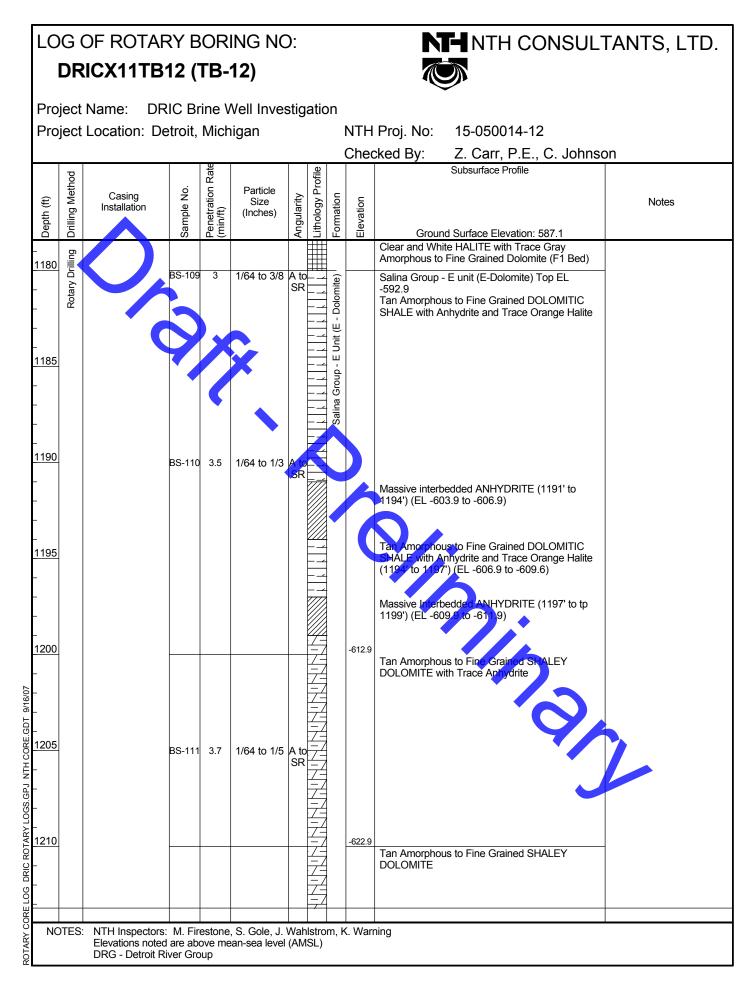


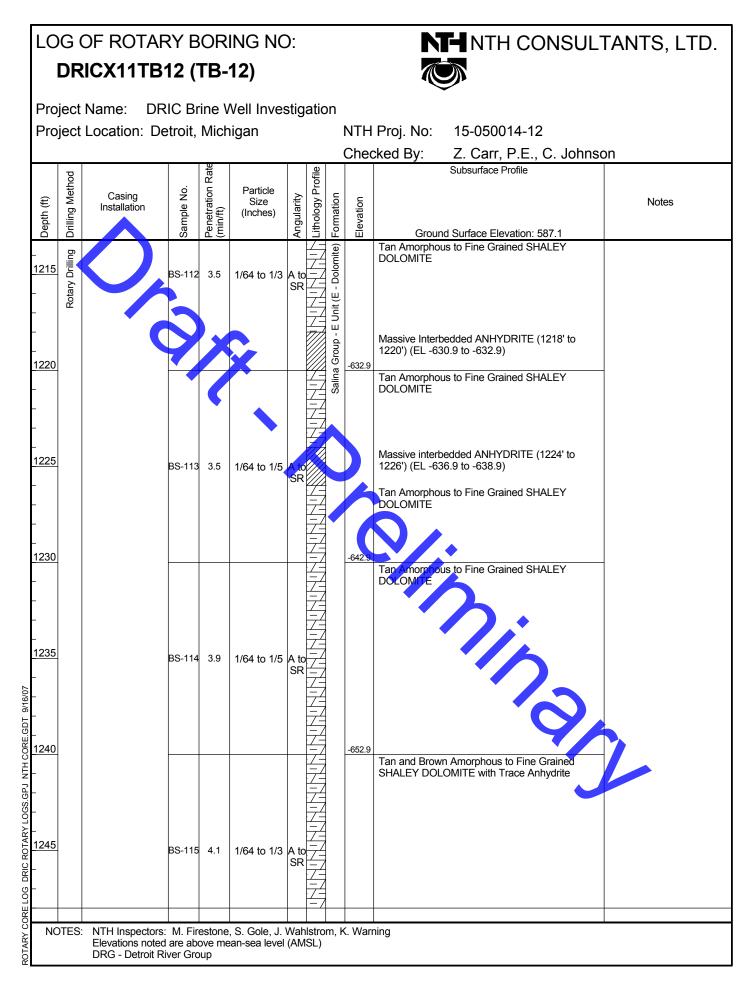


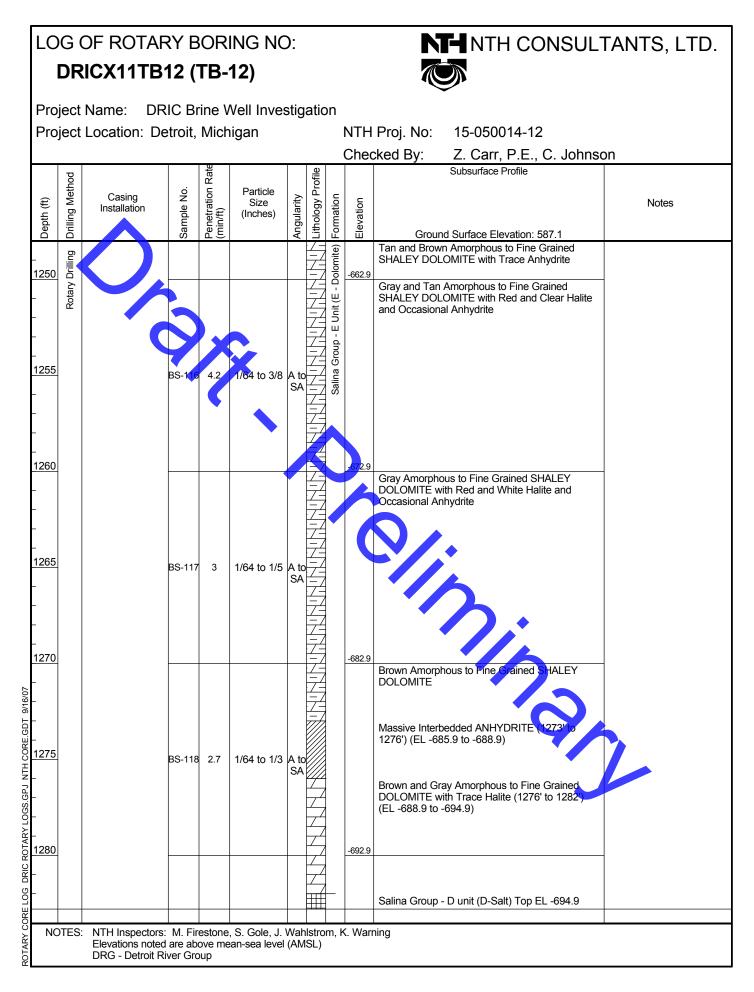


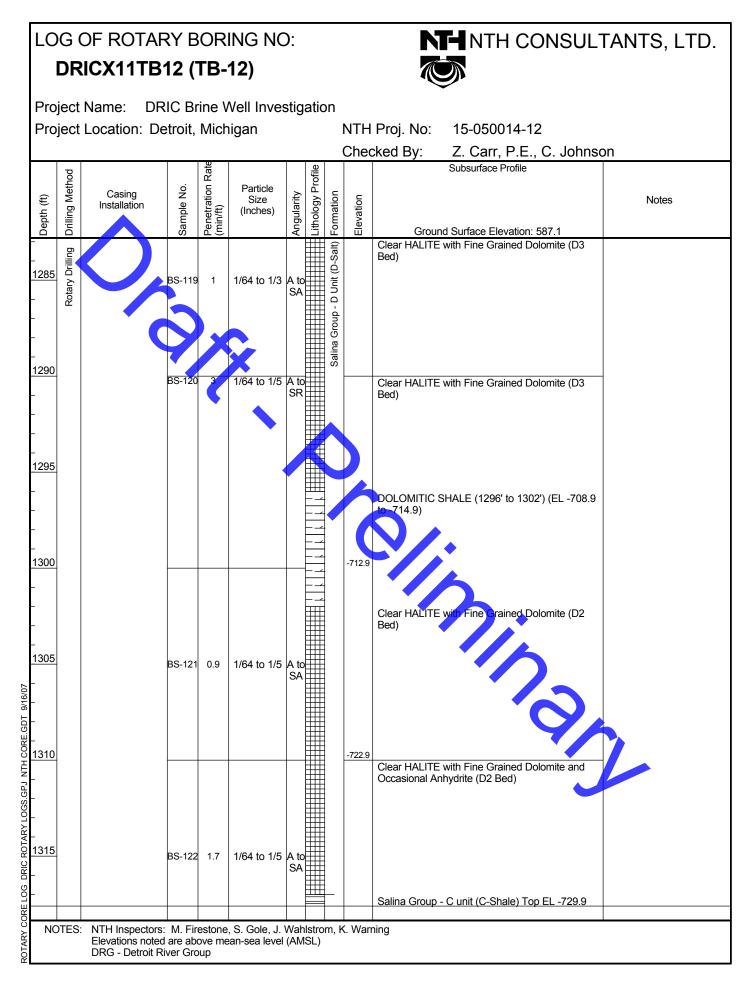


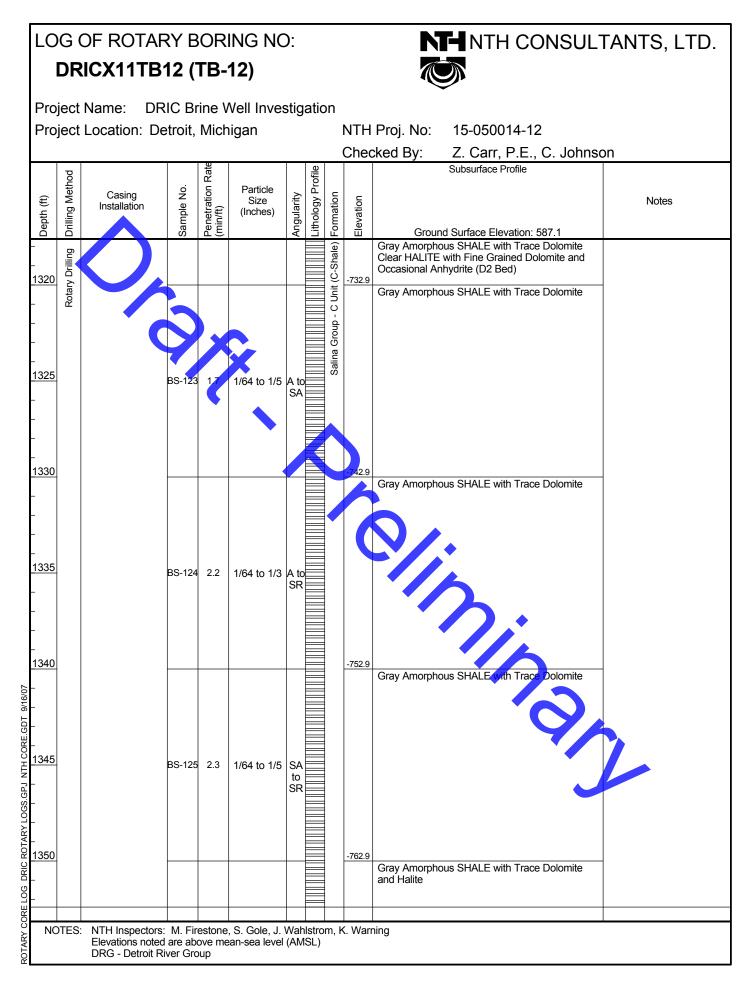


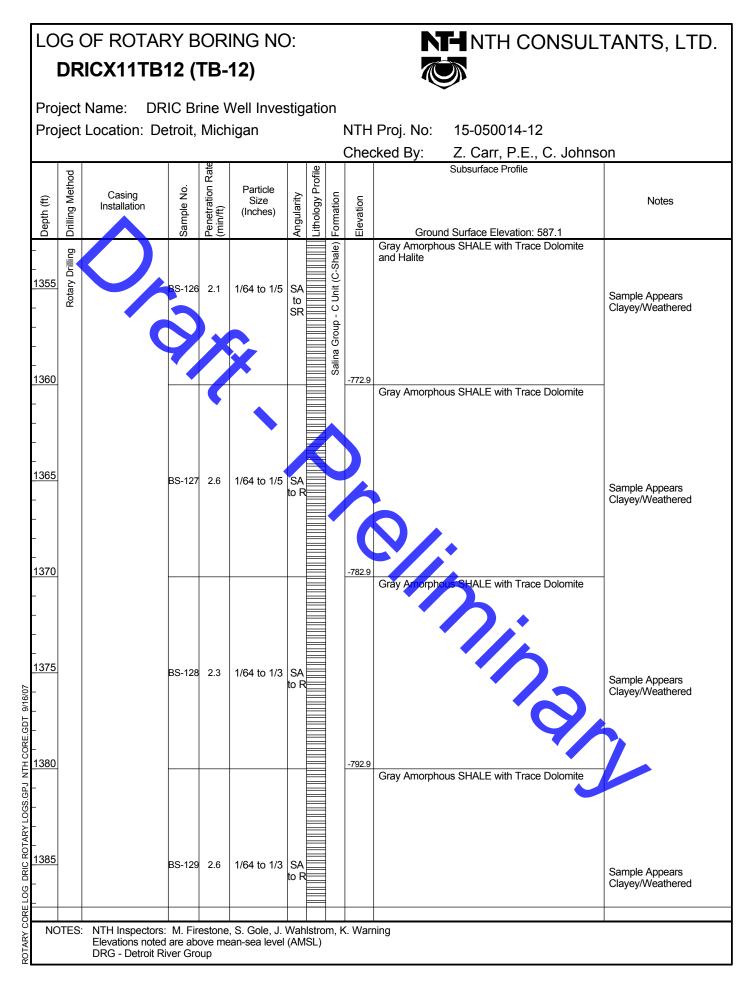


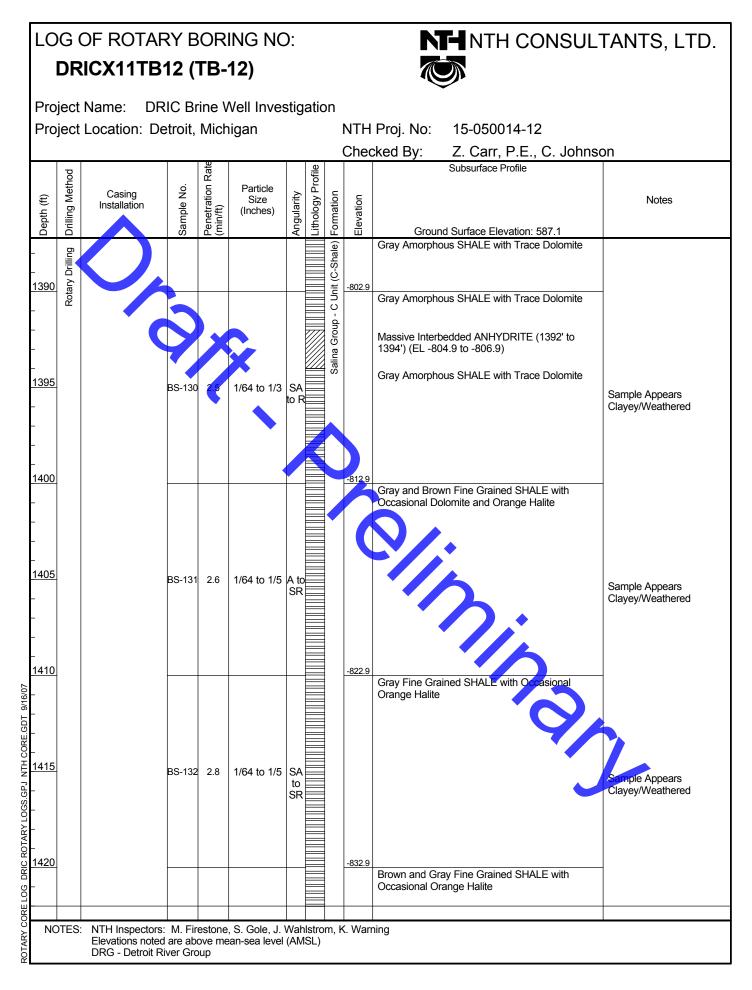


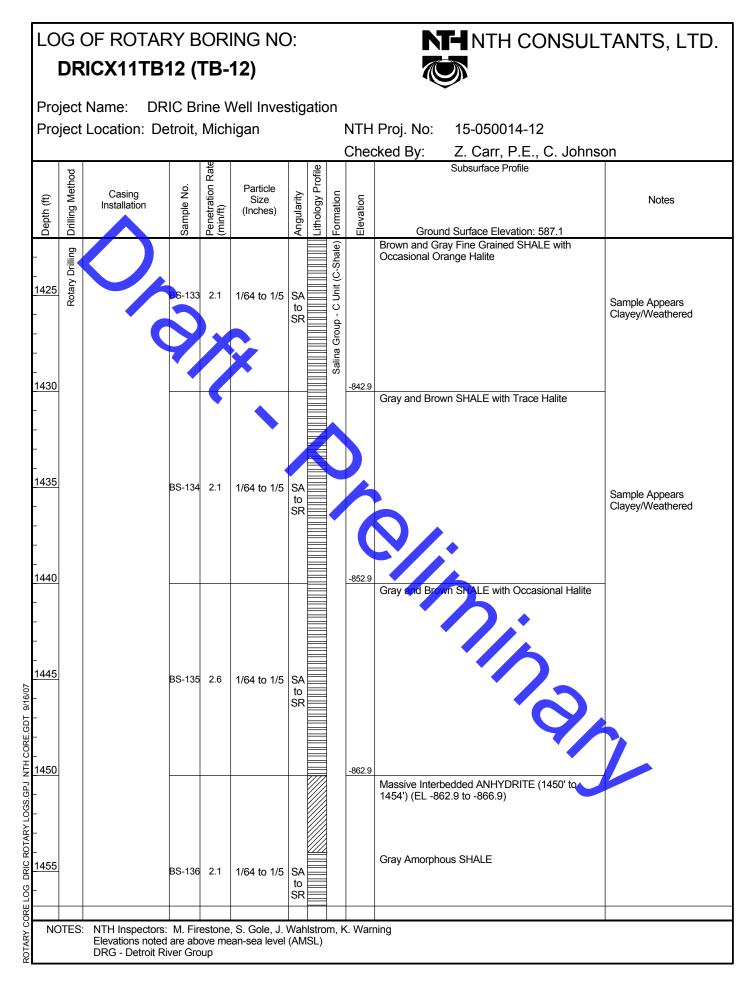


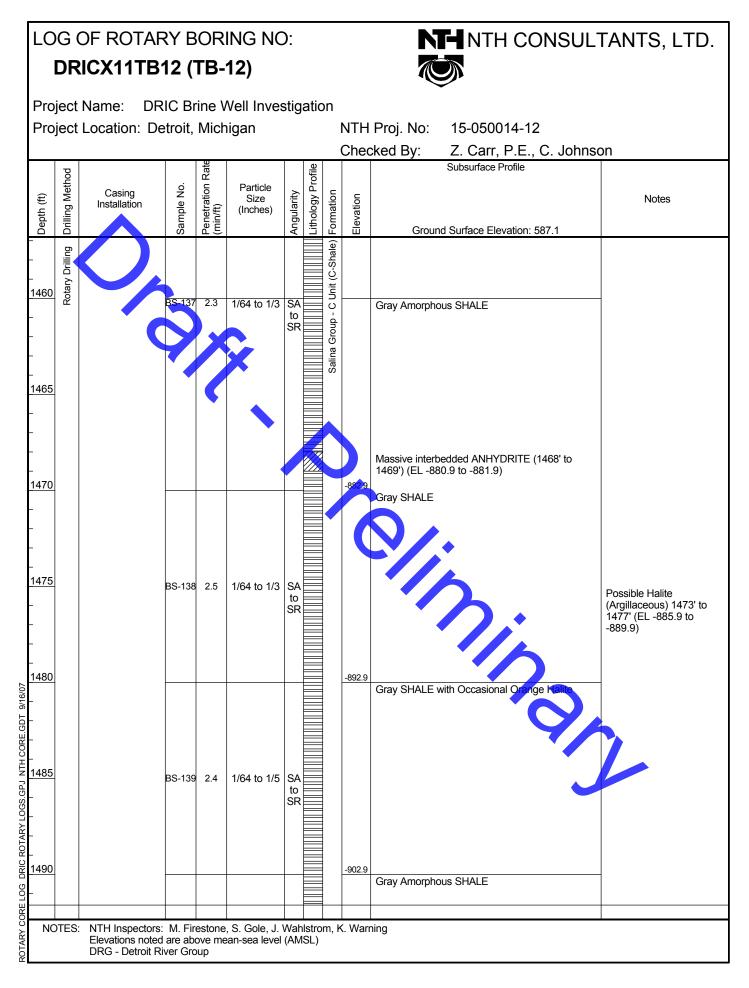


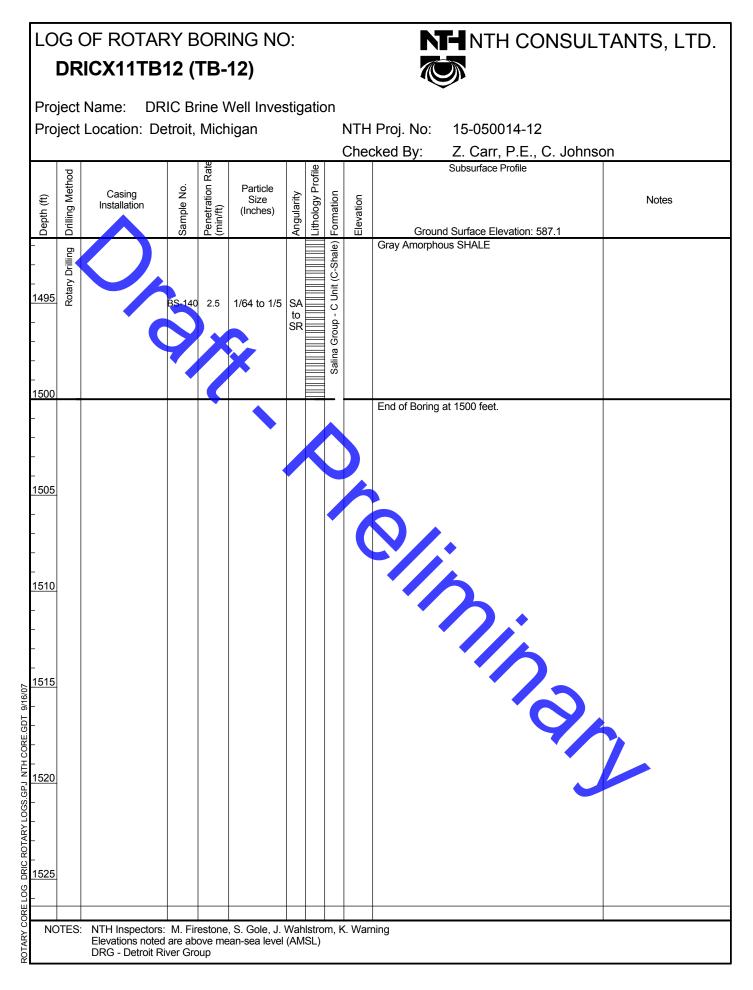




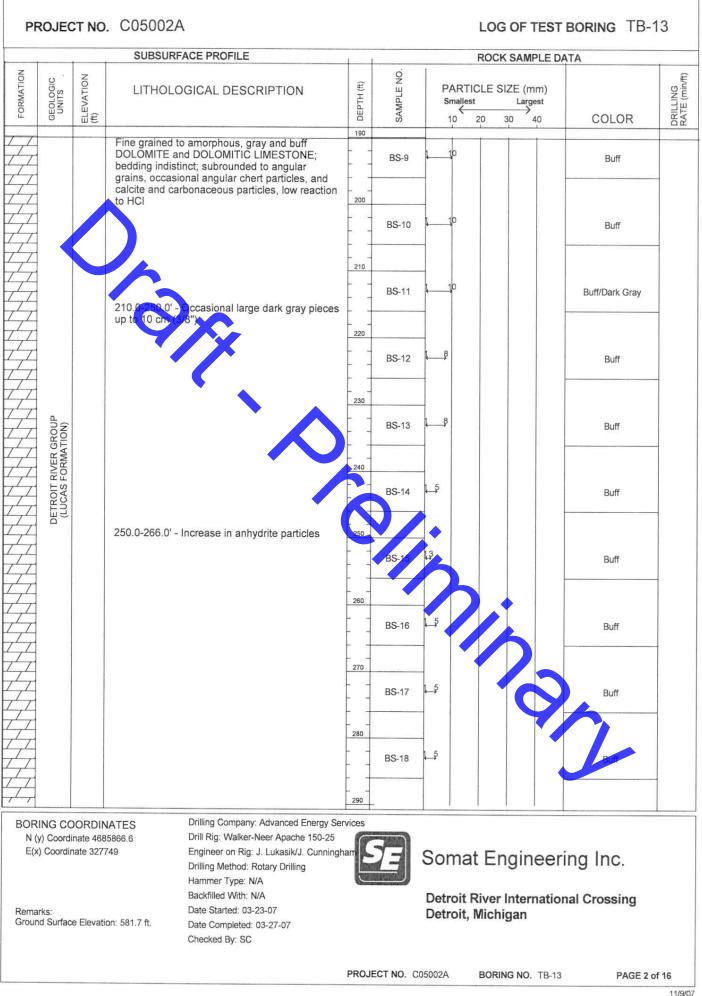




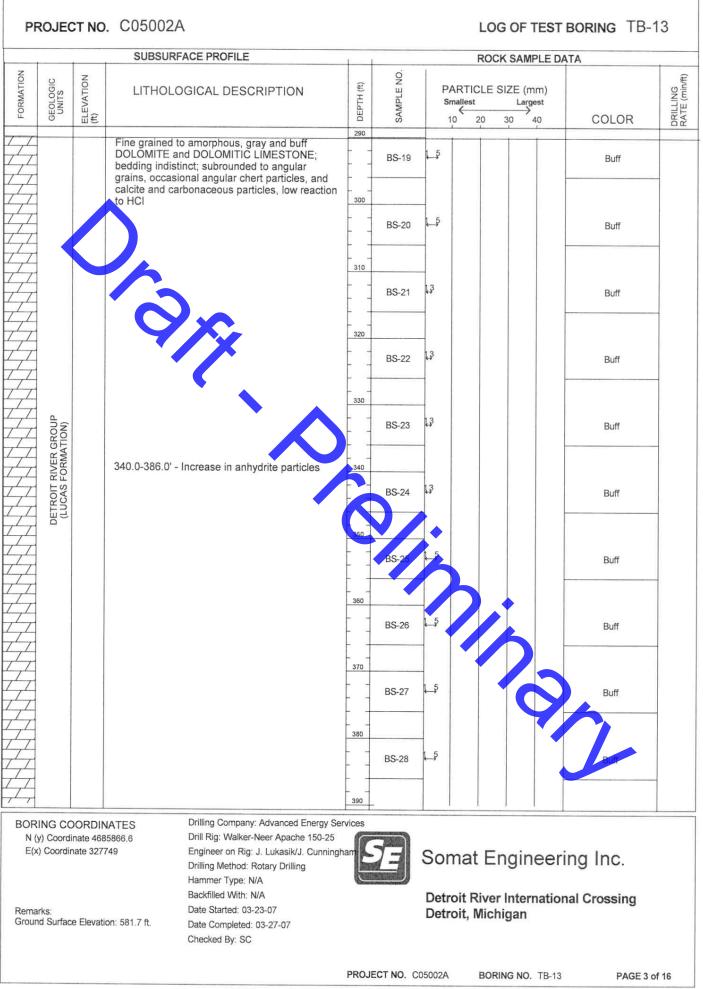


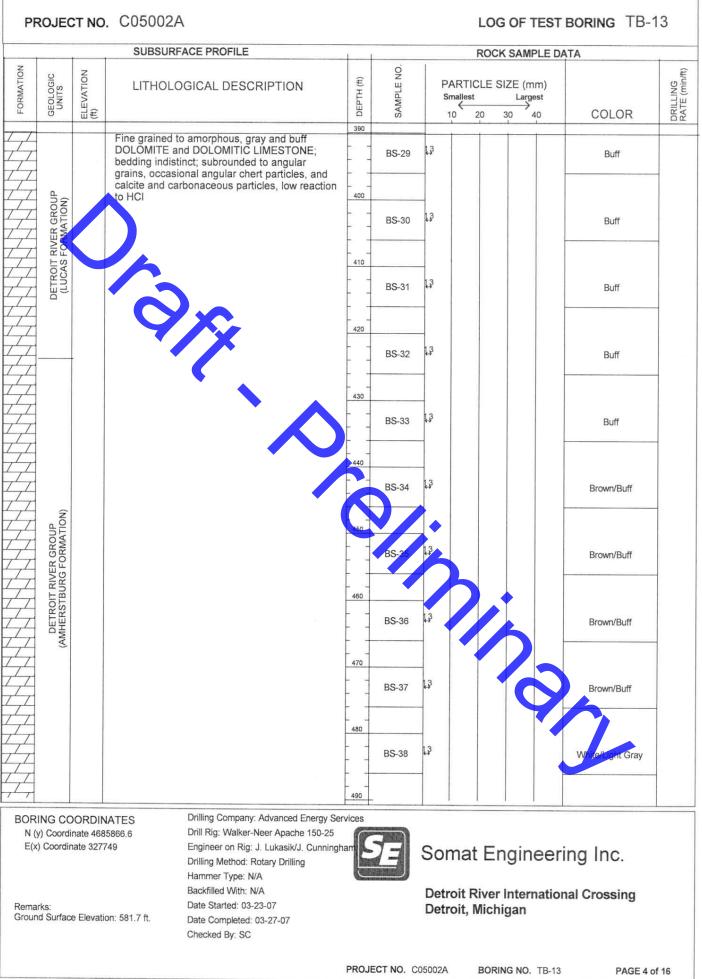


			SUBSURFACE PROFILE	1	i		ROCK SAMPL	E DATA	
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLOGICAL DESCRI	PTION (L) HLLAID	SAMPLE NO.	PART Smalle	FICLE SIZE (mm st Largest 20 30 40		DRILLING
UD			GLACIAL SEDIMENT (DRIFT)						-
UD,									
		481.7		100					
			Fine grained to amorphous, gray DIVESTONE; bedding laminar to sup angular to angular grains, occ angular chert particles, calcite ar	to buff indistinct; casional					
			carbonaceous particles, moderat HCI	e reaction to		4			4
					BS-1	<u> </u>	25	Gray/Buff	
			YX.	120					
	IONE				BS-2	<b>↓</b> ₿		Gray/Buff	
	IMES'		· · · · ·	130					
	DUNDEE LIMESTONE		•		BS-3	₹1 <sup>4</sup>	5	Gray/Buff	
	ā			140					
		-			BS-4	210		Gray/Buff	
					BS-5	2 10		Gray/Buff	
						0			-
Ż		421.7	Fine grained to amorphous, gray DOLOMITE and DOLOMITIC LIN bedding indistinct; subrounded to	IESTONE;	BS-6	2	-20	Gray/Buff	
7			grains, occasional angular chert p calcite and carbonaceous particle to HCI	particles, and					
7					BS-7	10		Gray/Buff	
7									-
4				180	<b>PO 0</b>	1 10			2.
7					BS-8			Sult	
Ľ,				190					
N ()	y) Coord	DORDIN inate 468 nate 327	5866.6 Drill Rig: Walker-Ne 49 Engineer on Rig: J. Drilling Method: Rot Hammer Type: N/A	Lukasik/J. Cunningham ary Drilling	SE)			ering Inc.	
Rema Groun		e Elevat	Date Started: 03-23- Date Started: 03-23- Date Completed: 03 Checked By: SC	-07			River Interna , Michigan	ational Crossing	



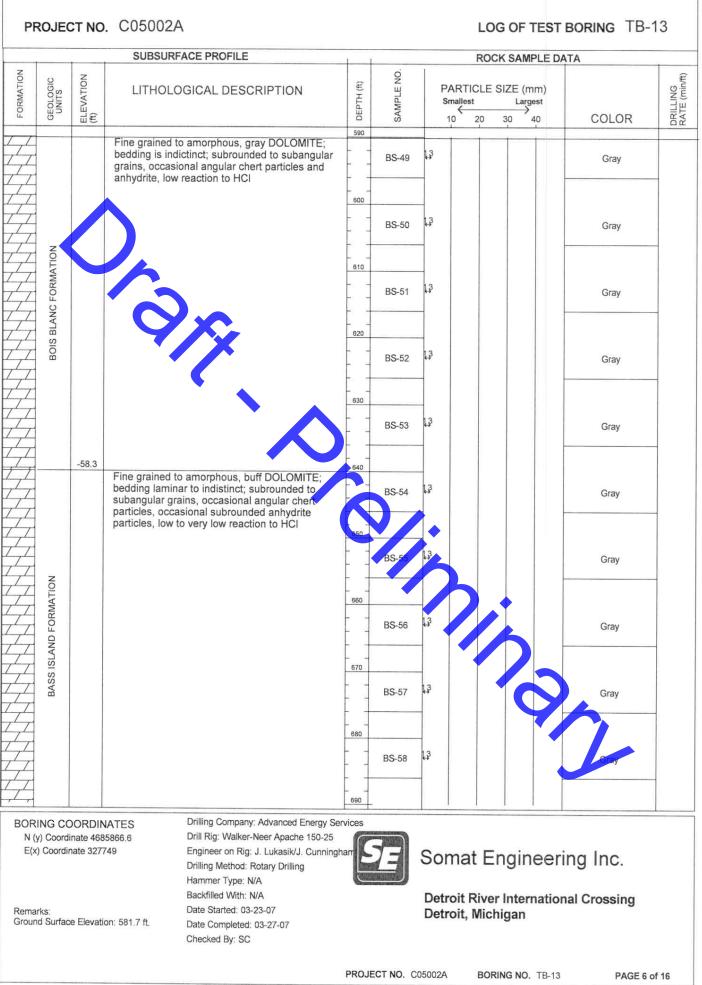
DRIC LOG DRIC.GPJ SOMAT.GDT

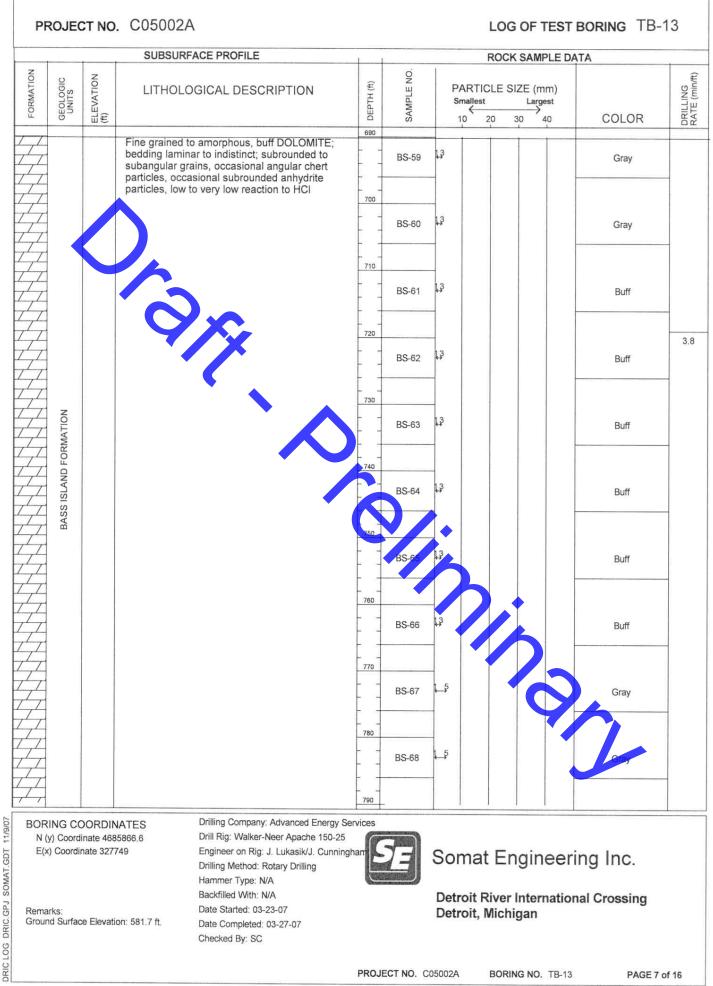




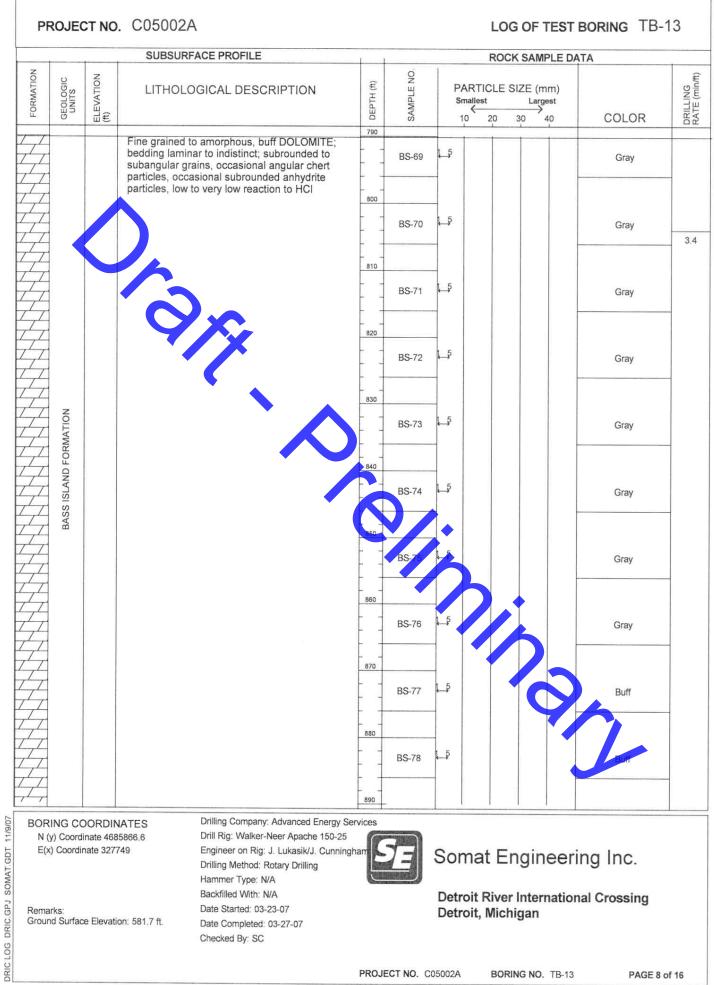
	(OJL)		. C05002A	4					LO	GO	F TEST	BORING TB-1	13
		1	SUBSUR	FACE PROFILE	+ +				ROO	CKSA	MPLE D	ATA	1
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLO	DGICAL DESCRIPTION	DEPTH (ft)	SAMPLE NO.		PART Smaller			(mm) argest → 40	COLOR	DRILLING RATE (min/ft)
7			DOLOMITE a bedding indist grains, occasi	o amorphous, gray and buff nd DOLOMITIC LIMESTONE; inct; subrounded to angular onal angular chert particles, and	490	BS-39	13					White/Light Gray	1.1
		79.7	to HCI Fine to mediu SACCHAROI	rbonaceous particles, low reaction m grained, light gray to white DAL SANDSTONE; subrounded to ains, occasional dolomite and	500	BS-40	ţ3					White/Light Gray	
			subangular gra gypsum particle	eles, low reaction to HCI	510	BS-41	13					White/Light Gray	
	ш		0	12	520	BS-42	13					White/Light Gray	-
	SYLVANIA SANDSTONE			<u>ن</u>	530	BS-43	13					White/Light Gray	-
	SYLVI					BS-44 BS-45	13					White/Light Gray White/Light Gray	
	z	21.7	bedding is ind grains, occasi	o amorphous, gray DOLOMITE; ictinct; subrounded to subangular onal angular chert particles and reaction to HCI	560	BS-46	13					White/Light Gray	-
	BOIS BLANC FORMATION				570	BS-47	13				0	Gray	2.8
	BOIS BLAN				<u>580</u>  	BS-48	13					2	
N () E(x Rema	y) Coord :) Coordii :rks:	DORDIN inate 468 nate 327 ce Elevati	5866.6	Drilling Company: Advanced Energy Se Drill Rig: Walker-Neer Apache 150-25 Engineer on Rig: J. Lukasik/J. Cunningl Drilling Method: Rotary Drilling Hammer Type: N/A Backfilled With: N/A Date Started: 03-23-07 Date Completed: 03-27-07 Checked By: SC	6	SE)	De		Rive	r Int	ernatio	ing Inc. nal Crossing	

11/9/07

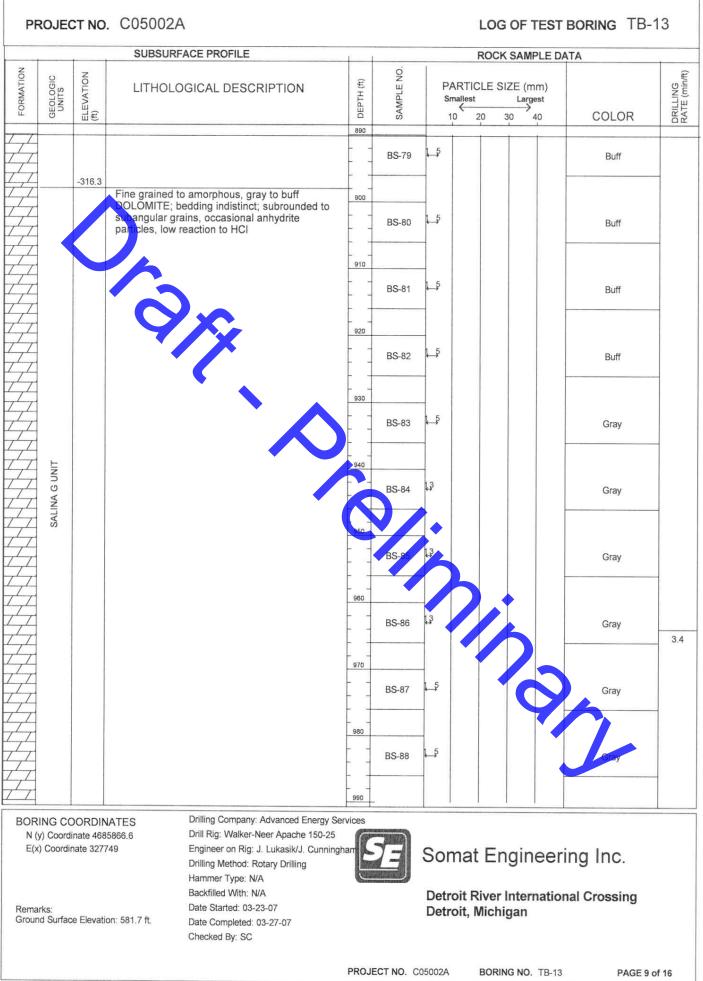




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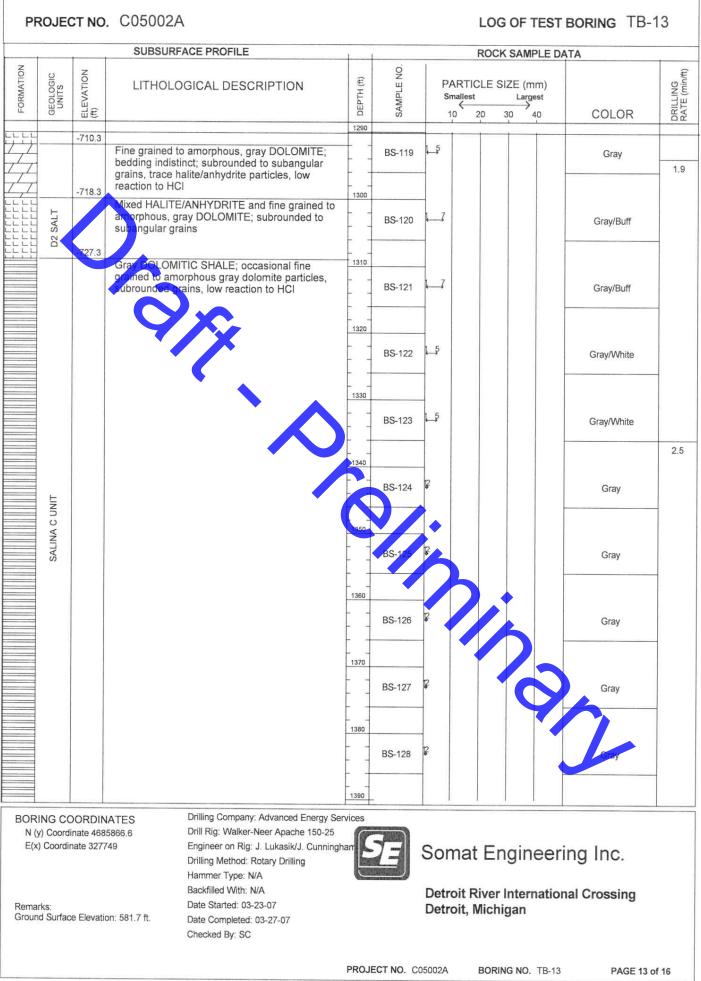


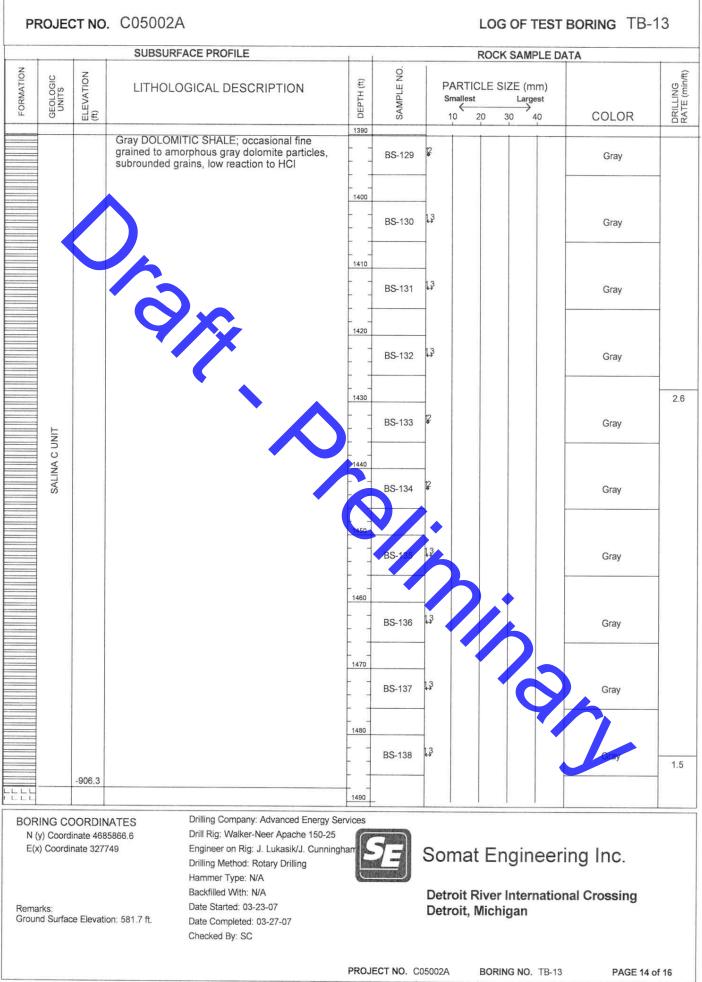
			SUBSURFACE PROFILE	1 1		RO	CK SAMPLE DA	ТА	
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLOGICAL DESCRIPTION	066 DEPTH (ft)	SAMPLE NO.	PARTICLE Smallest	SIZE (mm) Largest	COLOR	DRILLING
7		-412.3			BS-89	5		Gray	
	F5 SALT	-415.3	Crystalline HALTIE/ANHYDRITE and fine grained to amorphous DOLOMITE; subro	unded t t					2.
7			to subangular grains, very low reaction to Mixed fine grained to amorphous, gray DOLOMITE, LIMESTONE and CRYSTAL	1000		-			
7	SALINA		HALITE and ANHYDRITE, subrounded to angular grains	LINE	BS-90	13		Gray	
7		-430.3	6	1010					
			Orystalling HALITE/ANHYDRITE; occasio fine grained to amorphous, gray DOLOMI	nal TE;	BS-91	13		Buff	
			subrounded to subapontar grains	1020					
	F4 SALT				BS-92	1_5		Buff	
	F4								1.
		-453.3			BS-93	<b>↓</b> 5		Gray	
7		100.0	Mixed fine grained to amorphous, gray DOLOMITE and crystalline			-			-
4	SALINA F UNIT		HALITE/ANHYDRITE; subrounded to subangular grains, low raction to HCI	1040		1.5			
1		-465.3			BS-94			Gray	_
			Mixed HALITE/ANHYDRITE and fine grain amorphous, gray DOLOMITE; subrounded subangular grains	ned to					
					BS-95			Gray	
	SALT			1060			•		
	F3 S				BS-96	L5		Buff	
				1070					1
		-493.3			BS-97	<b>↓_</b> 5		Buff	
4	TIN		Fine grained to amorphous, gray DOLOMI bedding indistinct; subrounded to subangu	ılar 🖵 🚽					_
7	IA F UNIT		grains, trace halite/anhydrite particles, low reaction to HCI	1080	BS-98	L3			
7	SALINA								4.0
4				1090					
N ()	) Coordi	ORDIN inate 468 nate 3277	5866.6 Drill Rig: Walker-Neer Apache	150-25	E		Ingineeri		
Remar Groun		e Elevati	on: 581.7 ft. Date Completed: 03-27-07 Checked By: SC			Detroit Rive Detroit, Mic	er Internation higan	al Crossing	

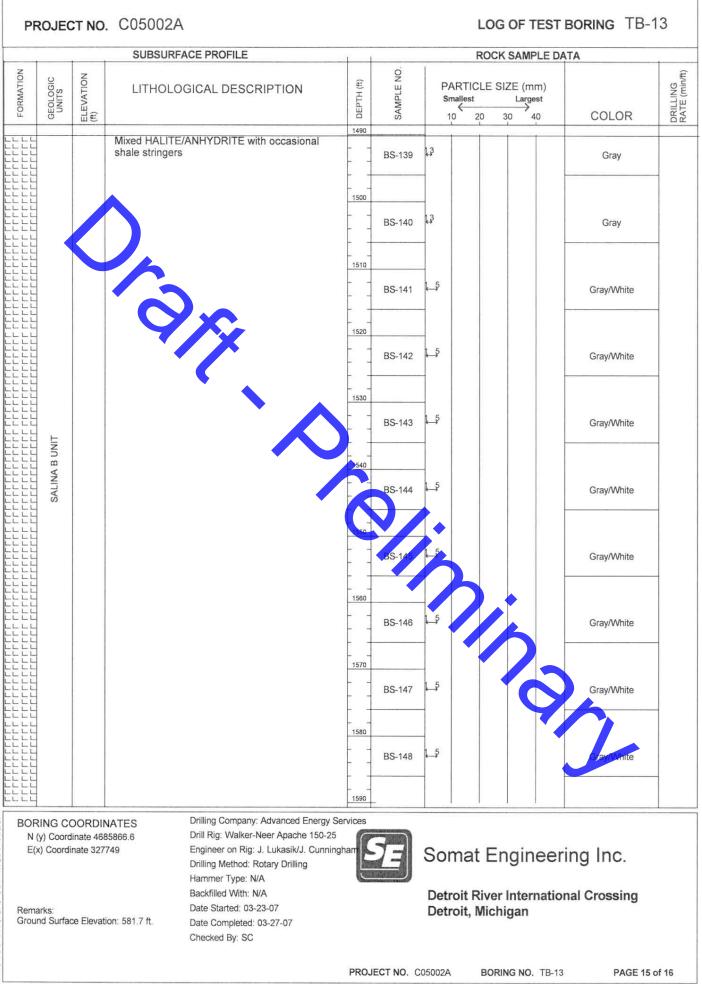
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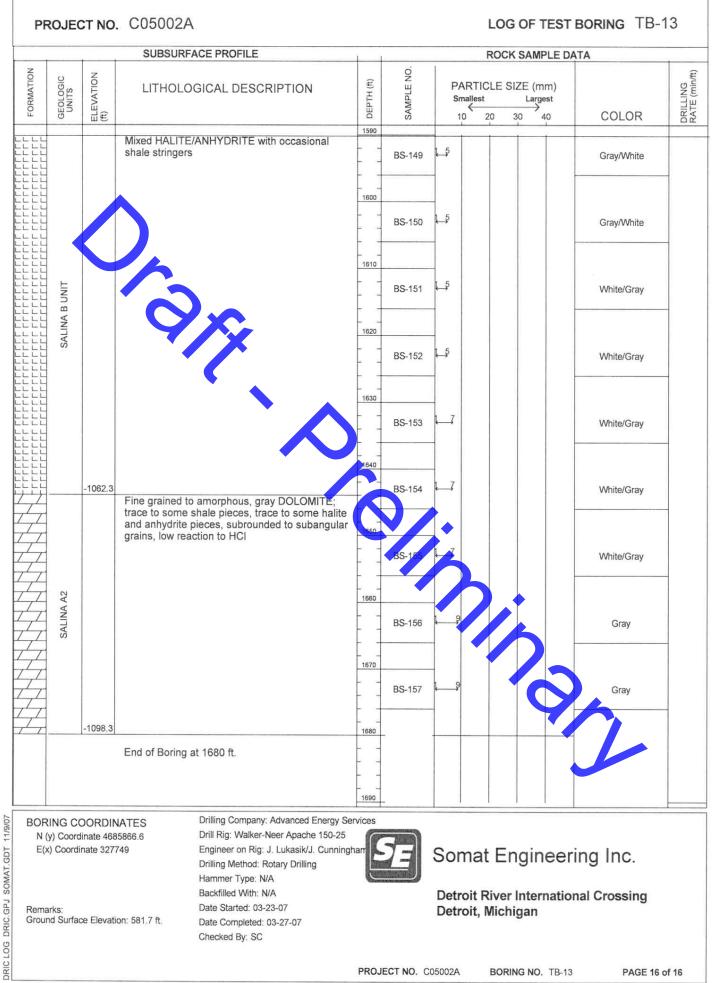
PR	ROJEC	CT NO	. C05002A				LO	G OF	TEST E	BORING TB-	13
		1	SUBSURFACE PROFILE				ROO	CK SAI	VIPLE DAT	A	
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLOGICAL DESCRIPTION	DEPTH (ft)	SAMPLE NO.		RTICLE		mm) rgest ≯ 40	COLOR	DRILLING RATE (min/ft)
	SALINA F UNIT		Fine grained to amorphous, gray DOLOMITE; bedding indistinct; subrounded to subangular grains, trace halite/anhydrite particles, low reaction to HCI	1090	BS-99	Ļ3				Buff	
	SALINA	-522.3	Mixed HALITE/ANHYDRITE and fine grained to	1100	BS-100	13				Buff	
	н		amorphous, gray DOLOMITE; subrounded to subangular grains	  	BS-101					Gray/White	
	F2 SALT		Qx.	1120							3.3
		-545.3	Fine grained to amorphous, gray DOLOMITE;		BS-102	13				White/Gray	
		-548.3	bedding indistinct; subrounded to subangular grains, trace halite/anhydrite particles, low reaction to HCI Mixed HALITE/ANHYDRITE and fine grained to amorphous, gray DOLOMITE; subrounded to subangular grains	1130	BS-103	- 13				White/Gray	_
					BS-104	1_5				Gray/White	
	1 SALT				BS-105					Gray/White	
	LL.			<u>1160</u>	BS-106	15				Gray/White	
				 	BS-107	- 15 -			9	Gray	
		-600.3	Fine grained to amorphous, gray DOLOMITE; bedding indistinct; subrounded to subangular grains, trace halite/anhydrite particles, low reaction to HCI	1180	BS-108	- 15 -				Gray	4.3
N (y E(x) Remar	r) Coordi ) Coordir 'ks:	DORDIN nate 468 nate 3277	5866.6 Drill Rig: Walker-Neer Apache 150-25		5E	Detro		r Inte	rnationa	ng Inc. Il Crossing	
				PROJI	ECT NO. CO	05002A	BOR	RING NO	D. TB-13	PAGE 11	of 16

PF	OJEC	CT NO	. C05002A					LOC	G OF T	EST B	oring TB-	13
ļ,			SUBSURF	ACE PROFILE				ROCI	K SAMP	LE DAT	A	
FORMATION	GEOLOGIC	ELEVATION (ff)	LITHOLO	GICAL DESCRIPTION	(ii) HLLH(ii)	SAMPLE NO.	S	ARTICLE SIZE (mm) mallest Largest 10 20 30 40			COLOR	DRILLING RATE (min/ft)
			bedding indistin	amorphous, gray DOLOMITE; act; subrounded to subangular lite/anhydrite particles, low		BS-109	1_5				Gray	_
					1200	BS-110	<b>↓</b> 5				Gray	
			1		1210	BS-111	13				Gray	
			0	<b>X</b>	 1220	50.110	15					_
			•		1230	BS-112					Gray/Buff	
	SALINA E UNIT					BS-113	<b>1</b> 5				Gray/Buff	
	SP				1240	BS-114	13				Gray	
					 	BS-115	43				Gray	
					1260 	BS-116	5	2			Light Gray	
					1270	BS-117	5		6	Q	Light Gray	
	F	-700.3	Mixed HALITE/	ANHYDRITE and fine grained to	1280	BS-118	5				Jeht Grav	
	D3 SALT		amorphous, gra subangular grai		1290	-						_
N ()	BORING COORDINATES       Drilling Company: Advanced Energy Serv         N (y) Coordinate 4685866.6       Drill Rig: Walker-Neer Apache 150-25         E(x) Coordinate 327749       Engineer on Rig: J. Lukasik/J. Cunningha         Drilling Method: Rotary Drilling       Hammer Type: N/A							mat Er				
Remai Groun		e Elevati		Backfilled With: N/A Date Started: 03-23-07 Date Completed: 03-27-07 Checked By: SC				oit River oit, Mich		ationa	I Crossing	
					PROJI	ECT NO. CO	05002A	BORI	NG NO.	TB-13	PAGE 12 0	of 16



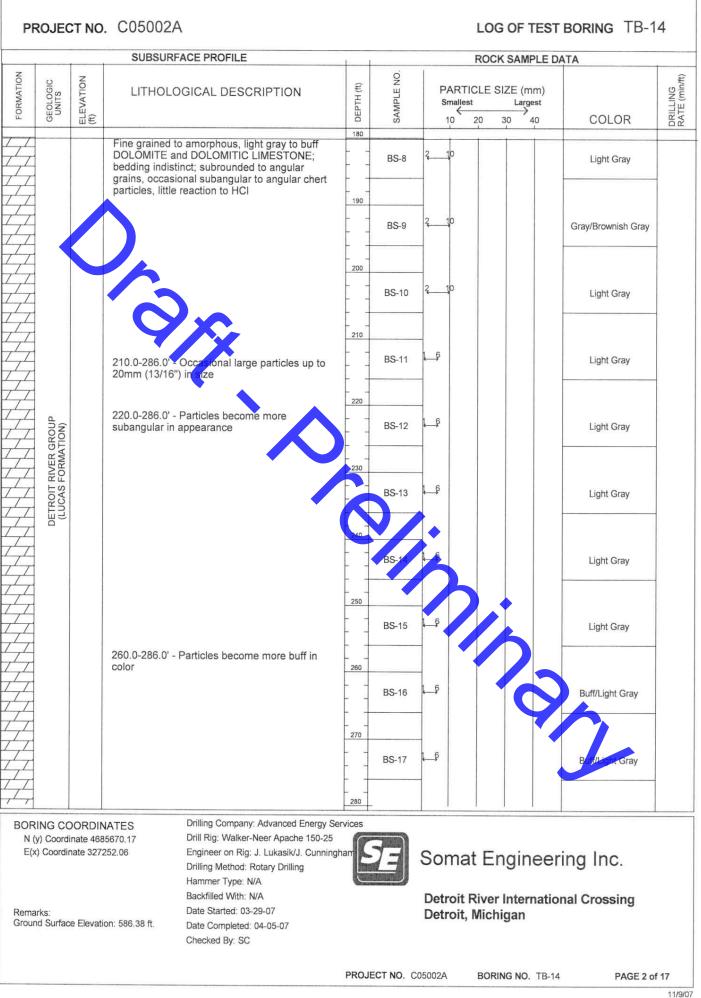




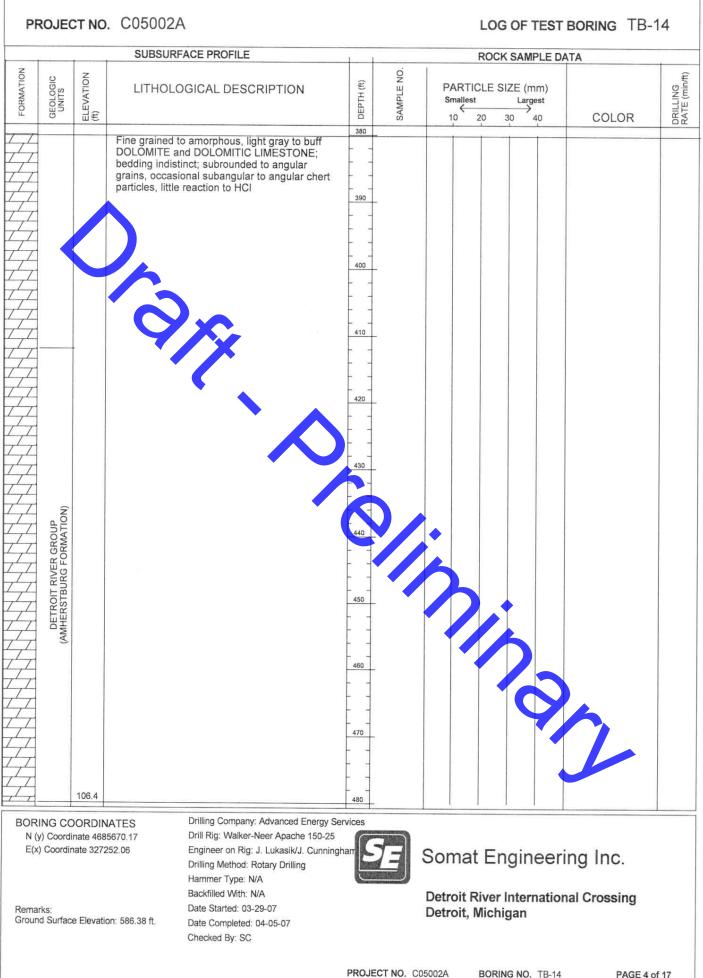


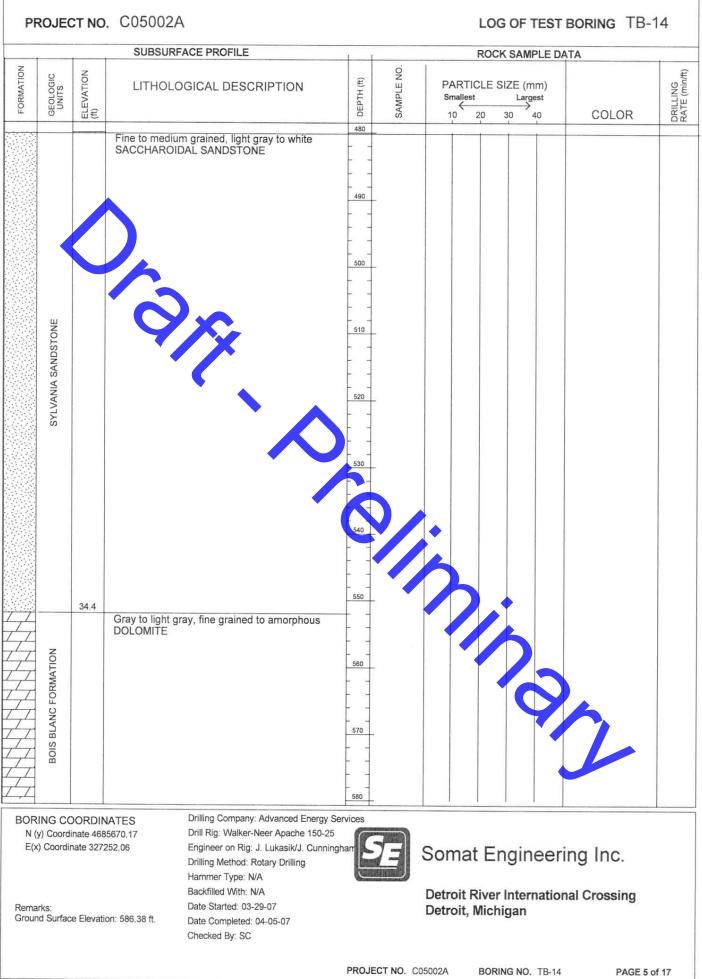
PF	ROJEC	T NO	. C05002A					LOG OF T	EST BOF	ring TB-1	14
			SUBSURFACE	PROFILE				ROCK SAMP	LE DATA		
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLOGIC	AL DESCRIPTION	8 DEPTH (ft)	SAMPLE NO.	PAR Small		ŧ	COLOR	DRILLING RATE (min/ft)
	DUNDEE LIMESTONE	496.4	LIMECTONE; beddi andular parains, occa subangular earbona hydrocarbon staining reaction to HCI	rrphous, light gray to brown ng indistinct; subrounded to ceous particles, occasional g, strong to moderate		BS-1 BS-2 BS-3 BS-5 BS-5 BS-6				Light Gray Light Gray Light Gray Light Gray Light Gray	
			DOLOMITE and DO bedding indistinct; si	rphous, light gray to buff LOMITIC LIMESTONE; ubrounded to angular ubangular to angular chert on to HCl	  180	BS-7	210			Light Gray	
N ( E(x Rema	BORING COORDINATES       Drilling Company: Advanced Energy Sen         N (y) Coordinate 4685670.17       Drill Rig: Walker-Neer Apache 150-25         E(x) Coordinate 327252.06       Engineer on Rig: J. Lukasik/J. Cunningha         Drilling Method: Rotary Drilling       Hammer Type: N/A         Backfilled With: N/A       Date Started: 03-29-07         Ground Surface Elevation: 586.38 ft.       Date Completed: 04-05-07         Checked By: SC       SC						Detroit	River Intern , Michigan	ational C		f 17

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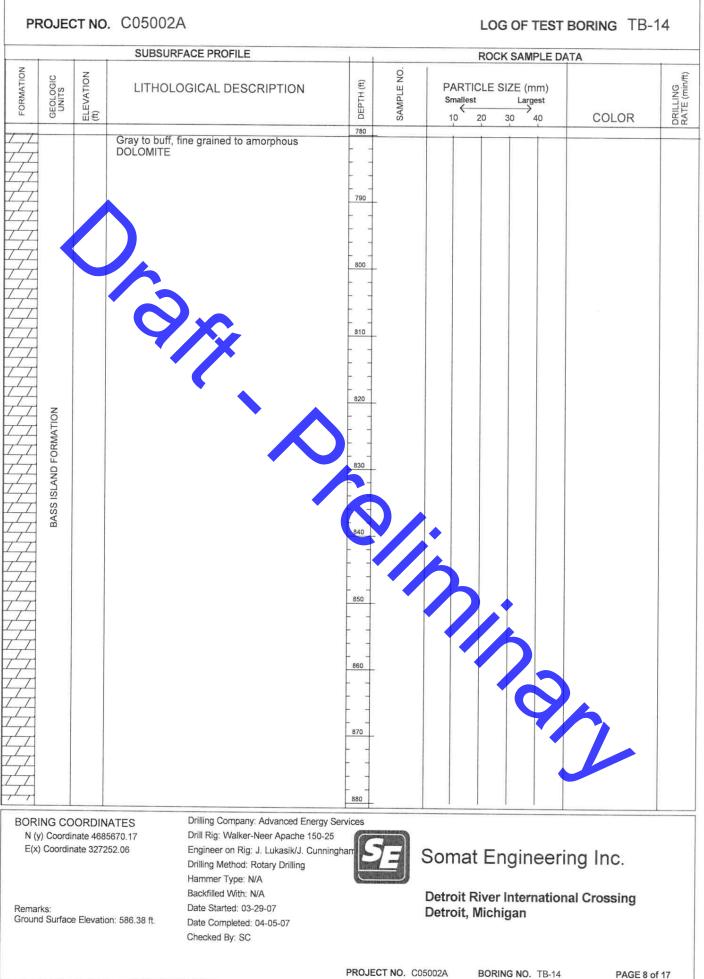


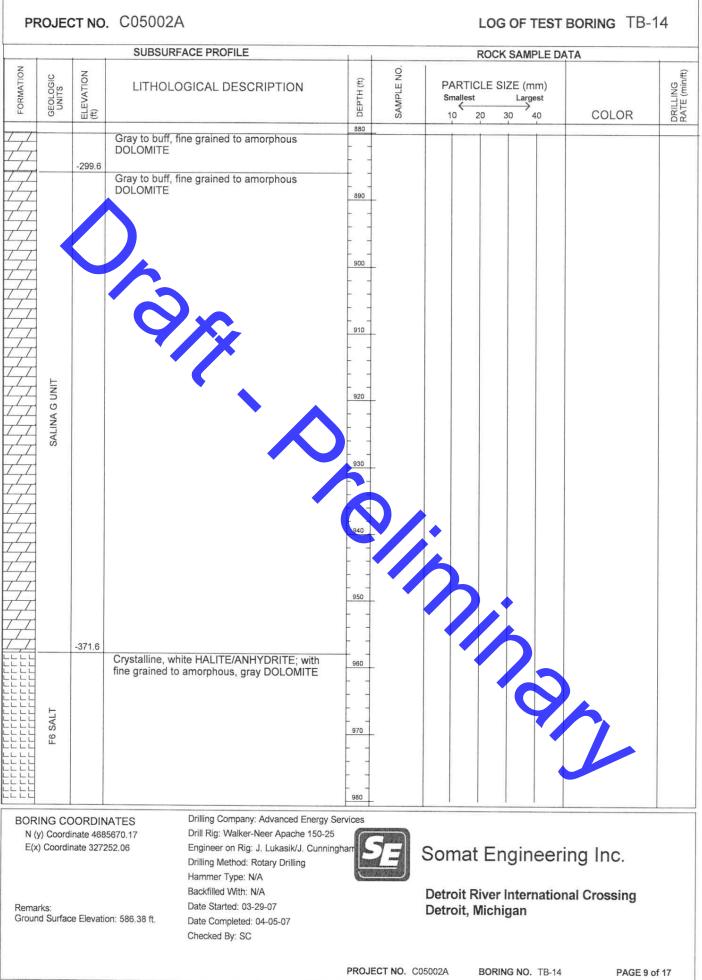




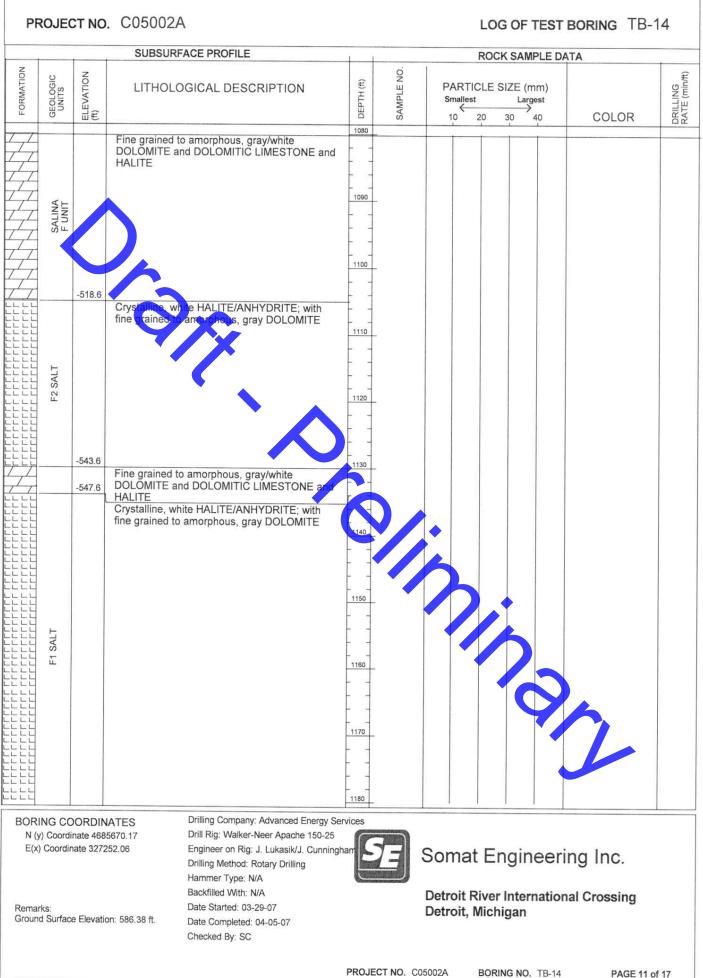
PROJECT NO. C05002A

BORING NO. TB-14

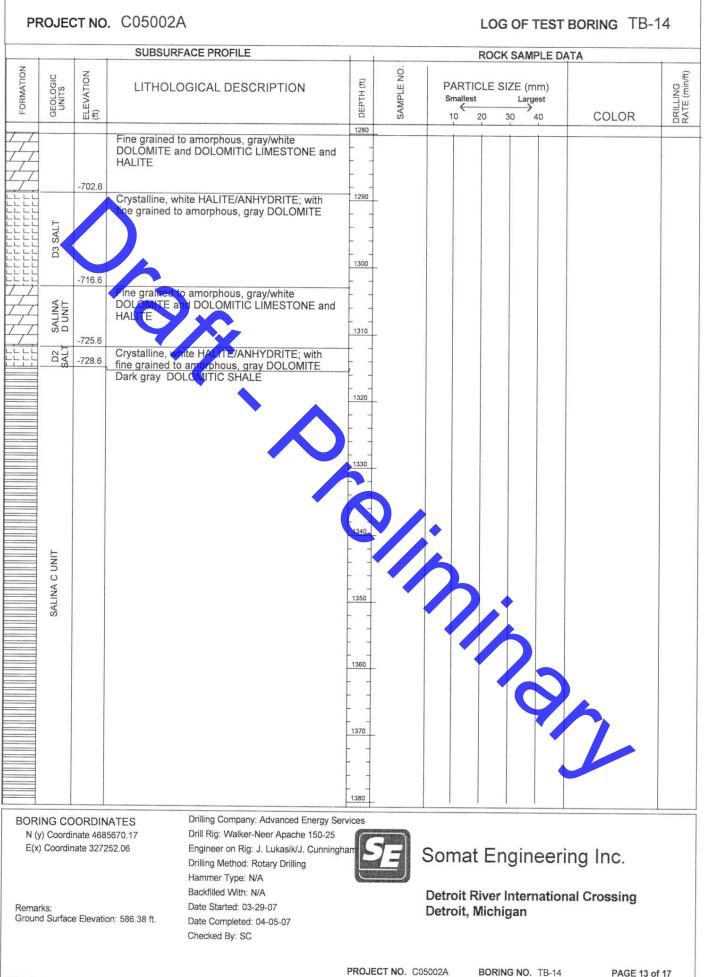


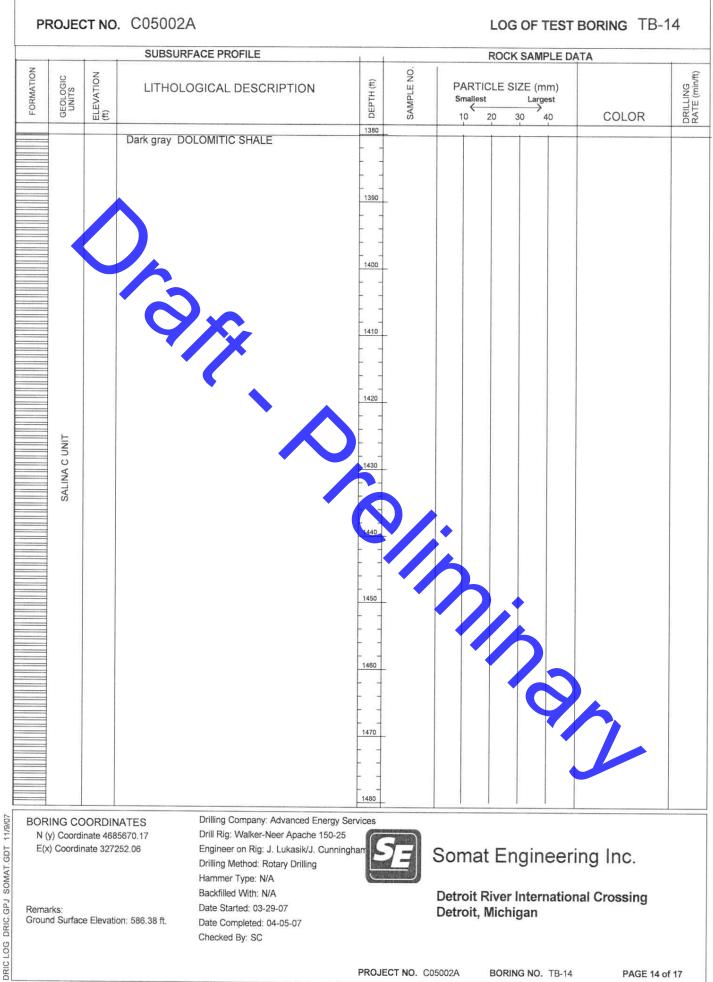


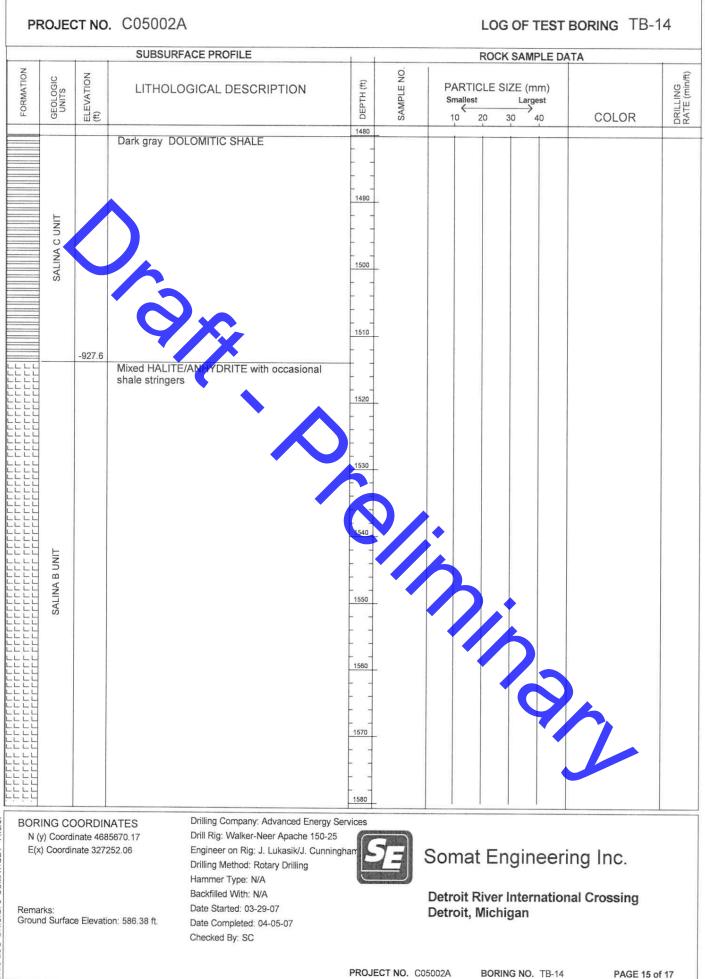
PR	OJEC	CT NC	. C05002A	Ą				LOG	OF TEST	boring TB-	14
		,	SUBSURF	FACE PROFILE		1		ROCK	SAMPLE DA	TA	
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLO	OGICAL DESCRIPTION	DEPTH (ft)	SAMPLE NO.	PART Smalle	FICLE SIZ		COLOR	DRILLING RATE (min/ft)
	F6 SALT	-399.6	Crystalline, wh fine grained to	nite HALITE/ANHYDRITE; with amorphous, gray DOLOMITE	980	-					
		-405,8	Fine grained t DOLOMITE an HALITE	o amorphous, gray/white nd DOLOMITIC LIMESTONE and	990						
	F5 SALT	412.6	Crystalline, wh fine grained to	nite HALITE/ANHYDRITE; with amorphous, gray DOLOMITE							
	UNIT		Fine grained to DOLOMITE an HALITE	o amorphous, gray/white nd DOLOMITIC LIMESTONE and	1000	-					
	SALINA F UNIT	-427.6	0	1	 1010	-					
		427.0	Crystalline, wh fine grained to	hite HALITE/ANHYDRITE; with an prophous, gray DOLOMITE	1020	-					
	F4 SALT				1030	-					
	41	-449.6	Fine grained to DOLOMITE ar	o amorphous, gray/white nd DOLOMITIC LIMESTONE and	1040						
	SALINA F UNIT	-460.6	HALITE								
			Crystalline, wh fine grained to	ite HALITE/ANHYDRITE; with amorphous, gray DOLOMITE	1050						
	F3 SALT					-			3		
					<u>1070</u>	-				2	
		-491.6			1080						
N (y	NG CC ) Coordi Coordir	nate 468	5670.17	Drilling Company: Advanced Energy Se Drill Rig: Walker-Neer Apache 150-25 Engineer on Rig: J. Lukasik/J. Cunning Drilling Method: Rotary Drilling Hammer Type: N/A	0	SE)	Soma	at Eng	gineeri	ng Inc.	
Remar Ground	2.252	e Elevati	on: 586.38 ft.	Backfilled With: N/A Date Started: 03-29-07 Date Completed: 04-05-07 Checked By: SC			Detroit Detroit,	River Ir Michig	iternation an	al Crossing	
					PROJ	ECT NO. C0	5002A	BORING	NO. TB-14	PAGE 10	of 17

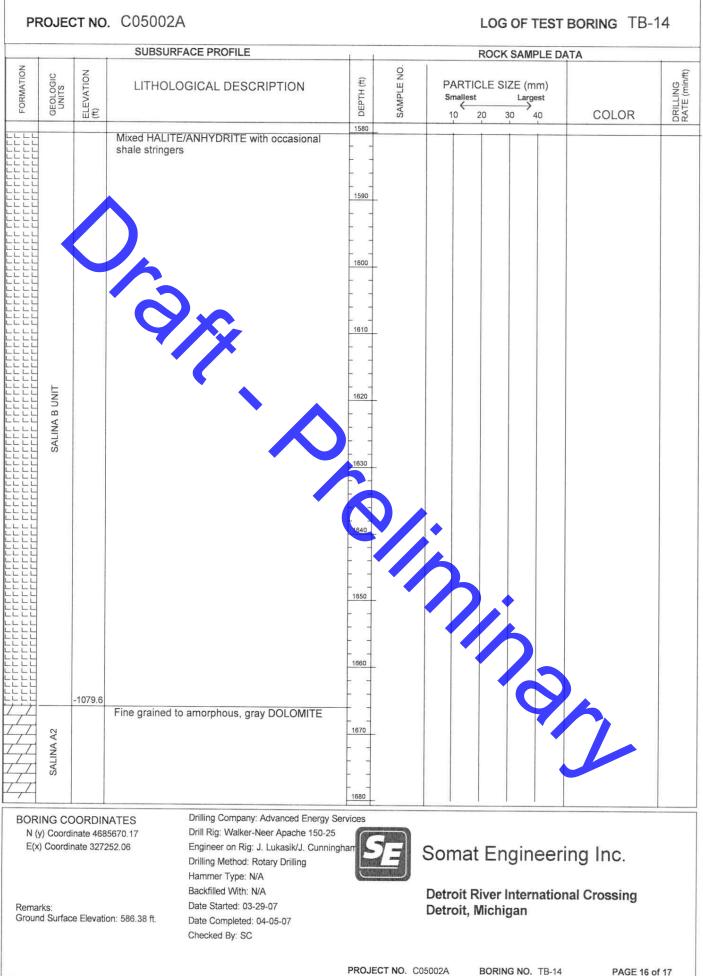


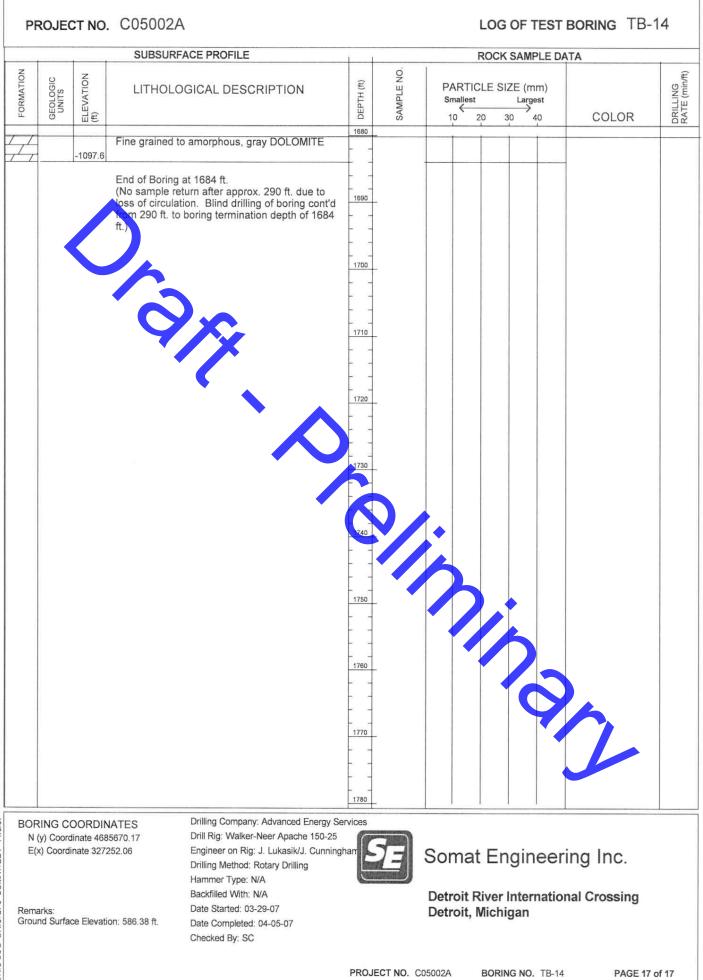




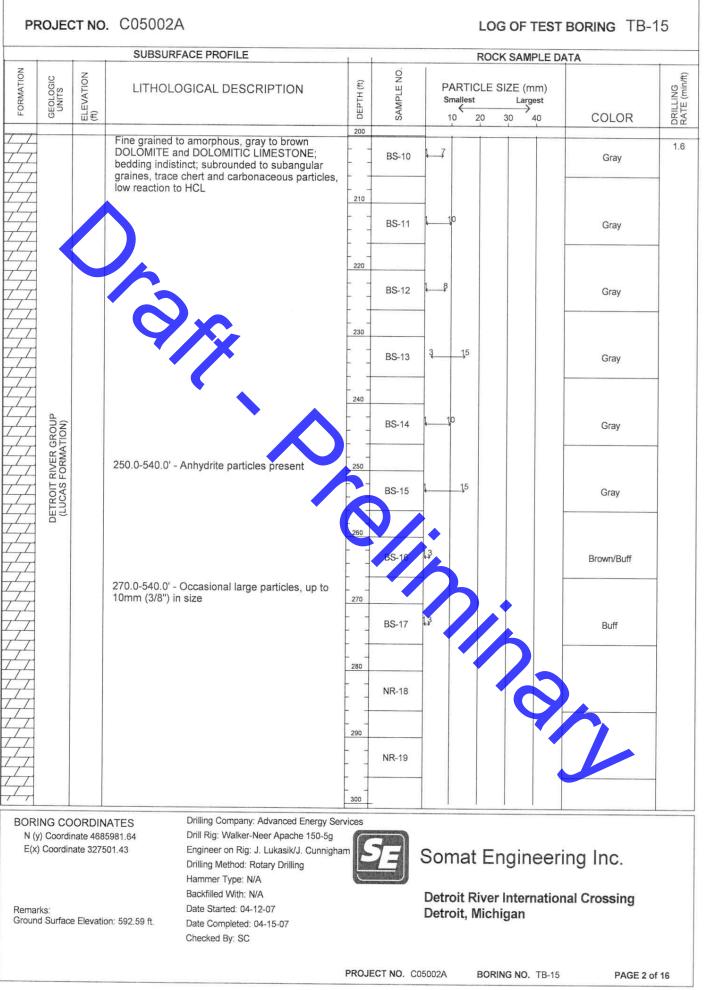


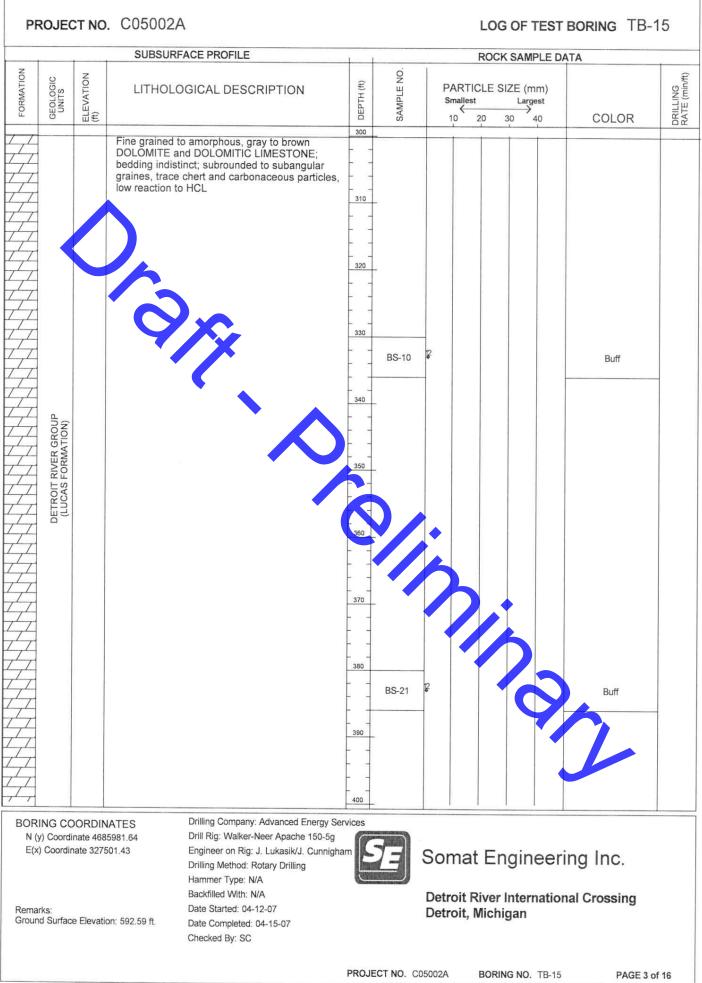






PR	ROJEC	CT NO	. C05002A					LOG (	OF TEST E	boring TB-	15
			SUBSURF	ACE PROFILE				ROCK S	AMPLE DA	ТА	
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLC	GICAL DESCRIPTION	DEPTH (ff)	SAMPLE NO.	PAR Small		E (mm) Largest 40	COLOR	DRILLING RATE (min/ft)
			GLACIAL SEE	DIMENT (DRIFT)	100						
		481.6	ting grained to		   <u>110</u>						3.8
			LIMESTONE; angular grains supangular ca	o amorphous, light gray bedding indistinct; subrounded to , occasional subrounded to rbonaceous particles, occasional		BS-1	10 			Buff/Gray	
	DUNDEE LIMESTONE		hydrosarbon s reaction to HC	taining, strong to moderate	120	BS-2	10			Light Gray	
	UNDEE LIN			×.	  130						
	Ō			<u> </u>		BS-3	10 - 10			Light Gray	
		452.6	DOLOMITE ar bedding indisti	amorphous, gray to brown ad DOLOMITIC LIMESTONE, nct; subrounded to subangular chert and carbonaceous particles HCL		BS-4	10 10			Light Gray	_
					150	BS-5	313			Gray	_
	(GROUP ATION)		160.0-166.0' -	Increase in hydrocarbon staining	160  	BS-6	2 10			Gray	
	DETROIT RIVER GROUP (LUCAS FORMATION)					BS-7				Gray	_
					180	BS-8	10 10			Gray	_
					190   	BS-9	2 <u>1</u> 0			Gray	_
N () E(x)	ING CC /) Coordi ) Coordir	nate 468	5981.64	Drilling Company: Advanced Energy S Drill Rig: Walker-Neer Apache 150-5g Engineer on Rig: J. Lukasik/J. Cunnig Drilling Method: Rotary Drilling Hammer Type: N/A Backfilled With: N/A Date Started: 04-12-07	F	DE)	Detroit	River In	ternation	ng Inc. al Crossing	
Remar Groun		e Elevati	on: 592.59 ft.	Date Completed: 04-15-07 Checked By: SC	PROJE	E <b>CT NO.</b> C		, Michiga Boring	nn. TB-15	PAGE 1 of	of 16

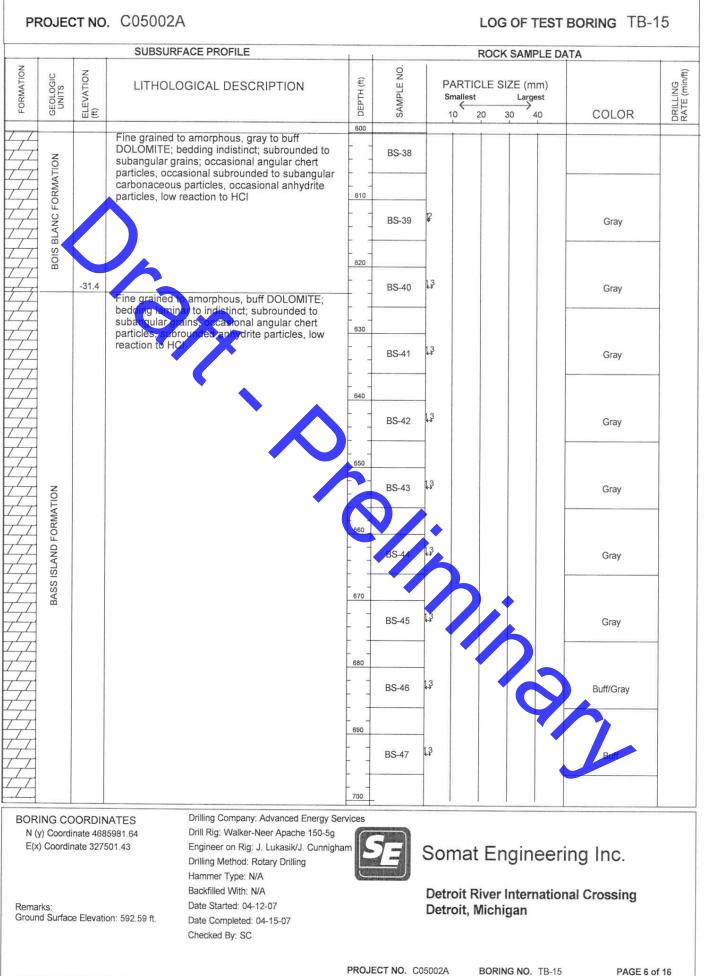


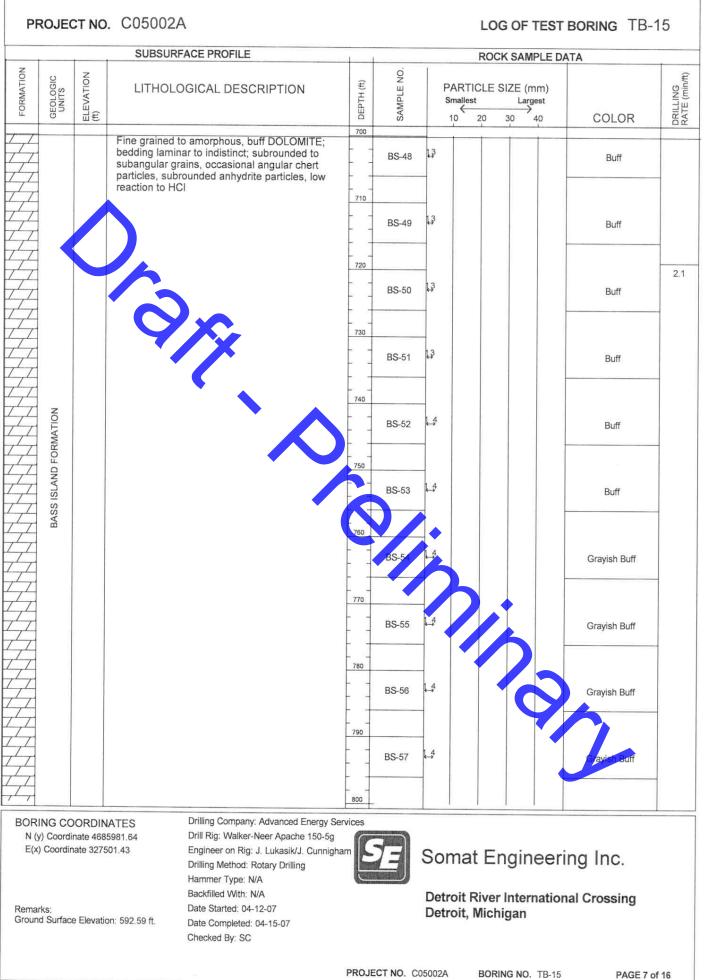


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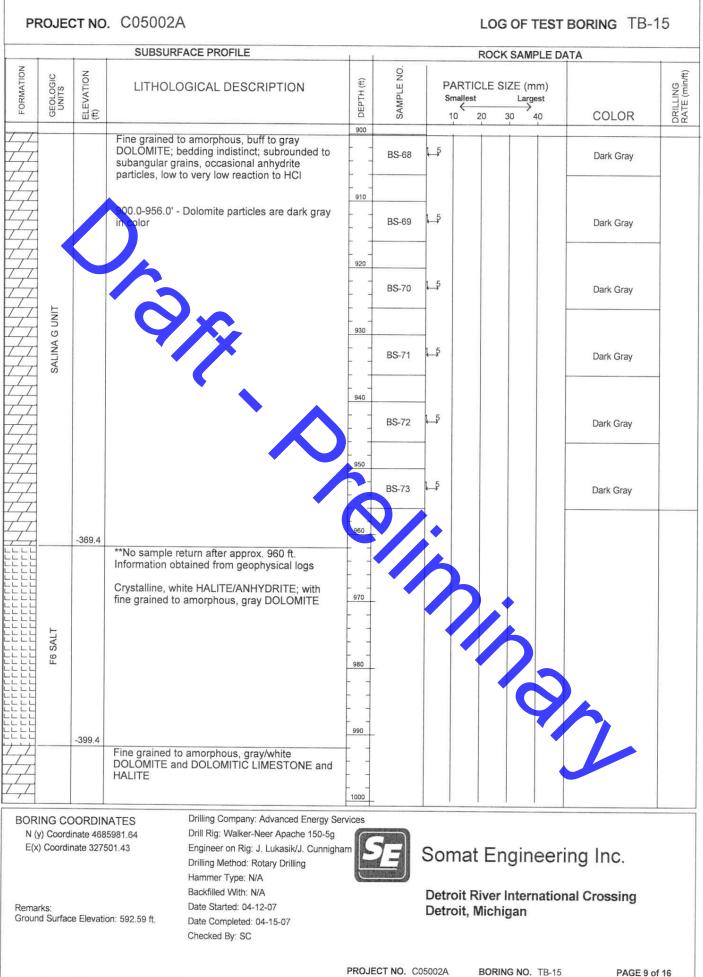
PF	ROJEC	T NO	. C050024	A				LO	G OF	TEST I	boring TB-	15
			SUBSURF	ACE PROFILE	-			ROO	CK SA	MPLE DA	TA	
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLO	OGICAL DESCRIPTION	GPTH (ft)	SAMPLE NO.	Sma			(mm) argest → 40	COLOR	DRILLING RATE (min/ft)
	DETROIT RIVER GROUP (AMHERSTBURG FORMATION)		DOLOMITE al bedding indist graines, trace low reaction to	o amorphous, gray to brown and DOLOMITIC LIMESTONE; inct; subrounded to subangular chert and carbonaceous particles, o HCL		BS-22 BS-23 BS-23 BS-25 BS-26	₽ ₽ ₽ ₽ ₽				Buff Buff Brown Brown	
		100.6	SACCHAROID	n grained, white to light gray DAL SANDSTONE; subrounded to lins, occasional dolomite eaction to HCI		BS-27	8				Вгенир	0.9
N () E(x Rema		nate 468 nate 3275	5981.64	Drilling Company: Advanced Energy Se Drill Rig: Walker-Neer Apache 150-5g Engineer on Rig: J. Lukasik/J. Cunnigh Drilling Method: Rotary Drilling Hammer Type: N/A Backfilled With: N/A Date Started: 04-12-07 Date Completed: 04-15-07 Checked By: SC	am		Detroi Detroi	it Rive it, Micl	r Inte higar	ernation	ng Inc. al Crossing	of 16

		,	SUBSUR	FACE PROFILE	1 1			ROCI	K SAMPLE D	ATA	
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLO	OGICAL DESCRIPTION	00 DEPTH (ft)	SAMPLE NO.	s	ARTICLE S	SIZE (mm)	COLOR	DRILLING
			SACCHAROI subangular gr	m grained, white to light gray DAL SANDSTONE; subrounded ains, occasional dolomite reaction to HCl		BS-28	12			Brown	
					 510 	BS-29	12			Brown	
			1		520	BS-30	24			Brown	
	STONE		Ò	×.	530						
	SYLVANIA SANDSTONE					BS-31	22- 			Brown	
	SYLVA			` ^	540	BS-32				White/Light Gray	
			560.0-590.0' -	Particles become more	550	BS-33	P			White/Light Gray	
			subangular		 	BS-3	8			White/Light Gray	
		20.6	DOLOMITE; b	o amorphous, gray to buff edding indistinct; subrounded to ains; occasional angular chert	0	BS-35	2	2		White/Light Gray	1.8
	BOIS BLANC FORMATION		particles, occa carbonaceous	particles, occasional anhydrite eaction to HCI	ar	BS-36	12			White/Light Gray	
	BOIS BLANC				 	BS-37	R.			Car	
N (y E(x) Remar	/) Coord ) Coordii rks:	DORDIN inate 468 nate 3275 e Elevatio	5981.64	Drilling Company: Advanced Energ Drill Rig: Walker-Neer Apache 150- Engineer on Rig: J. Lukasik/J. Curu Drilling Method: Rotary Drilling Hammer Type: N/A Backfilled With: N/A Date Started: 04-12-07 Date Completed: 04-15-07 Checked By: SC	-5g	Ē	Detr		Internatio	ring Inc. nal Crossing	



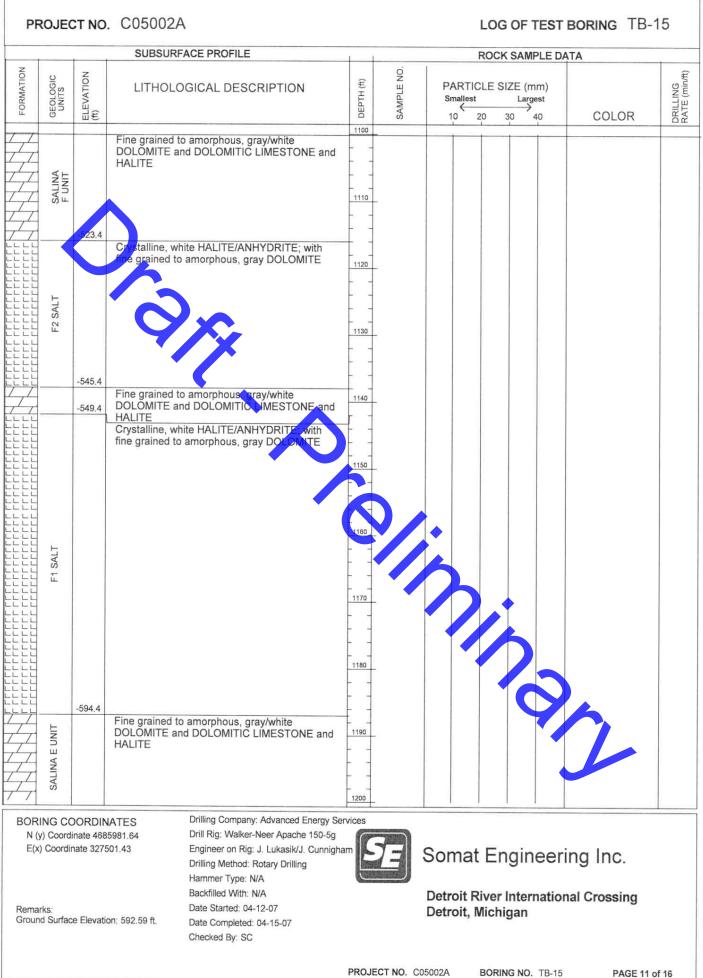


PF	OJEC	T NO	. C05002A	N N				LOG OF	TEST BC	DRING TB-	15
			SUBSURF	ACE PROFILE				ROCK SAM	PLE DATA		
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLO	GICAL DESCRIPTION	00 DEPTH (ft)	SAMPLE NO.	Sn	ARTICLE SIZE (n nallest Larg		COLOR	DRILLING RATE (min/ft)
			bedding lamin subangular gra	o amorphous, buff DOLOMITE; ar to indistinct; subrounded to ains, occasional angular chert ounded anhydrite particles, low		BS-58	L4			Gray	0.8
				4	810	BS-59	₩4			Gray	
			1		820	BS-60	4			Gray	
	VTION		Q	R.	830						
	BASS ISLAND FORMATION		•		840	BS-61	4			Gray	
	BASS ISL					BS-62	<b>↓</b> 4			Gray	
					850	BS-63	L.4			Gray	
					860	BS-64	L4			Buff	
		-283.4			870	BS-65	4			Buff	
	UNIT		DOLOMITE; be subangular gra	amorphous, buff to gray edding indistinct; subrounded to ins, occasional anhydrite o very low reaction to HCI	880  	BS-66	L4		0	Buff	
	SALINA G UNIT		890.0-956.0' - ( 10mm (3/8'') in	Occasional large particles up to size	890	BS-67	1_4 4			But	
		ORDIN		Drilling Company: Advanced Energy Se	900 rvices						
		nate 468 late 3275		Drill Rig: Walker-Neer Apache 150-5g Engineer on Rig: J. Lukasik/J. Cunnigha Drilling Method: Rotary Drilling Hammer Type: N/A		<b>SE</b>	Son	nat Engin	eering	g Inc.	
Remar Ground		e Elevatio	on: 592.59 ft.	Backfilled With: N/A Date Started: 04-12-07 Date Completed: 04-15-07 Checked By: SC				oit River Inter oit, Michigan	national	Crossing	
					PROJE	ECT NO. CO	)5002A	BORING NO.	TB-15	PAGE 8 c	of 16

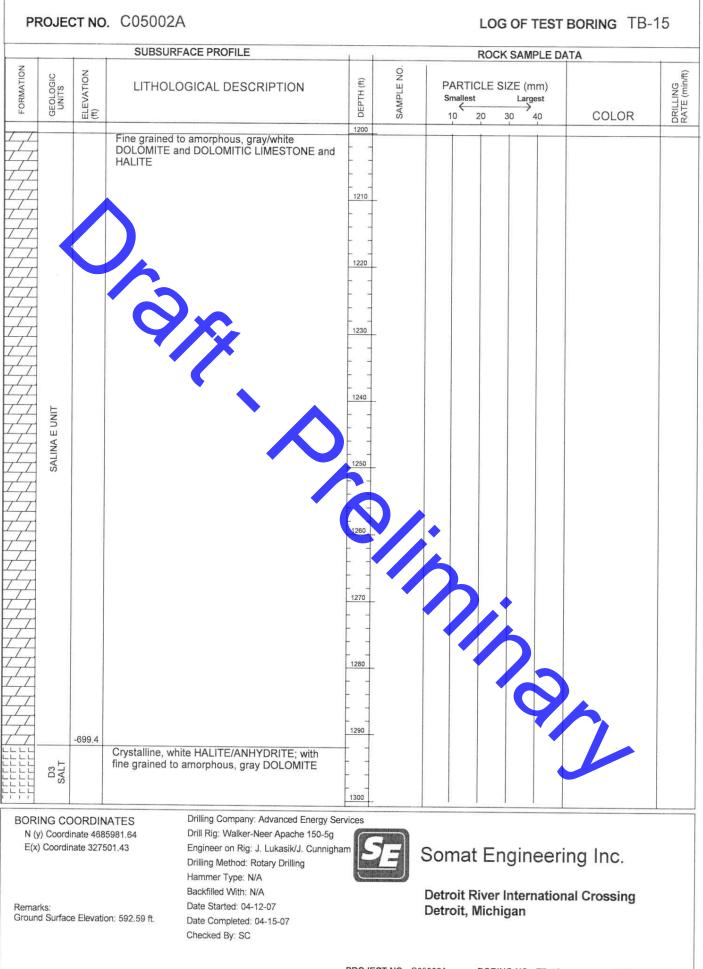


PR	ROJEC	CT NO	. C05002A	Ą				LO	G OF 1	TEST I	BORING TB-	15
		1	SUBSURF	ACE PROFILE	+ +			ROC	K SAM	PLE DA	TA	1
FORMATION	GEOLOGIC	ELEVATION (ft)	LITHOLO	OGICAL DESCRIPTION	DEPTH (ft)	SAMPLE NO.	PAR Small	TICLE S	Larg		COLOR	DRILLING RATE (min/ft)
		-409.4			1000	-						
	F5 SALT	-415.4	fine grained to	ite HALITE/ANHYDRITE; with amorphous, gray DOLOMITE								
	SALINA		Fine grained to DOLOMITE an HALITE	o amorphous, gray/white nd DOLOMITIC LIMESTONE and								
		-431.4	Crystalline, w fine grained to	ite HALITE/ANHYDRITE; with	1020	r. T						
	F4 SALT				 <u>1030</u>   <u>-</u> <u>1040</u> 							
	SALINA F UNIT	-454.4	Fine grained to DOLOMITE ar HALITE	o amorphous, gray/white ad DOLOMITIC LIMESTONE and	1050							
	F3 SALT		Crystalline, wh fine grained to	ite HALITE/ANHYDRITE; with amorphous, gray DOLOMITE	1060    1070    1080  		· ?		2	C		
	SALINA F UNIT	-494.4	Fine grained to DOLOMITE an HALITE	amorphous, gray/white d DOLOMITIC LIMESTONE and	 1090    1100						2	
N (y E(x) Remari	) Coordin Coordin ks:	ORDIN nate 468 aate 3275	5981.64	Drilling Company: Advanced Energy S Drill Rig: Walker-Neer Apache 150-5g Engineer on Rig: J. Lukasik/J. Cunnigh Drilling Method: Rotary Drilling Hammer Type: N/A Backfilled With: N/A Date Started: 04-12-07 Date Completed: 04-15-07 Checked By: SC	C			River	Interr		ng Inc. al Crossing	<u> </u>
					PROJF	CT NO. CO	5002A	BORI	NG NO.	TB-15	PAGE 10 c	of 16

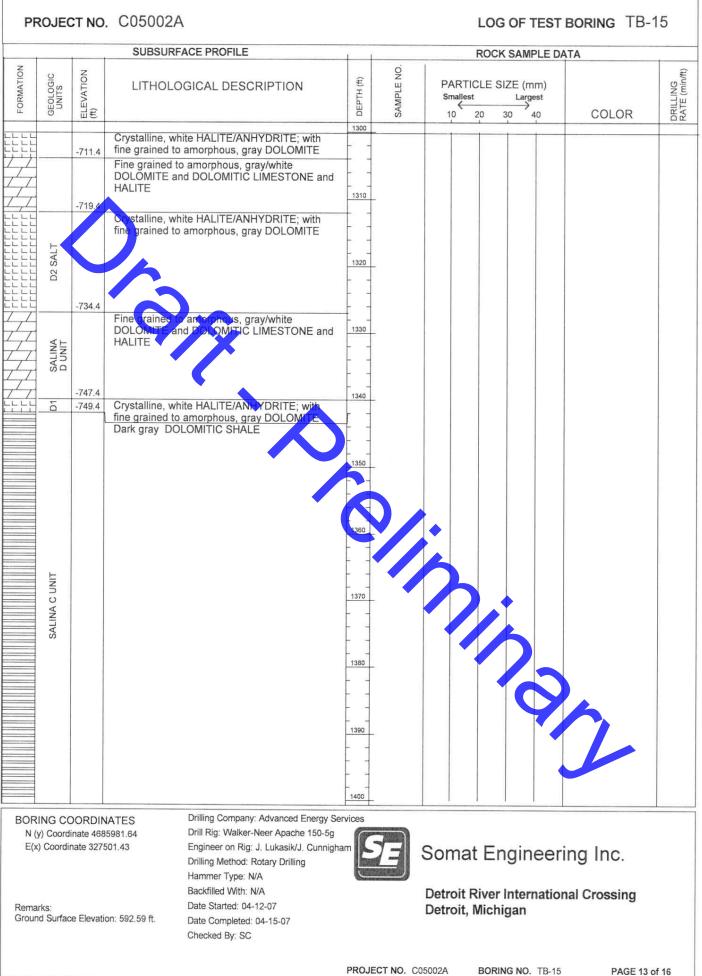
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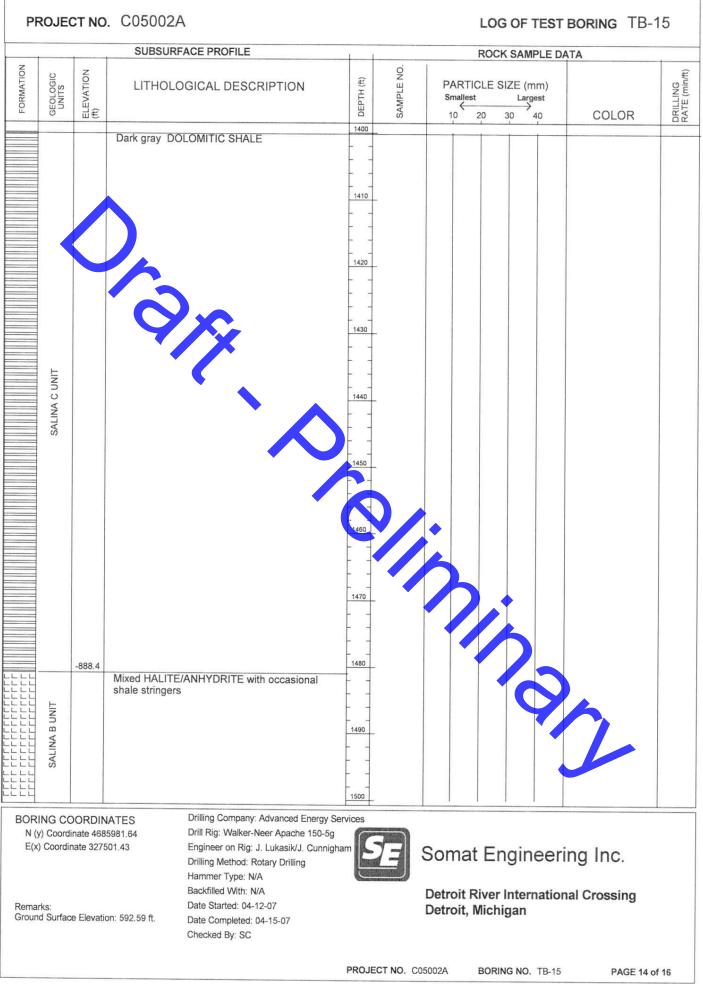


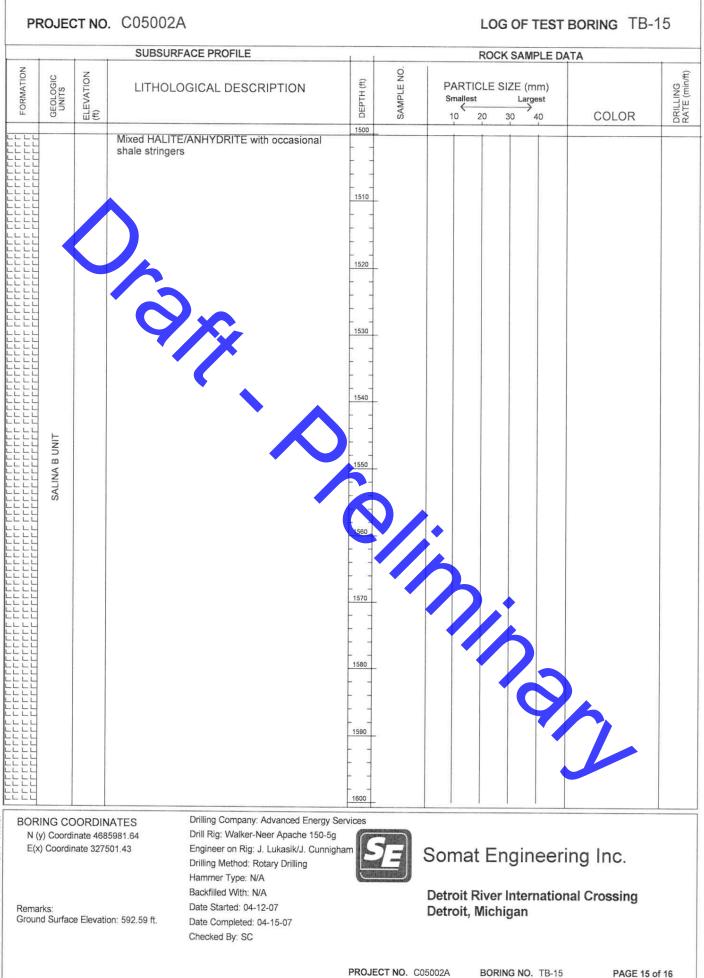
PAGE 11 01



PROJECT NO. C05002A







PROJECT NO. C05002A

